

PLAINFIELD COMMUNITY  
SCHOOL CORPORATION  
PLAINFIELD OUTBUILDINGS

PLAINFIELD, IN  
100% CONSTRUCTION DOCUMENTS      MAY 08, 2025

CLARKS CREEK  
ELEMENTARY SCHOOL

401 ELM DR., PLAINFILED, IN 46168

BRENTWOOD  
ELEMENTARY SCHOOL

1630 W OLIVER AVE, PLAINFIELD, IN 46168

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S402	TYPICAL DETAILS
S701	SECTIONS AND DETAILS

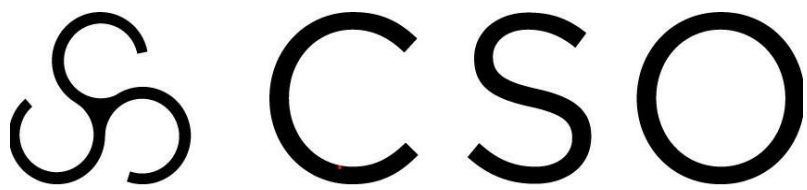
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E201	OUTBUILDING PLANS - ELECTRICAL
E601	SCHEDULES - ELECTRICAL

CONSTRUCTION MANAGER



ARCHITECT



8831 Keystone Crossing, Indianapolis, IN 46240  
317.848.7800 | csoinc.net

MECHANICAL/ELECTRICAL/PLUMBING ENGINEER



STRUCTURAL ENGINEER



CIVIL ENGINEER



8831 Keystone Crossing, Indianapolis, IN 46240  
317.848.7800 | csoinc.net  
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PLAINFIELD COMMUNITY  
SCHOOL CORPORATION  
PLAINFIELD OUTBUILDINGS  
PLAINFIELD, IN

SCOPE DRAWINGS:  
These drawings indicate the general scope of the project in terms of architectural design, concept, the dimensions of the building, the major architectural elements and the type of structural, mechanical and electrical systems.  
The drawings do not necessarily indicate or describe all work required for full performance and completion of the requirements of the Contract.  
On the basis of the general scope indicated or described, the sub-contractor shall furnish all items required for the proper execution and completion of the work.

PLAINFIELD COMMUNITY  
SCHOOL CORPORATION  
100% CONSTRUCTION DOCUMENTS  
MAY 08, 2025



# BRENTWOOD & CLARKS CREEK ELEMENTARY - STORAGE BUILDINGS

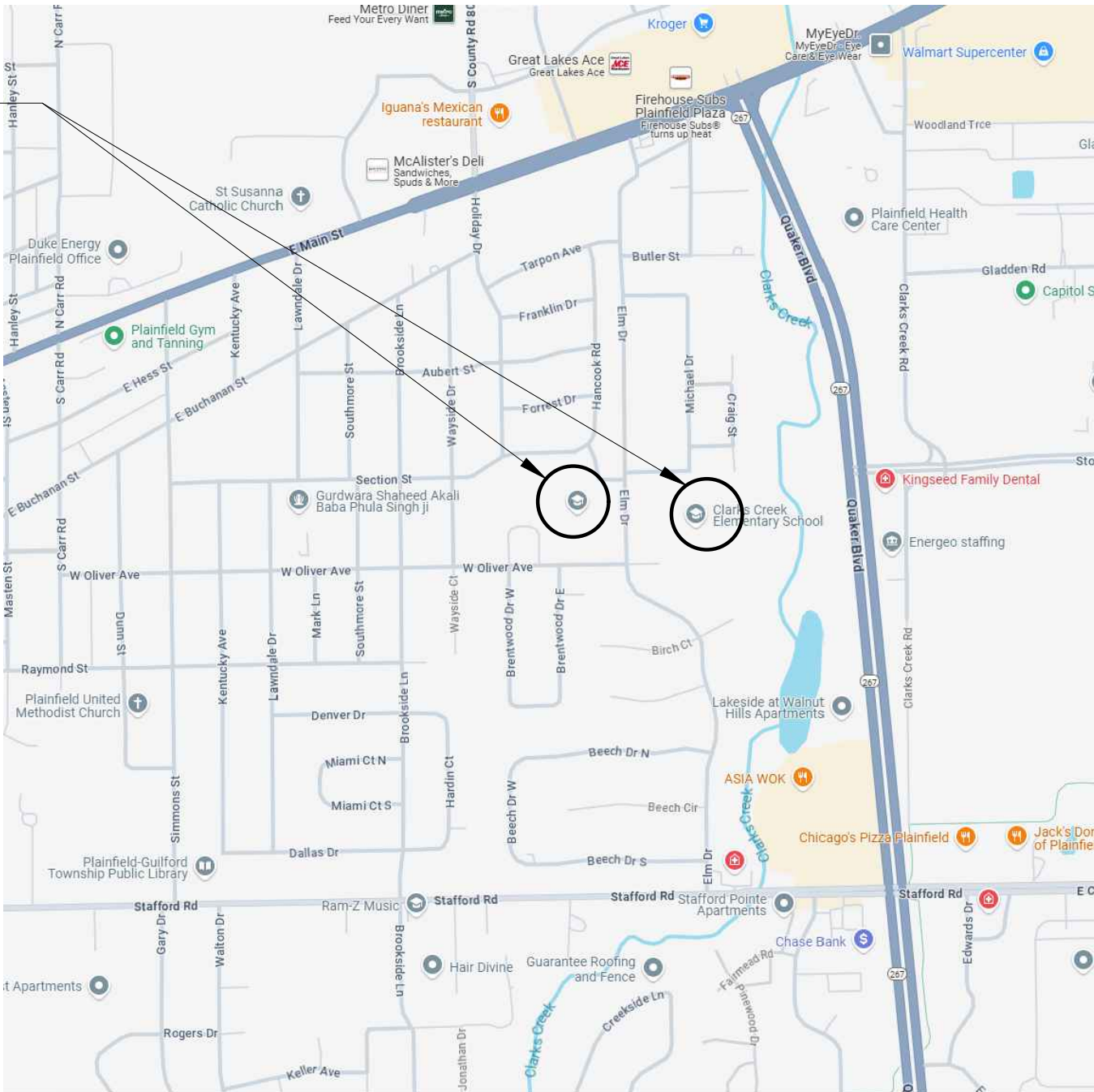
NW 1/4, SEC. 36-T15N-R1E, HENDRICKS COUNTY, GUILFORD TOWNSHIP, PLAINFIELD, INDIANA  
PROJECT ADDRESS: BRENTWOOD - 1630 OLIVER AVE., CLARKS CREEK - 401 ELM DR.  
ZONED: S

PLANS PREPARED FOR:  
CSO ARCHITECTS  
8831 KEYSTONE CROSSING  
INDIANAPOLIS, IN 46240  
317-848-7800  
CONTACT: BRET HITE  
EMAIL: bHITE@csoinc.net



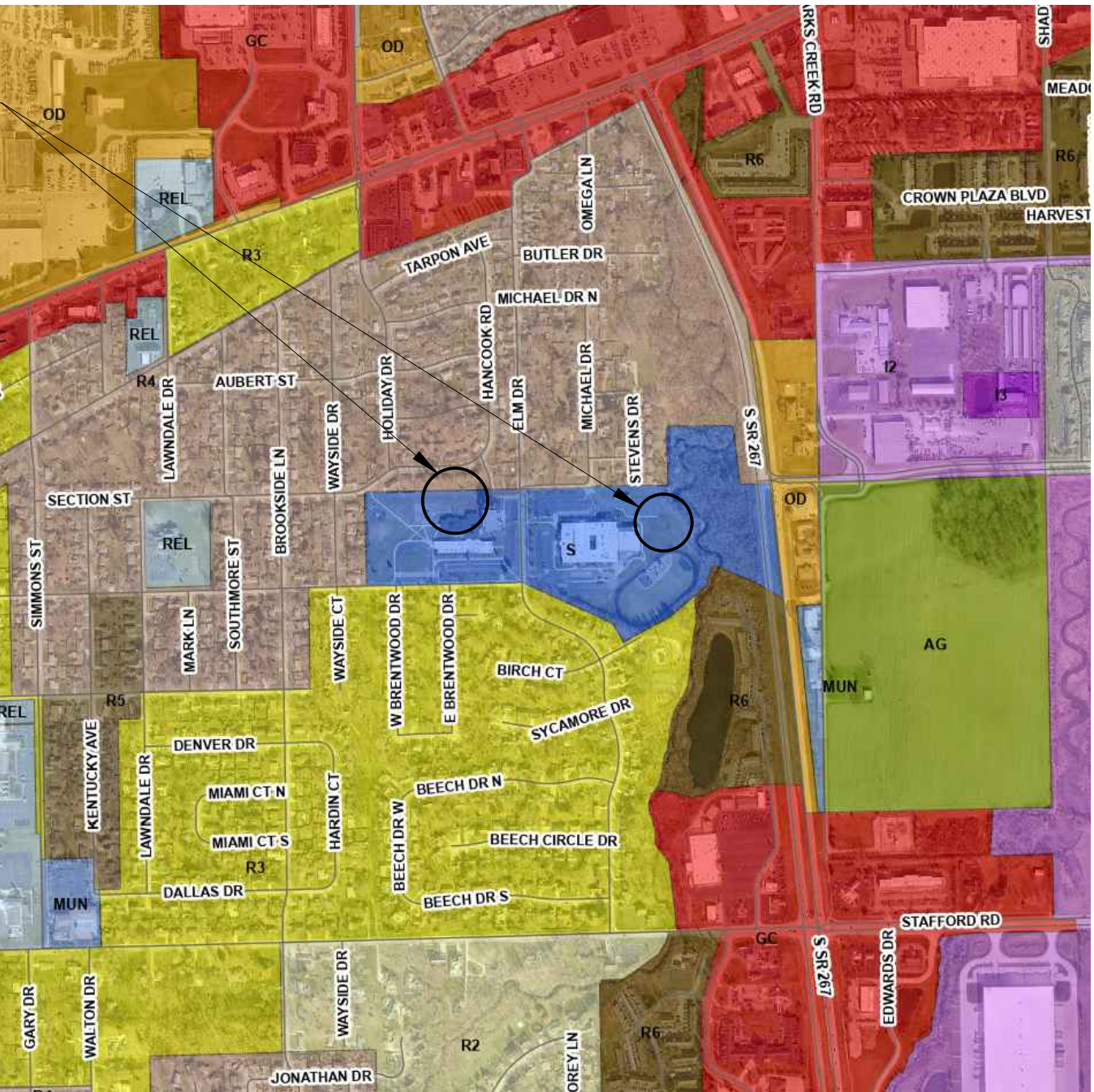
OWNER:  
PLAINFIELD COMMUNITY  
SCHOOL CORPORATION  
985 LONGFELLOW DRIVE  
PLAINFIELD, IN 46168  
317-839-2578  
CONTACT: MARK SHAYOTOVICH  
EMAIL: mshayotovitch@plainfield.k12.in.us

PROJECT  
LOCATION



VICINITY MAP  
NO SCALE

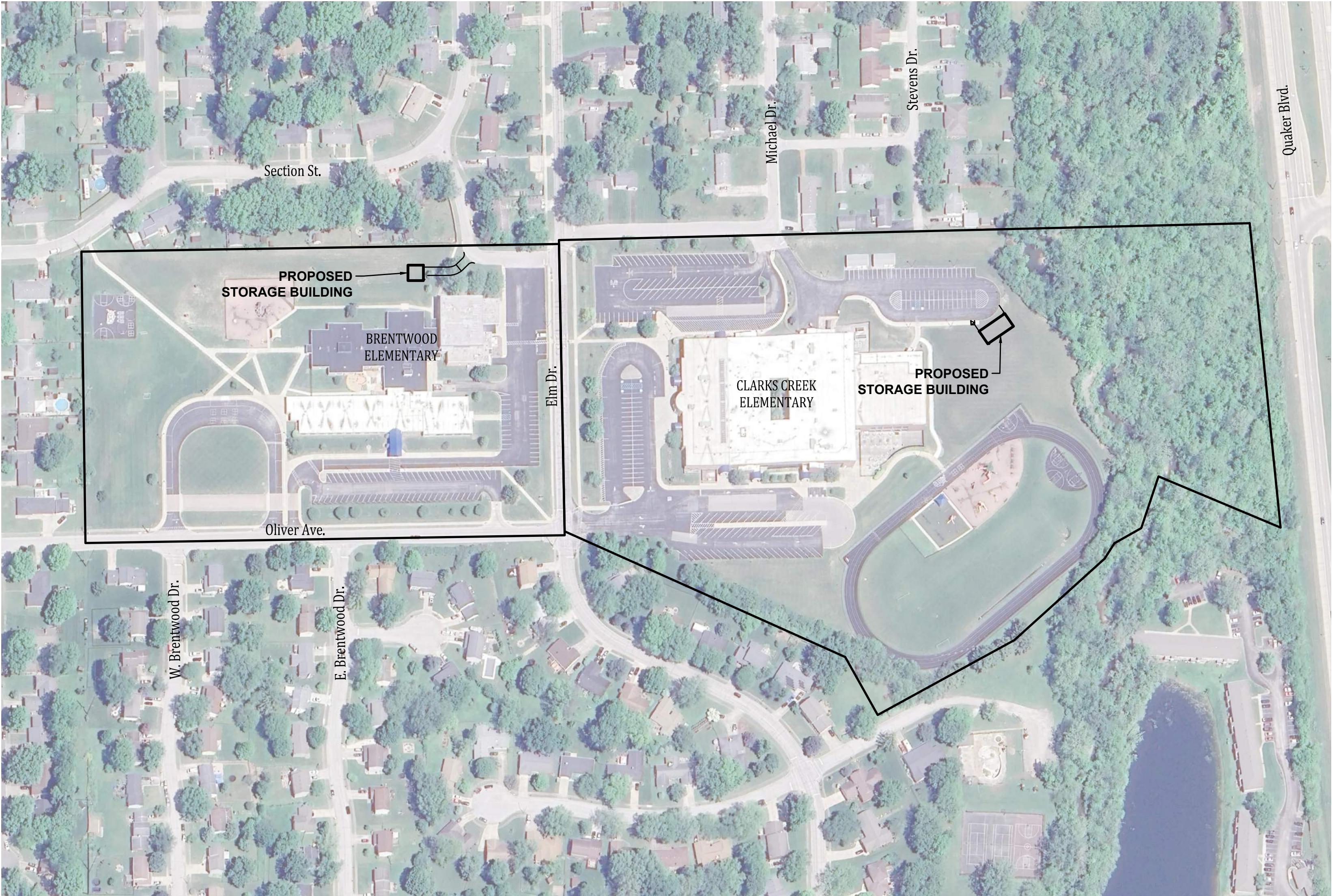
PROJECT  
LOCATION



LOCATION MAP  
NO SCALE

## OPERATING AUTHORITIES

<b>GAS</b> CenterPoint Energy 1995 E. Main Street Danville, IN 46122 317-718-3617	<b>FIBER OPTIC</b> Zayo Bandwith 722 N. High School Road Indianapolis, IN 46214 765-341-1199	<b>WATER</b> Town of Plainfield 986 S. Center Street Plainfield, IN 46168 317-839-3490
<b>ELECTRIC</b> Duke Energy 5095 E. Main Street P.O. Box 29 Danville, IN 46122 317-745-4481	<b>COMMUNICATIONS</b> Windstream 4005 N Rodney Parham Road Little Rock, AK 72212 800-289-1901	<b>COMMUNICATIONS</b> AT&T 240 N. Meridian Street Indianapolis, IN 46207 317-722-2299
<b>SANITARY</b> Town of Plainfield 986 S. Center Street Plainfield, IN 46168 317-839-3490	<b>STORM</b> Town of Plainfield 986 S. Center Street Plainfield, IN 46168 317-839-3490	<b>FIRE DEPARTMENT</b> Plainfield Fire Territory 591 Moon Road Plainfield, IN 46168 317-839-6939
<b>SCHOOL DISTRICT</b> Plainfield Community School Corporation 985 Longfellow Drive Plainfield, IN 46168 317-839-2578	<b>CABLE</b> Comcast of Indiana P.O. Box 20911 Indianapolis, IN 46220 317-594-8509	



SITE MAP  
NO SCALE

## PLANS PREPARED BY:

**BANNING**  
ENGINEERING

853 COLUMBIA ROAD, SUITE #101  
PLAINFIELD, IN 46168  
BUS: (317) 707-3700, FAX: (317) 707-3800  
E-MAIL: Banning@BanningEngineering.com  
WEB: www.BanningEngineering.com

CONTACT: RYAN LINDLEY  
EMAIL: rlindley@banning-eng.com

## CONSTRUCTION DOCUMENTS

PROJECT MANAGER: Ryan R. Lindley DATE: 05-19-2025  
THESE PLANS ARE NOT TO BE CONSIDERED FINAL OR TO BE UTILIZED FOR  
CONSTRUCTION UNLESS SIGNED AND DATED BY THE APPROPRIATE BANNING  
ENGINEERING PROJECT MANAGER.  
THESE PLANS ARE NOT INTENDED TO BE REPRESENTED AS A RETRACEMENT  
OR ORIGINAL BOUNDARY SURVEY, A ROUTE SURVEY, OR A SURVEYOR  
LOCATION REPORT.



CERTIFIED BY: W. Chad Ziegler

## REVISIONS

NUMBER	DESCRIPTION	DATE

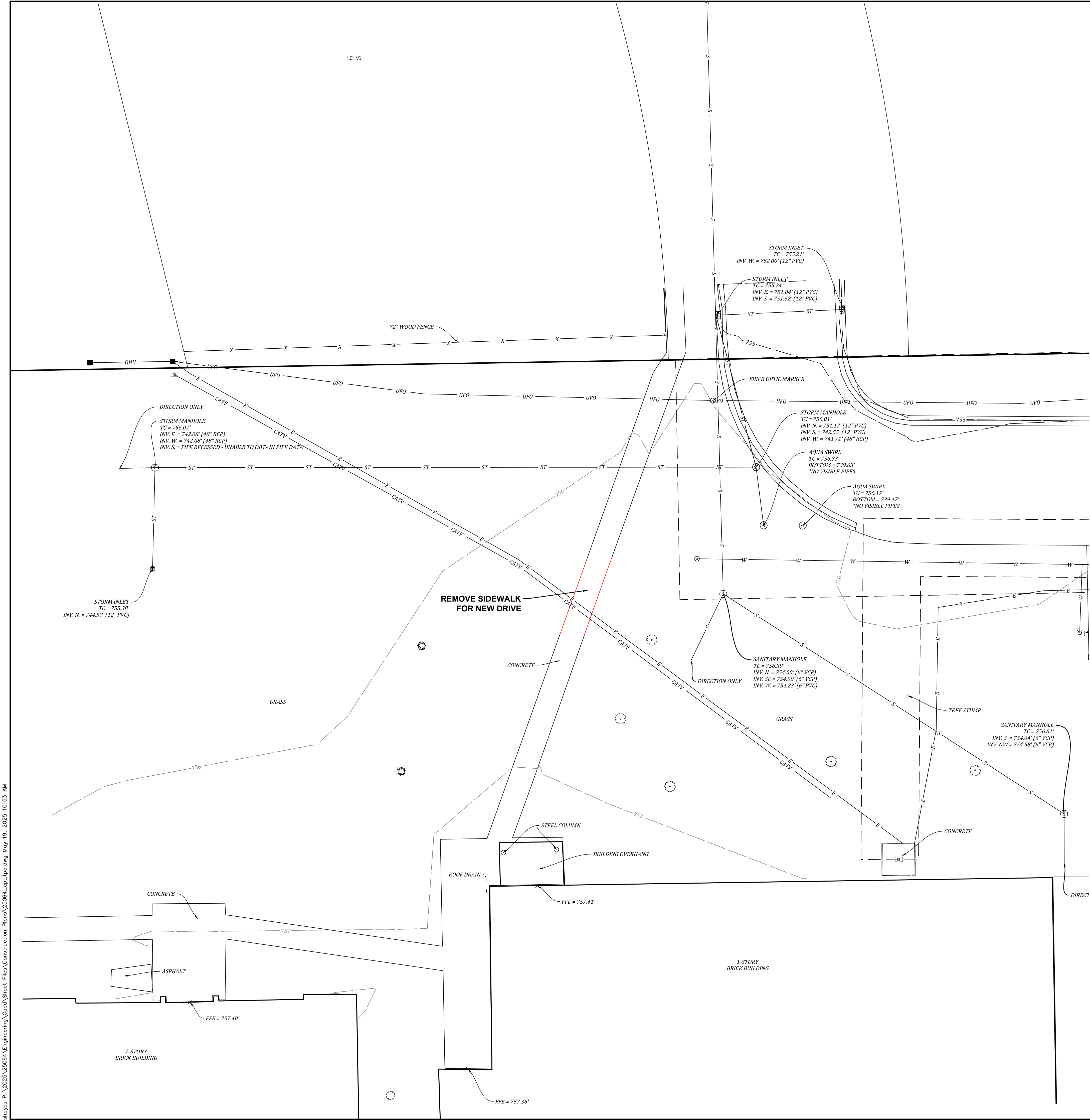
## SHEET INDEX

SHEET NO	DESCRIPTION
C100	TITLE SHEET
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s:\shoyes P:\2025\2506A\Engineering\Cadd\Sheet Files\Construction Plans\25064\_cp\_bod.dwg, May 19, 2025 10:53 AM



**Project Benchmark**

The elevations shown hereon were derived from GPS observation using NAVD 88 Datum from the VRS (Virtual Reference Station) Control Network established by Sailer Company. The elevations were established for this network by continuously streaming data through fixed reference stations ((CORS)-Continuously Operating Reference Station) distributed throughout the network area to a central server running Trimble GPSNet and RTKNet software. Through previous survey work, the elevations derived from GPS observations are relatively accurate to published benchmarks (usually within +/-0.2 feet). Control point 1 was used as the benchmark and levels were ran through the temporary benchmarks shown below.  
C.P. #1 Elev = 752.12' (NAVD 88)

**Site Benchmark**

**TBM #50**  
Cut square on the southeast side of concrete light base located about 115 feet north of the Clarks Creek Elementary School building, about 100 feet east of the centerline of Michael Drive and 2.5 feet north of the back of curb.  
Elev. = 755.49' (NAVD 88)

**TBM #51**  
Cut "X" on the east bonnet bolt of fire hydrant located about 51 feet north of the northeast corner of the Brentwood Elementary School building, about 112 feet west of the centerline of Elm Drive, about 31 feet south of the centerline of Hancock Road.  
Elev. = 758.23' (NAVD 88)

**Notes:**  
Per 865 IAC 1-12-12 this drawing is not intended to be represented as a retracement or original boundary survey, a route survey, or a Surveyor Location Report.

The horizontal data shown on this exhibit is based upon standard radial survey techniques and by global positioning equipment, utilizing the VRS Network, a real-time kinematic (RTK) correction service over the internet. The coordinate values shown are in Indiana State Plane West Zone on the 1983 North American Datum.

All bearings, distances and coordinates are referenced to the Indiana State Plane West Zone (NAD 83) Coordinate System. The Combined Scale Factor for this project is 1.000000. Distances shown hereon are GRID distances (US survey feet).

The topographic information shown hereon was obtained in the field during March 2025. The topographic data was gathered using a robotic total station and data collector applying standard radial surveying techniques and by global positioning equipment, utilizing the VRS Network, a real-time kinematic (RTK) correction service over the internet.

Elevations on hard surfaces or structures are accurate to within 0.05 feet, elevations on natural surfaces are accurate to within 0.15 feet. The contours shown hereon were plotted based upon interpolation of spot elevations and other topographic information and are accurate to within one half of the contour interval.

This survey reflects above ground indications of utilities and information available from utility companies. The surveyor makes no guarantee that the underground utilities shown comprise all such utilities in the area, either in service or abandoned. The surveyor further does not warrant that the underground utilities shown are in the exact location indicated, although they are located as accurately as possible from the information available. The surveyor has not physically located the underground utilities.

Underground utilities shown per private utility locates were provided internally via Banning Engineering's Utilities Coordinator.

**FLOOD STATEMENT**

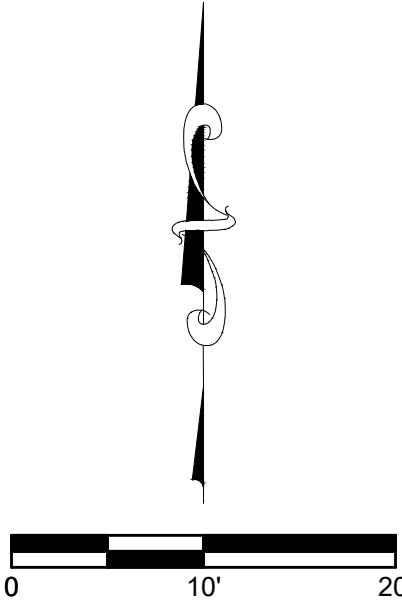
The surveyed property appears to be located in Zone "X" (areas determined to be outside the 0.2% annual chance floodplain) per the Flood Insurance Rate Map (FIRM) for Hendricks County, Indiana, community panel 18063C0259D, dated September 25, 2009, Federal Emergency Management Agency, National Flood Insurance Program. Per said FIRM, this parcel does not lie within the boundary of a Special Flood Hazard Area. The accuracy of any flood hazard statement is subject to map scale uncertainty.

**LEGEND**

**EXISTING**

- OHU — OVERHEAD UTILITY LINE
- W — WATER LINE
- G — GAS LINE
- E — UNDERGROUND ELECTRIC LINE
- UFO — UNDERGROUND FIBER OPTIC LINE
- CATV — UNDERGROUND CABLE TV LINE
- ST — STORM SEWER LINE
- S — SANITARY SEWER LINE
- X — FENCE
- ⊕ STORM SEWER STRUCTURES
- Ⓢ SANITARY SEWER STRUCTURES
- UTILITY POLE
- ⊗ LIGHT POLE
- Ⓢ UTILITY VAULT
- ⊙ WATER METER
- Ⓢ ELECTRIC TRANSFORMER
- ⊙ FIRE HYDRANT
- Ⓢ SEWER CLEAN-OUT
- Ⓢ SIGN
- Ⓢ TREE

Control Point				
Pt. #	Northing	Easting	Elev.	Description
1	1623640.709	3152139.164	752.12	Cut "X"
2	1623778.087	3151727.105	758.85	Cut "X"
3	1623732.377	3151305.536	756.51	Cut "X"



EXISTING TOPOGRAPHY AND DEMO PLAN  
BRENTWOOD & CLARKS CREEK  
ELEMENTARY - STORAGE BUILDINGS  
PLAINFIELD, INDIANA



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PLAINFIELD, IN 46169  
BUS: (317) 707-3700 FAX: (317) 707-3800  
E-MAIL: Banning@BanningEngineering.com  
WEB: www.BanningEngineering.com

Project No: 25064  
Sheet No:



C101

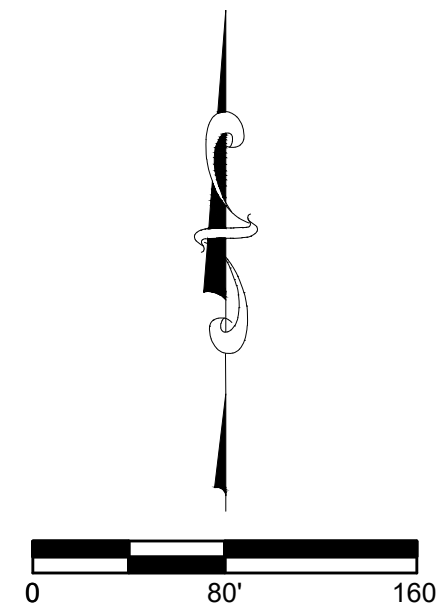






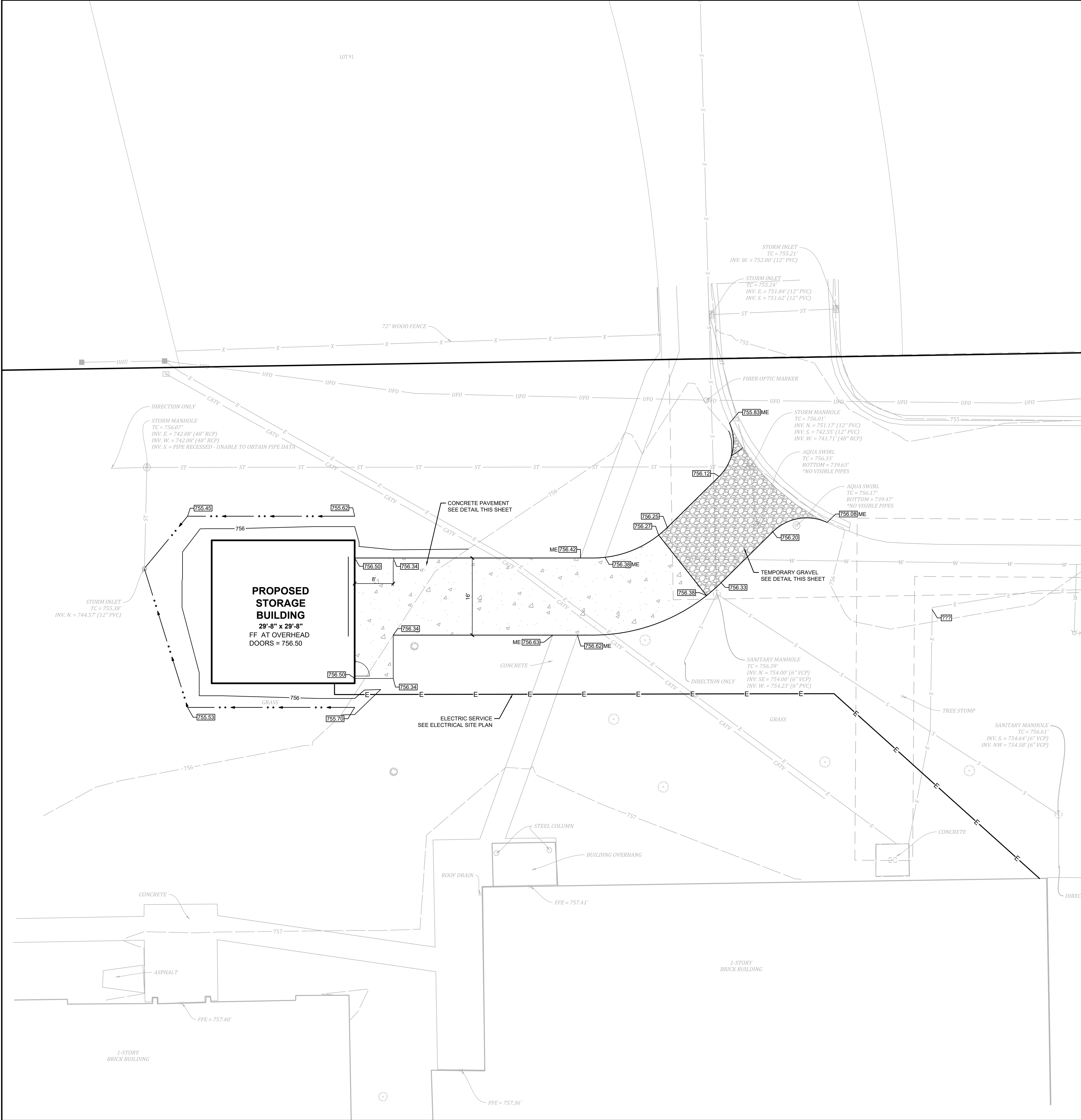
west quarter of Section 25, township 15 north, range 1 east for (an assumed meridian) and on along the north line of said quarter section 907.33 feet to an old corner stone, thence run North 89 degrees 52 minutes east on and along said North line 12.50 feet and to the center of a well, thence along said well 12.50 feet to the center of a well, thence along said well and along an extension of said Elm Drive 561.08 feet; thence North 89 degrees 45 minute s 45 seconds West a distance of 922.83 feet and to the west line of said quarter section center, at the center of Oliver Avenue in the town of Plainfield, Indiana; thence run North 00 degrees 21 minute East 211.00 feet and along said west line 561.00 feet and to the place of beginning. Containing 91 acres or less. Subject to any and all easements or rights of way which may affect the above real estate.

Commencing at the Southwest corner of said quarter section; thence north 88 degrees 47 minutes 00 seconds East (assumed bearing) on and along the South line of said quarter section 2431.9 feet to a point that is south 88 degrees 47 minutes 00 seconds west 269.69 feet from the Southwest corner of said quarter section; thence north 00 degrees 29 minutes 40 seconds west 16.44 feet to the West right-of-way line of State Road 267 as now located and established; thence north 48 degrees 38 minutes 13 seconds East on and along said west right-of-way line 53.33 feet to the Northwest corner of said quarter section; thence north 00 degrees 30 minutes west right-of-way line 1818.72 feet to the beginning point of this description; thence continue north 05 degrees 30 minutes 00 seconds west on and along the last described course 588.94 feet to a point on the north line of said quarter section, said point being south 88 degrees 54 minutes 53 seconds west 450.80 feet from the Northeast corner of said quarter section; thence south 88 degrees 54 minutes 53 seconds west on and along said North line 1330.39 feet to a point that is north 88 degrees 54 minutes 53 seconds east 919.83 feet from the northwest corner of said quarter section; thence south 00 degrees 48 minutes 19 seconds east 550.96 feet to a point on the northeast boundary line of said quarter section; thence continue north 61 degrees 36 minutes 36 seconds east on and along the centerline of Clark's Creek following its winding course to the mouth of said creek in the office of the Recorder of Hendricks County, Indiana; thence with northerly and easterly boundaries of said Walnut Hill Section Four south 65 degrees 34 minutes 12 seconds east 592.00 feet; thence south 28 degrees 53 minutes 15 seconds east 127.96 feet; thence north 61 degrees 36 minutes 05 seconds east 100.00 feet; thence continue north 61 degrees 36 minutes 05 seconds east leaving the boundary of Walnut Hill Section Four 200.00 feet; thence north 48 degrees 12 minutes 36 seconds east 234.31 feet to a point in the centerline of Clark's Creek as now located and established; thence on and along the centerline of Clark's Creek the following distance: thence north 61 degrees 36 minutes 05 seconds east 100.00 feet; thence north 60 degrees 48 minutes 08 seconds east 59.25 feet; thence north 18 degrees 46 minutes 30 seconds east 105.00 feet; thence south 66 degrees 58 minutes 54 seconds east 255.00 feet to the point of beginning containing 21.79 acres; more or less subject to all legal highways, rights-of-way and easements of record.



C103





























### GENERAL NOTES

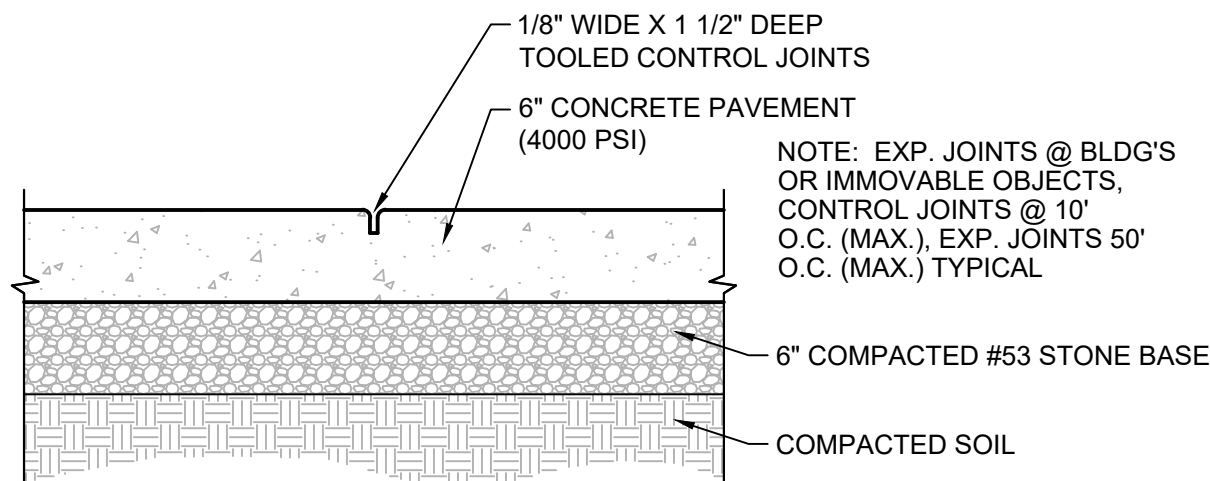
1. All work shall be performed in conformance with the Subdivision Control Ordinance of the Town of Plainfield and all other ordinances which pertain to this type of work.
2. No changes in or departure from the plans or specifications shall be made without prior approval, in writing, by the Engineer.
3. The Contractor shall be responsible for obtaining all Federal, State, County and Town of Plainfield permits, or any other permits required.
4. Before construction begins, the Contractor shall field verify the location of all utilities shown on the plans, and contact all utility companies to locate all mains, conduits, service lines, etc., in the construction area, and shall protect all utilities during construction.
5. Before construction begins, the Contractor shall notify the Owners, and/or the Owner's Engineer, so that an inspector may be present.
6. It shall be the responsibility of the Contractor to maintain quality control throughout the project; failure to do so may result in removal and replacement of the defective work. It is recommended that the Owner have a qualified inspector on the job site at all times during construction.
7. The Engineer shall be notified of all field tie located on the site during construction. All such field tie shall be incorporated into the storm sewer system so that it remains in working condition.
8. Plans shall be bid as a working system. Any errors or omissions shall be brought to the attention of the Engineer prior to construction. In the event of the Contractor's failing to give such notice, they shall be held responsible for the results of any such errors or omissions, and the cost of rectifying the same.
9. Structural fill shall be compacted in maximum 6" lifts to 95% standard proctor.
10. Liability Insurance Policy shall be furnished to the Owner before any work is started.
11. The contractor shall notify the Town of Plainfield at least 72 hours prior to any bonded or bank credit letter site improvements are installed. A pre-construction meeting shall be set up with the Town of Plainfield, Contractor, engineer & owner prior to any construction.
12. All accessible pathways, sidewalks and drive crossings shall not exceed 5% running slope and 2% cross slope or the latest requirements of the Americans with Disabilities Act (ADA). Accessible ramps shall not exceed 1/12 slope and 2% cross slope or the latest requirements of the Americans with Disabilities Act (ADA).
13. Accessible Ramp Detectable Warning Surface. A detectable warning surface shall consist of truncated domes and be placed at each street, highway or railroad crossing. The detectable warning surface shall extend a minimum of 2ft in the pedestrian travel and be placed the entire width of a ramp, blended transition, or turning space.
14. These plans shall be used in conjunction with Town of Plainfield standards, refer to sheet 1 of 29 for "directions for use."
15. Refer to General Note #11 on sheet 1 of 29 of the Town of Plainfield standards for "Land Disturbing Mitigation Options".

**LEGEND**  
**EXISTING**

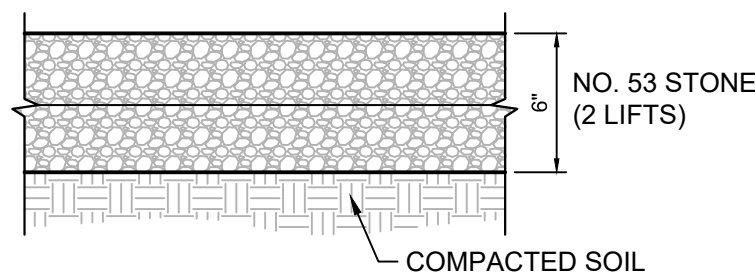
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|---|------------------------------|
|    | OVERHEAD UTILITY LINE        |
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|  | UNDERGROUND ELECTRIC LINE    |
|  | UNDERGROUND FIBER OPTIC LINE |
|  | UNDERGROUND CABLE TV LINE    |
|  | STORM SEWER LINE             |
|  | SANITARY SEWER LINE          |
|  | FENCE                        |
|  | STORM SEWER STRUCTURES       |
|  | SANITARY SEWER STRUCTURES    |
|  | UTILITY POLE                 |
|  | LIGHT POLE                   |
|  | UTILITY VAULT                |
|  | WATER METER                  |
|  | ELECTRIC TRANSFORMER         |
|  | FIRE HYDRANT                 |
|  | SEWER CLEAN-OUT              |
|  | SIGN                         |
|  | TREE                         |

**PROPOSED**

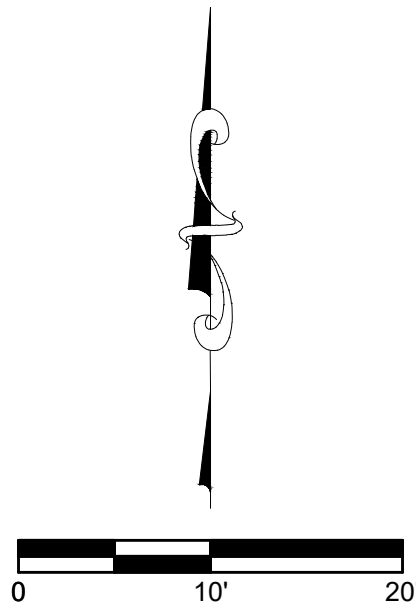
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 FLOWLINE  
 FLOW DIRECTION  
 SPOT GRADE  
 ME MATCH EXISTING



**CONCRETE PAVEMENT**  
NO SCALE



**GRAVEL SECTION**  
NO SCALE



Sym.		Revisions		Date
Designed:	SJH/RRL			
Drawn:	SJH			
Checked:	RRL			
Scale:	1"=10'			
Date:	05-06-2025			

**SITE DEVELOPMENT PLAN  
BRENTWOOD & CLARKS CREEK  
ELEMENTARY - STORAGE BUILDINGS  
PLAINFIELD, INDIANA**



W. Chad Ziegler



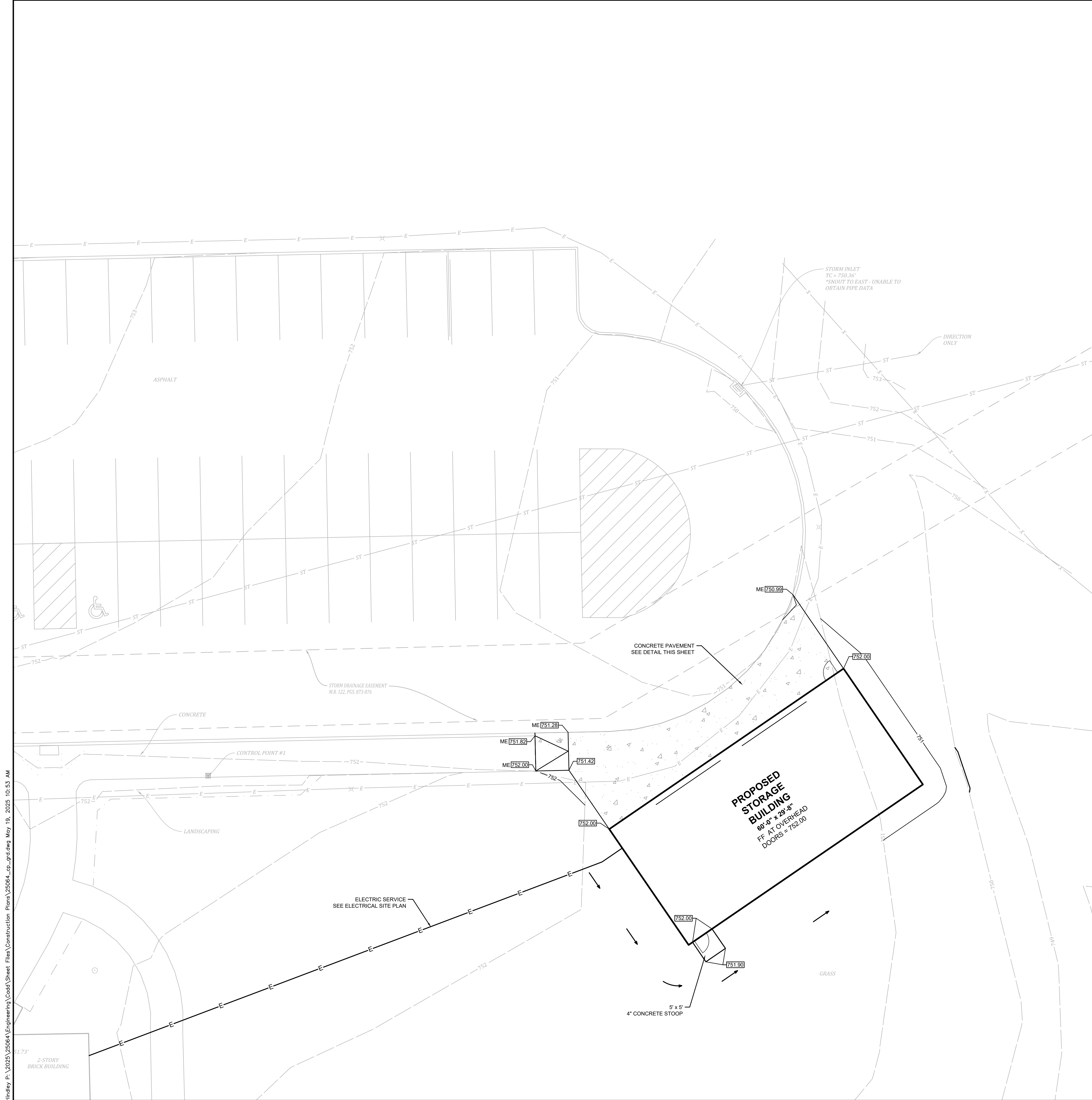
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Sheet No:

C110


























**GENERAL NOTES**

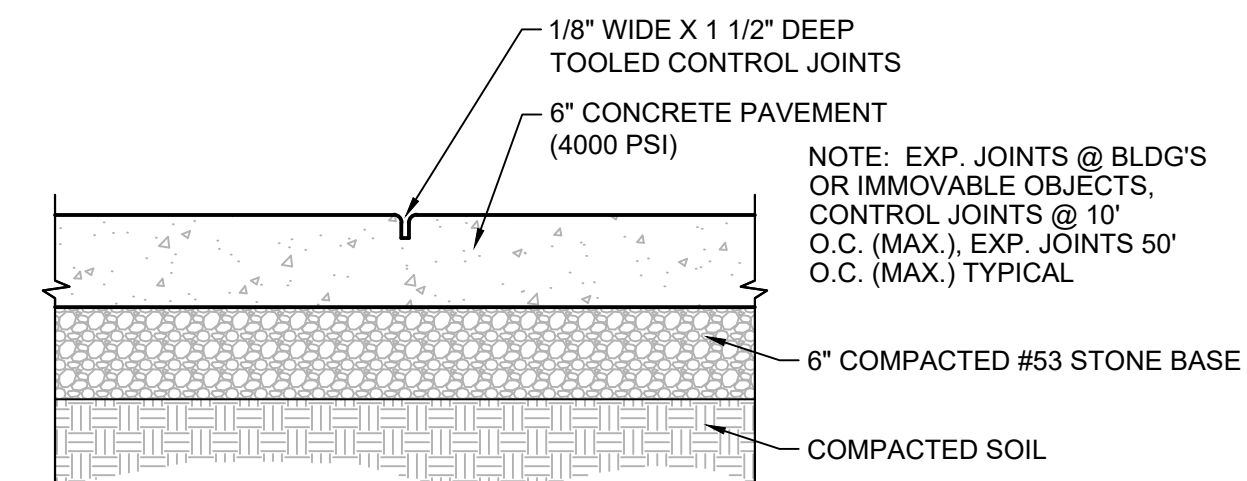
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  7. The Engineer shall be notified of all field tie located on the site during construction. All such field tie shall be incorporated into the storm sewer system so that it remains in working condition.
  8. Plans shall be bid as a working system. Any errors or omissions shall be brought to the attention of the Engineer prior to construction. In the event of the Contractor's failing to give such notice, they shall be held responsible for the results of any such errors or omissions, and the cost of rectifying the same.
  9. Structural fill shall be compacted in maximum 6" lifts to 95% standard proctor.
  10. Liability Insurance Policy shall be furnished to the Owner before any work is started.
  11. The contractor shall notify the Town of Plainfield at least 72 hours prior to any bonded or bank credit letter site improvements are installed. A pre-construction meeting shall be set up with the Town of Plainfield, Contractor, engineer & owner prior to any construction.
  12. All accessible pathways, sidewalks and drive crossings shall not exceed 5% running slope and 2% cross slope or the latest requirements of the Americans with Disabilities Act (ADA). Accessible ramps shall not exceed 1/12 slope and 2% cross slope or the latest requirements of the Americans with Disabilities Act (ADA).
  13. Accessible Ramp Detectable Warning Surface. A detectable warning surface shall consist of truncated domes and be placed at each street, highway or railroad crossing. The detectable warning surface shall extend a minimum of 2ft in the pedestrian travel and be placed the entire width of a ramp, blended transition, or turning space.
  14. These plans shall be used in conjunction with Town of Plainfield standards, refer to sheet 1 of 29 for "directions for use."
  15. Refer to general note #11 on sheet 1 of 29 of the Town of Plainfield standards for "Land Disturbance Mitigation Options".

**LEGEND**  
**EXISTING**

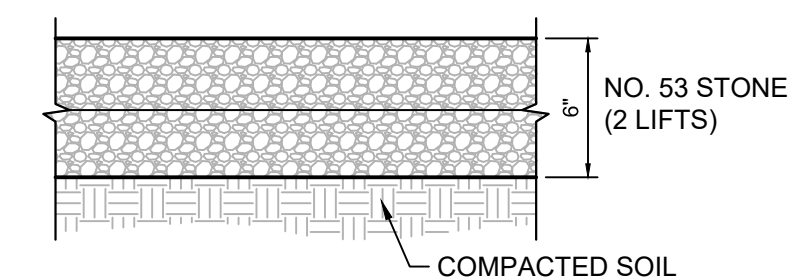
- |   |                              |
|---|------------------------------|
|    | OVERHEAD UTILITY LINE        |
|    | WATER LINE                   |
|  | GAS LINE                     |
|  | UNDERGROUND ELECTRIC LINE    |
|  | UNDERGROUND FIBER OPTIC LINE |
|  | STORM SEWER LINE             |
|  | SANITARY SEWER LINE          |
|  | FENCE                        |
|  | STORM SEWER STRUCTURES       |
|  | SANITARY SEWER STRUCTURES    |
|  | UTILITY POLE                 |
|  | LIGHT POLE                   |
|  | UTILITY VAULT                |
|  | WATER METER                  |
|  | ELECTRIC TRANSFORMER         |
|  | FIRE HYDRANT                 |
|  | SEWER CLEAN-OUT              |
|  | SIGN                         |
|  | TREE                         |

**PROPOSED**

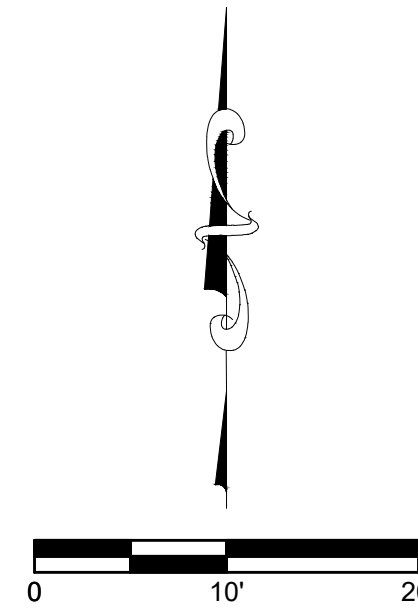
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**CONCRETE PAVEMENT**  
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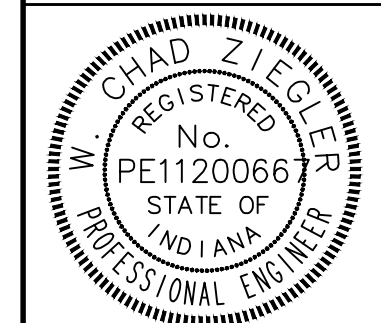


**GRAVEL SECTION**  
NO SCALE



Designed	Sym.	Revisions	Date
SH/RL			
Drawn:	SH		
Checked:	RL		
Scale:	1"=10'		
Date:	05-06-2025		

SITE DEVELOPMENT PLAN  
BRENTWOOD & CLARKS CREEK  
ELEMENTARY - STORAGE BUILDINGS  
PLAINFIELD, INDIANA



W. Chad Ziegler



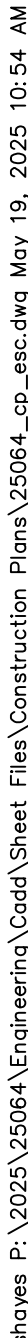
Project No:	25064
-------------	-------

Sheet No:

C111







Project No: 25064

Sheet No:

C400







# PLAINFIELD, INDIANA

## TOWN STANDARDS

### DIRECTIONS FOR USE

- 1.) Details, Notes, Or Specifications Prepared By Outside Sources Shall Not Be Included In The Construction Drawings When Said Details Cover Work Which Is Covered By Town Standards.
- 2.) Individual Town Standards That Do Not Apply May Be Crossed-Out By Design Engineer Through The Placement Of A Single Large X Over The Detail. Minor Reference Notations May Be Placed Adjacent To Individual Standard Titles For Coordination However, The Standards Themselves Shall Not Be Modified In Any Way.
- 3.) Details Prepared By Outside Sources Covering Work Which Is Not Covered By Town Standards Are The Sole Responsibility Of The Design Engineer And Shall Be Placed On Sheets Other Than The Town Standards Sheets.
- 4.) Failure To Properly Execute The Above Directions For Use Will Not Effect The Applicability Nor The Enforcement Of The Town Standards.
- 5.) Town Of Plainfield Shall Be Contacted When Required By Calling 317-839-3490.

### GENERAL NOTES

- 1.) Contractor Shall Verify The Exact Location Of All Existing Utilities At Least 24 Hours Prior To Any Construction Or Excavation. During Construction, All Utilities Shall Be Adequately Supported To Minimize Damage. The Contractor Shall Be Responsible For Repairing Or Replacing Damaged Utilities To The Satisfaction Of The Town Of Plainfield And The Owner Of The Affected Utility.
- 2.) Installation Of Or Provisions For The Installation Of All Underground Utilities (Including Service Laterals) To Be Placed Under Pavement Areas Shall Be Established Prior To The Construction Of The Pavement. The Town Reserves The Right To Require Trenchless Construction For Crossing Of Existing Streets.
- 3.) All Benchmarks And Elevations Shall Be U.S.C. & G.S. Datum.
- 4.) Wherever Proprietary Equipment Is Specified, All Proposals For Substitution Shall Be Submitted In Writing To The Plainfield DPW And Shall Be Subject To The Findings Of The Plainfield DPW.
- 5.) Whenever A Non-Parallel Trench Opening Encroaches Within 5' Of An Existing Street Or Whenever Centerline Of Water Main Is Within 3' Of An Existing Street, Flowable Fill Shall Be Used For Trench Backfill.
- 6.) Except For Water Main Construction, Whenever A Non-Parallel Trench Opening Encroaches Within 5' Of A Proposed Street, Private Drive Or Sidewalk, Granular Backfill If Testing Confirms Compaction. Coarse Aggregate No. 8, Or Flowable Fill Shall Be Used For Trench Backfill.
- 7.) For Water Main Parallel With Adjacent Pavement And Having A Centerline Of Pipe At Least 3' Behind Back-Of-Curb, Approved Backfill Material May Be Used For Trench Backfill. Whenever Centerline Of Water Main Encroaches Within 3' Of A Proposed Street, Private Drive Or Sidewalk, Coarse Aggregate No. 8, Or Flowable Fill Shall Be Used For Trench Backfill.
- 8.) Approved Excavated Material May Be Used For Backfill Outside Of Limits Specified Herein And Under Proposed Sidewalks Provided Sidewalks Are Constructed 6 Months After Backfilling Of Trench. In Order For Excavated Material To Be Approved For Backfill It Shall Be Free Of Organic Material, Rocks Larger Than 6 Inches, Frozen Material, Debris, Excessive Water, Or Other Unsuitable Material As Determined By Plainfield DPW.
- 9.) Black Foundry Sand Is NOT Approved For Use In The Town Of Plainfield.
- 10.) Whenever Granular Backfill Is Placed In A Trench, Contractor Shall Compact Material To A Minimum Of 95% Maximum Dry Density As Per AASHTO T99. The Contractor Shall Demonstrate That Compaction Is Achieved By Means Of In Place Density Tests Performed By An Independent Testing Firm. Testing Frequency Shall Be One Test Per Trench Or 1 Test Per 100 Linear Feet Of Trench, Whichever Is Greater.
- 11.) In Order To Mitigate The Impact Of Land Disturbing Activities On The Public, The Town Currently Allows Two Options. Option 1: Preparation, Implementation, & Maintenance Of A Lime Stabilization Plan For Building Area, Activity Area Adjacent To Building, Access Road(s), & Staging Area Utilizing A Minimum Of 4" Of Compacted Aggregate No. 53 Over A Minimum Of 8" Thickness Lime Subgrade Treatment. Option 2: Preparation, Implementation, & Maintenance Of A Sufficient Washbay Area. If Option 2 Is Deemed Insufficient At The Sole Discretion Of The Town Engineer, Option 1 Will Need To Be Implemented Prior To Any Other Construction Activity Proceeding At The Site.
- 12.) The Construction Of New Combined Sewers Within The Town Of Plainfield's Service Area Is Prohibited. New Construction That Is Tributary To An Existing Combined Sewer Shall Be Designed To Minimize Or Delay The Inflow Contribution To The Existing Combined Sewer. Where New Construction Is Served By Existing Combined Sewers, The Inflow/Clear Water Connection To The Existing Combined Sewer Shall Be Made Separate And Distinct From The Sanitary Waste Connection To Facilitate Disconnection Of The Former If A Separate Storm Sewer Subsequently Becomes Available.
- 13.) As-Built Drawings Of All Storm Sewer, Water Main, And Sanitary Sewer Installation Shall Be Submitted To The Town Of Plainfield. As-Built Drawings Shall Be A Red Lined PDF Version Of The Drawing Showing All Changes And Deviations And GIS Shapefiles Showing Coordinates Of All Utility Locations. All Horizontal Coordinates Shall Be In The Horizontal Datum NAD 83 Indiana State Plane West Datum And All Elevations Provided In The As-Built Drawings Shall Be In The Vertical Datum NGVD 1988. GPS Collected Coordinates Shall Depict Actual Horizontal And Vertical Locations Of Utility Assets Such As, But Not Limited To: Manholes, System Valves, Hydrant, Blow-Offs, Air Release Valves, Master Meters, Cleanouts, Risers, Pump Stations/Wet Wells, And BMPs. Contractor Shall Submit As-Built Drawings Within 30 Days Of Successful Completion Of All Testing Requirements.

Town Standards Apply To Public Property & Private Property.

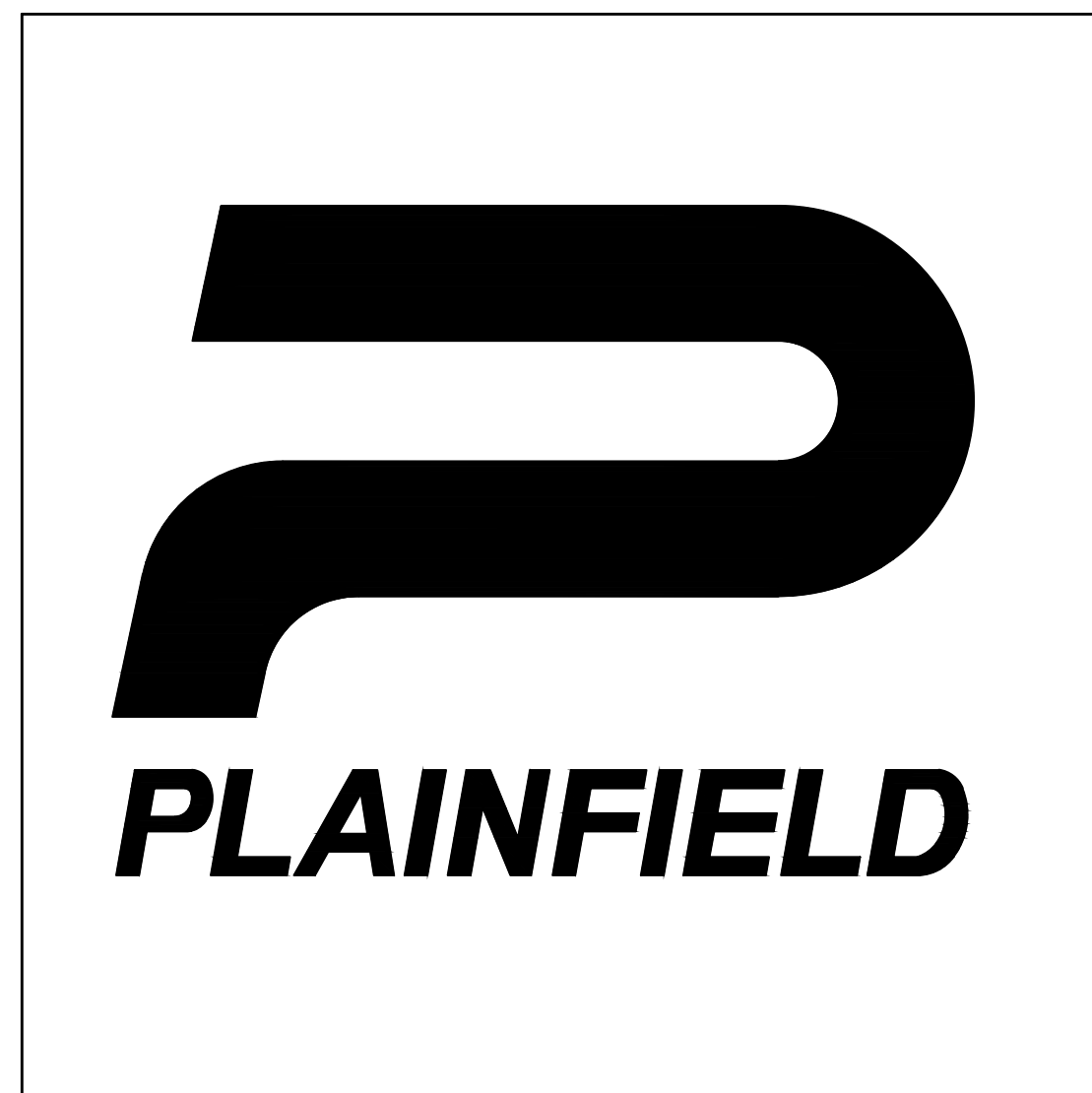
Developed In Accordance With Subdivision Control Ordinance & Zoning Ordinance.

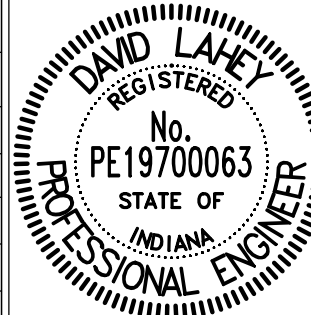


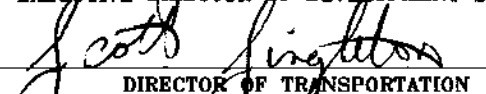
The Entire Set Of Full Size Town Standards Shall Be Attached To The Construction Drawings And Shall Be Considered Part Thereto.

DATE OF CURRENT ISSUANCE: 01/01/2025

### INDEX

SHEET NO.	DESCRIPTION
01	DIRECTIONS FOR USE, GENERAL NOTES
02	RIGHT-OF-WAY, UTILITY EASEMENT & UTILITY LOCATION GUIDELINES
03	PAVEMENT, CURB & SIDEWALK DETAILS & NOTES
04	ROADWAY (R) DEVELOPMENT STANDARDS
05	BUS SHELTER DETAILS AND MISCELLANEOUS DETAILS
06-07	MISCELLANEOUS DETAILS AND NOTES
08	GENERAL (G) DEVELOPMENT STANDARDS
09	STORM SEWER BEDDING DETAILS AND NOTES
10	STORM SEWER DETAILS AND NOTES
11	STORM DRAINAGE (D) DEVELOPMENT STANDARDS
12	WATER MAIN BEDDING DETAILS & NOTES
13	WATER MAIN DETAILS & NOTES
14	WATER (W) DEVELOPMENT STANDARDS
15	SANITARY SEWER BEDDING DETAILS AND NOTES
16	SANITARY SEWER DETAILS AND NOTES
17	SANITARY SEWER LIFT STATION STANDARDS & GUIDELINES
18	SANITARY SEWER (S) DEVELOPMENT STANDARDS
19	SANITARY SEWER (S) DEVELOPMENT STANDARDS
20-23	EROSION CONTROL MEASURES & EROSION CONTROL (E) DEVELOPMENT STANDARDS
24	LOW SPEED URBAN / SUBURBAN ROUNDABOUT DETAIL
25-26	STREET LIGHTING DETAILS
27	TRAFFIC SIGNAL DETAILS
28	WIRELESS DETECTION DETAILS
29	LANDSCAPE DETAILS

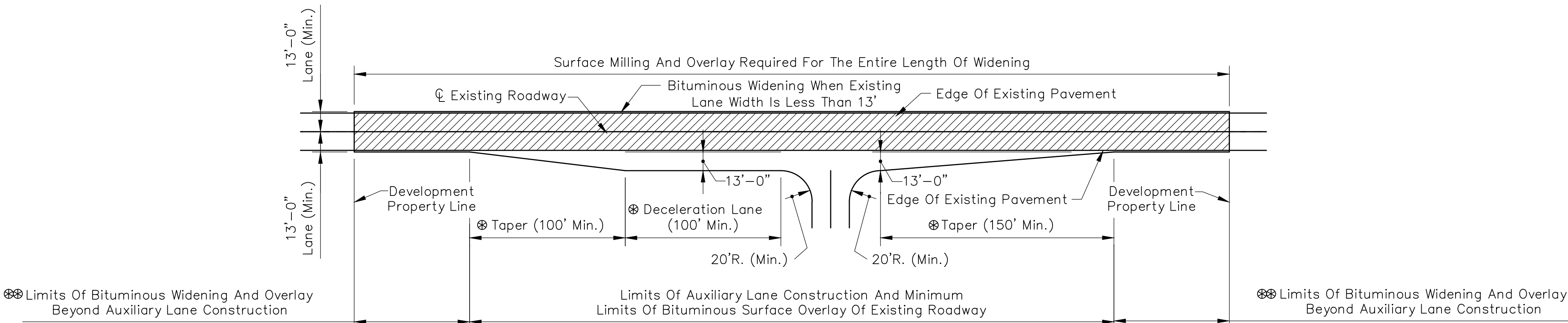


REVISIONS				RECOMMENDED FOR APPROVAL			01/01/2025 DATE	TOWN OF PLAINFIELD	SHEET 01 OF 29					
Rev. No.	Description	Date		APPROVED				DIRECTIONS FOR USE, GENERAL NOTES						
														
														



GENERAL NOTES

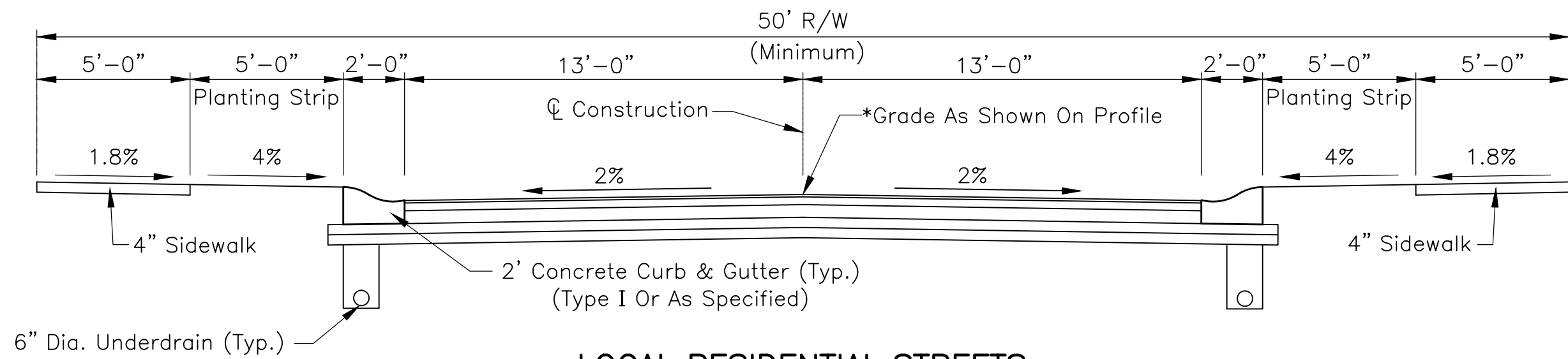
1. The Right-Of-Way Widths, Pavement Widths, And Easement Widths Indicated On This Sheet Are Minimum Distances Required By The Town Of Plainfield. Greater Widths May Be Provided. The Contractor Shall Review The Plat And The Plans To Confirm The Various Widths Indicated On This Sheet And Shall Report Any Discrepancy To The Town Engineer Prior To Proceeding With Construction.
2. The Location Of Proposed Utilities As Indicated Hereon Are Based Upon The Experience Of The Town Of Plainfield And Are So Indicated To Ensure The Orderly Development Of The Land. Strict Adherence To The Indicated Location Is Required. Requests To Change The Location Of The Proposed Utilities Shall Be Submitted In Writing To The Town Engineer And The Director Of Public Works. Utilities Not Meeting These Requirements Shall Be Removed And And Replaced As Directed By The Town Engineer.
3. Arterial Streets And Divided Arterial Streets Are To Be Coordinated With The Town Engineer And Shall Be In Accordance With The Minimum Design Standards Outlined By The Subdivision Control Ordinance.
4. Local Residential Streets Require Only Stop Bars And Crosswalk Marking. Markings Shall Be Thermoplastic In Accordance With The Most Recent INDOT Standard Specification. Refer To Such Drawings Covering Pavement Markings, Street Signs, And Traffic Control Signs. A Plan Of Proposed Pavement Markings Shall Be Submitted To The Plainfield DPW For Approval. For Streets Requiring Resurfacing With Surface Overlay, Mill 8 Feet Wide Along Sides Of Street To A Depth Of 2". Overlay Terminations Shall Also Be Milled 2".
5. Vertical Curves Of A Minimum Length Of 20 Feet Shall Be Provided At All Grade Changes in Accordance with The Town Of Plainfield Subdivision Control Ordinance. For Phased Development, The Vertical Curve Shall Be Constructed To The EVC.
6. Provide A Minimum 0.5% Grade At Curb Flowlines.
7. Selection Of Combination Of Sidepath, Sidewalk, And Planting Strip Widths Shall Be Selected And Approved By The Town Of Plainfield.



- NOTES:
- ⊗ Taper and Deceleration Lane Length Shall Be Designed Based Upon Design Speed Of Existing Roadway
  - ⊗⊗ Bituminous Widening And Overlay Required When Development's Frontage Extends Beyond the Limits Of The Auxiliary Lane Construction

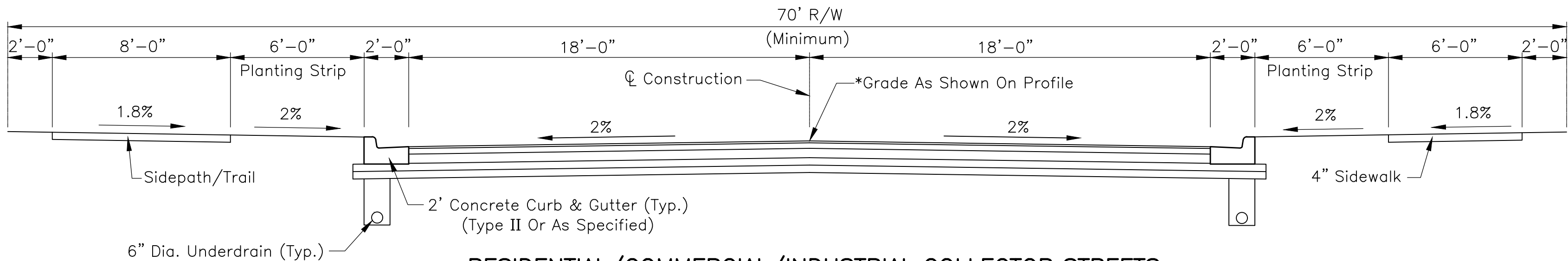
AUXILIARY LANE CONSTRUCTION

Scale: 1/4" = 1'-0"



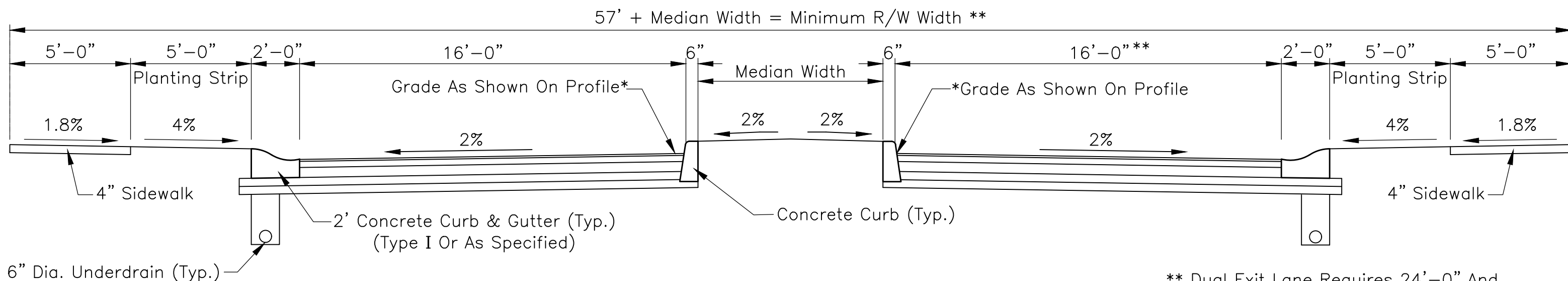
LOCAL RESIDENTIAL STREETS

Scale: 1/4" = 1'-0"



RESIDENTIAL/COMMERCIAL/INDUSTRIAL COLLECTOR STREETS

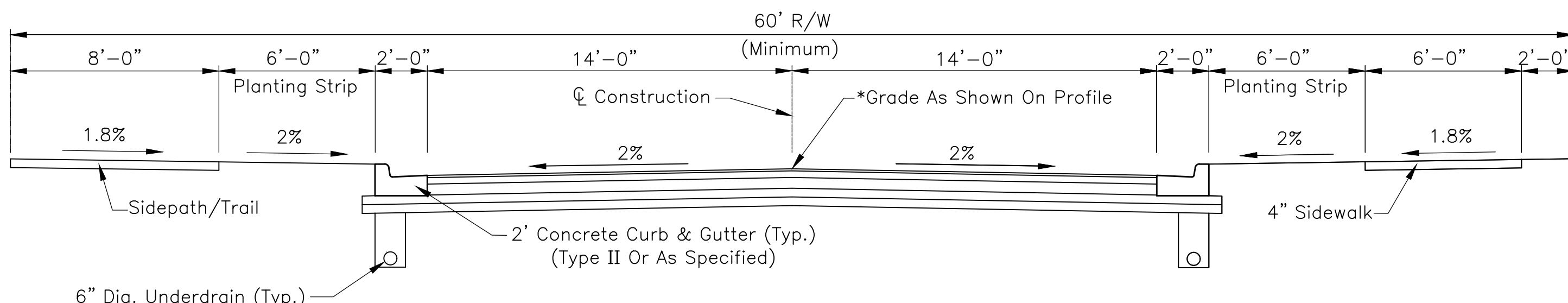
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LOCAL RESIDENTIAL STREETS ENTRY MEDIAN DETAIL

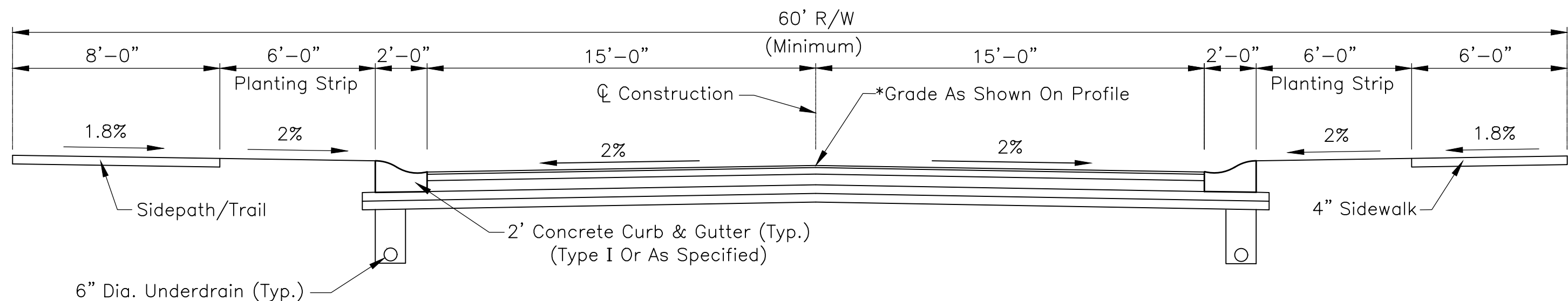
Scale: 1/4" = 1'-0"

\*\* Dual Exit Lane Requires 24'-0" And 8'-0" Additional R/W



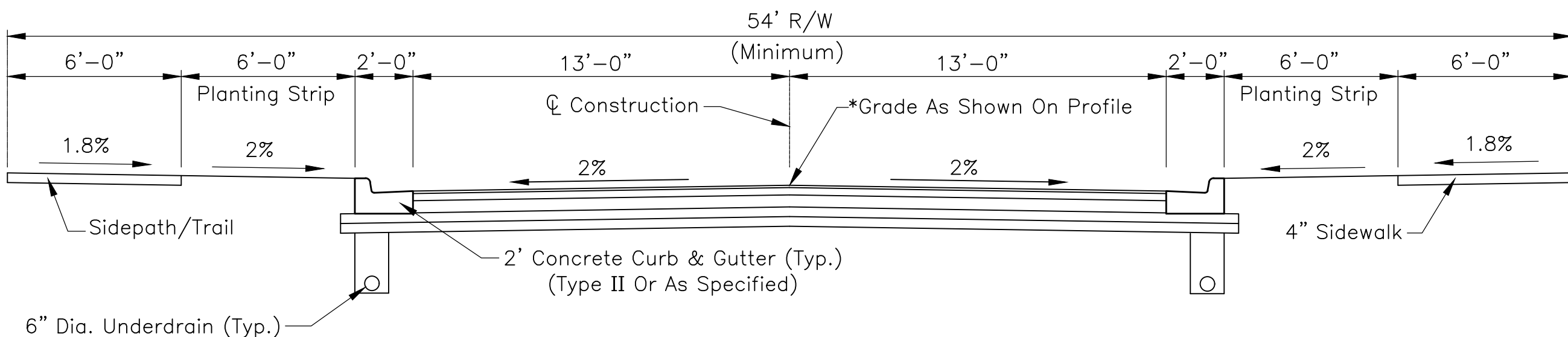
LOCAL INDUSTRIAL STREETS

Scale: 1/4" = 1'-0"



LOCAL RESIDENTIAL COLLECTOR STREETS

Scale: 1/4" = 1'-0"



LOCAL COMMERCIAL STREETS

Scale: 1/4" = 1'-0"

REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Lacey</i>	DESIGN ENGINEER	01/01/2015	DATE
APPROVED	<i>David Lacey</i>	EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES	01/01/2015	DATE
APPROVED	<i>John F. Fingleton</i>	DIRECTOR OF TRANSPORTATION	01/01/2015	DATE

TOWN OF PLAINFIELD
RIGHT-OF-WAY, UTILITY EASEMENT & UTILITY LOCATION GUIDELINES

SHEET
02
OF
29

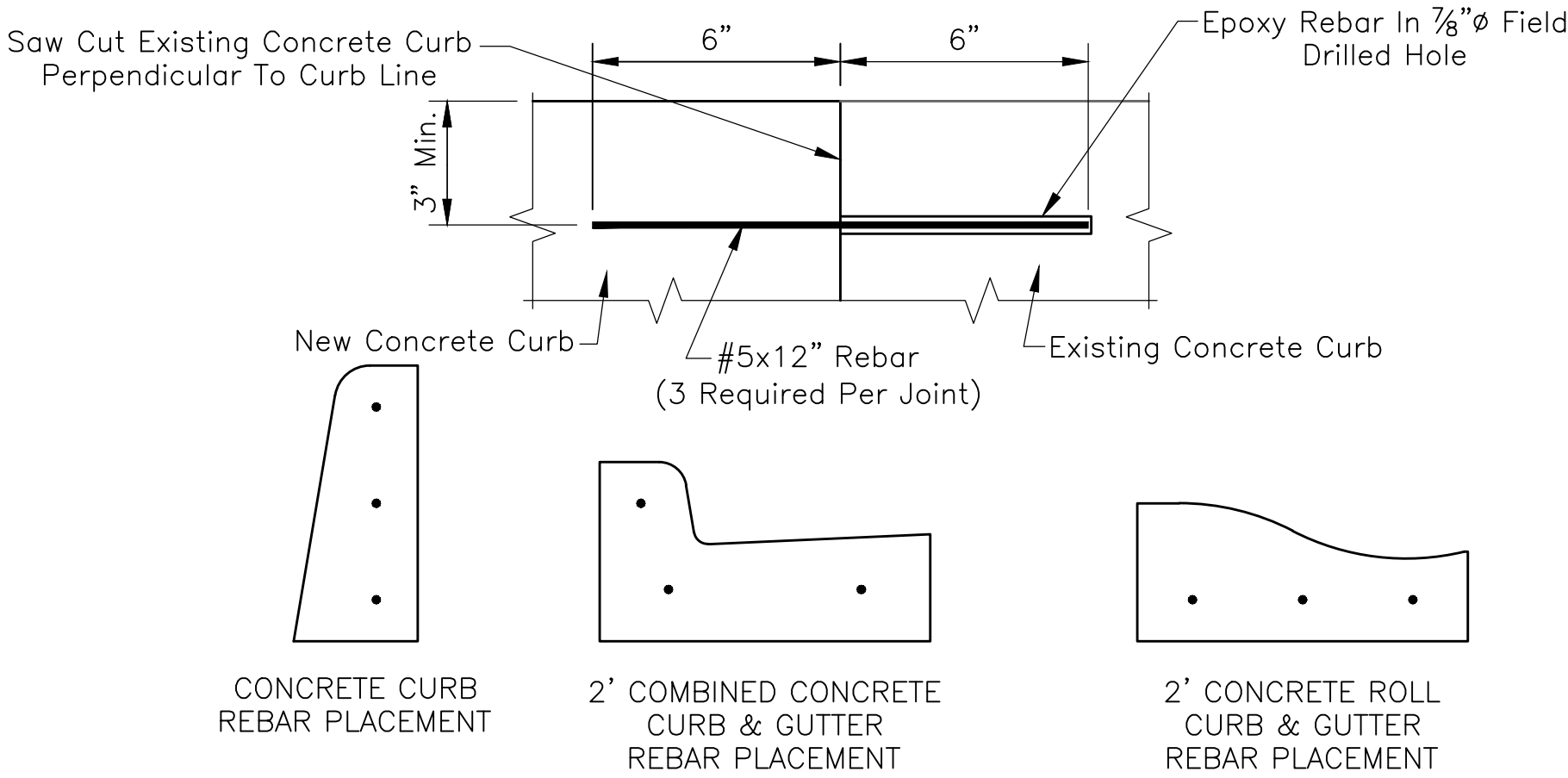


PAVEMENT CONSTRUCTION

- 1.) Subbase And Subgrade Shall Be At Least 100 Percent Of The Maximum Dry Density In Accordance With AASHTO T99. Compaction Testing Shall Be At The Contractor's Expense And Shall Be Performed By An Independent Laboratory. Test Results Shall Be Submitted To The Director Of Public Works Prior To Placing Any Material On The Subbase Subgrade. One In-Place Density Test Shall Be Completed For Each Lift For Every 400 Linear Feet Of Traffic Lanes.
- 2.) For Local Residential Streets With Concrete Pavement, Four Inch Compacted Aggregate No. 53 Is Optional If Adequate Subgrade Is Present. Adequacy Of Subgrade Shall Be Determined Solely By The Town Based On A Contractor Performed Proof-Roll With A Fully Loaded Tri Axle Dump Truck.
- 3.) Hot Poured Joint Adhesive Shall Be Applied To Longitudinal Joints Constructed Between Two Adjacent HMA Surface And Intermediate Courses In Accordance With The Most Recent INDOT Standard Specifications.
- Liquid Asphalt Sealant Shall Be Applied To Longitudinal Joints A Minimum Width Of 24 In., Centered On The Joint Line In Accordance With The Most Recent INDOT Standard Specifications.
- 4.) Wherever Rigid Pavement Is To Be Used, The Contractor Shall Submit A Detailed Paving Plan To The Town Engineer. The Paving Plan Shall Show The Location And Type Of Jointing To Be Used In The Construction. The Location And Type Of Jointing Shall Meet The Requirements Of The Most Recent INDOT Standard Details.
- 5.) Upon Approval Of The Mix Design By The Town Engineer, Chemical Modification Of Soils Per INDOT Standard Specifications Section 215, Shall Be Performed To A Minimum Depth Of 14 Inches. Following Soil Modification, Compaction Shall Be Performed Until The Modified Layer Has A Density Not Less Than 100% Of The Maximum Dry Density Or The Zone Below The Modified Layer Has A Density Not Less Than 95% Of The Maximum Dry Density. Maximum Dry Densities Shall Be Determined In Accordance With AASHTO T99. The Mix Design Shall Be Determined In Accordance With INDOT Design Procedures For Soil Modification Or Stabilization. The Proposed Design And Construction Procedure Shall Be Submitted To The Town Engineer. Unsatisfactory Soil Modifications, As Determined By The Town Engineer, May Require An Increase In Depth Of The Aggregate Base Or Binder. Tensor TriAx Geogrid May Be Used In Lieu Of, Or In Conjunction With, The Chemical Modification Of Soils, As Directed By The Town Engineer. In Conjunction With The Usage Of Tensor TriAx Geogrid, A Modified Pavement Section May Be Provided By The Town Engineer.
- 6.) The Town Engineer May Require Polypropylene (PP) Fiber In The Surface HMA Mixture Where Heavy Truck Traffic Is Expected To Impact Pavement Sections.

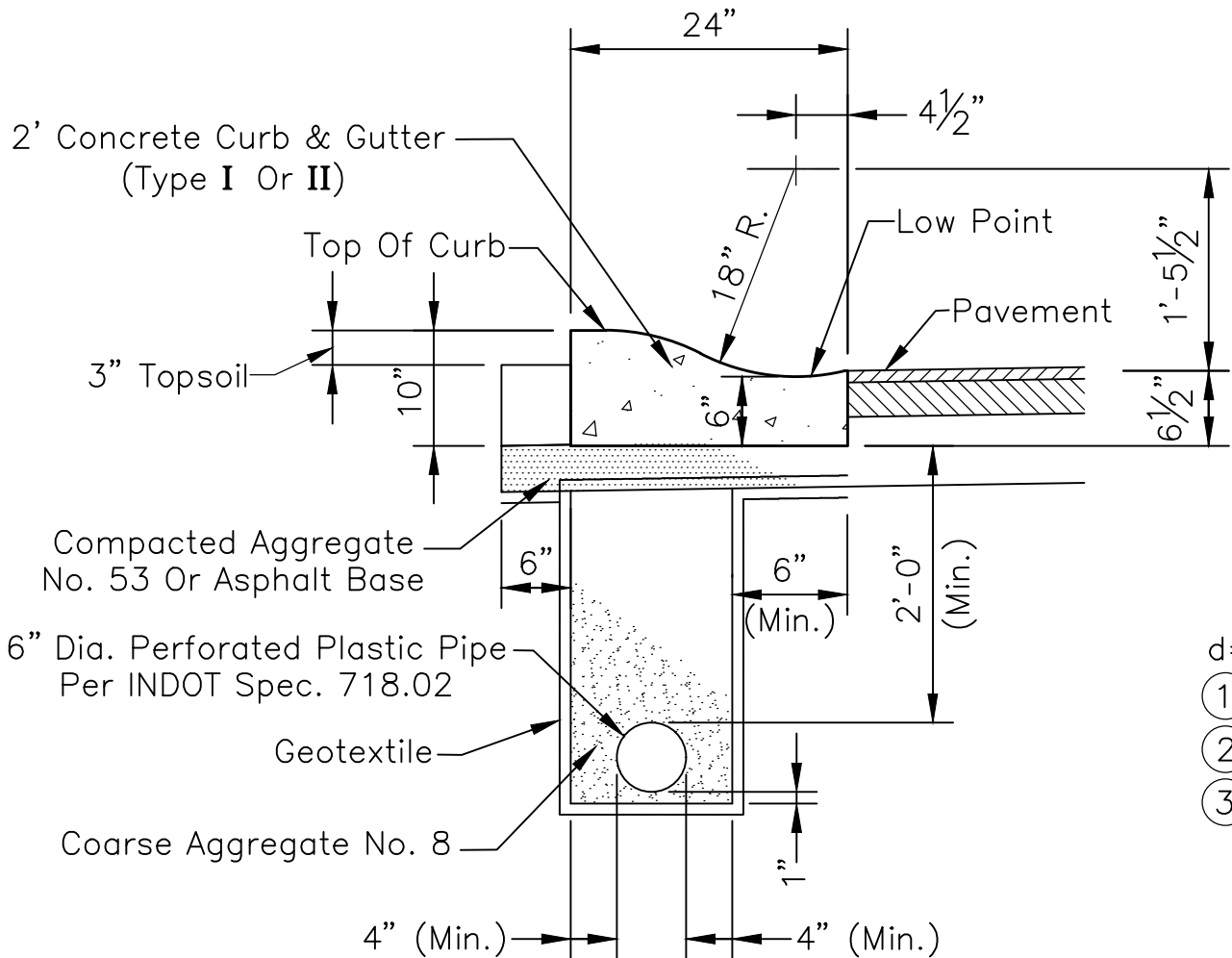
CURB RAMP CONSTRUCTION

- 1.) All Curb Ramps Shall Meet The Requirements Of The Americans With Disabilities Act, The Most Recent INDOT Standard Specifications And The Town Of Plainfield's Most Recent Standards. Curb Swipes Required For Handicap Ramps Shall Be Provided With Initial Curb Construction.
- 2.) Minimum Width Of Curb Ramp Shall Be 4 Feet, Not Including Flares. Maximum Slope Of Ramps Shall Be 8.33% (12:1). Handicap Ramps Are To Be Located As Shown On The Plans, Or As Directed By The Town Engineer Or Director Of Public Works.
- 3.) Type E Ramps Shall Be Provided At The Center Line Of The Radius At All Corners Of Every Street Intersection Where There Is An Existing Or Proposed Sidewalk And Curb. In Case Of "T" Intersection, A Type C Ramp Shall Be Provided Adjacent To Each Corner Ramp. Type C Ramps Also Shall Be Provided At Walk Locations At Mid-Block In Hospital, Medical Center Or Athletic Stadium Vicinities. The Use Of Details Contrary To Those Shown Hereon Shall Require The Prior Written Approval Of The Town Engineer.
- 4.) Surface Texture Of The Ramp Shall Be That Obtained By A Coarse Brooming Transverse To The Slope Of The Ramp.
- 5.) Ramps Shall Be Provided Where The Driveway Curb Extends Across The Sidewalk.
- 6.) Care Shall Be Taken To Assure A Uniform Grade On All Ramps With No Grade Breaks.
- 7.) Drainage Structures Shall Not Be Placed In Line With The Ramps Except Where Existing Drainage Structures Are Being Utilized In The New Construction. Location Of The Ramps Shall Take Precedence Over Location Of Drainage Structures.
- 8.) The Normal Gutter Line Profile Shall Be Maintained Through The Area Of The Ramp.
- 9.) Expansion Joint For The Ramp Shall Be A Maximum 1/2" Wide. The Top Of The Joint Filler For All Ramp Types Shall Be Flush With Adjacent Concrete.
- 10.) Slope Of Ramp May Be Warped When Field Conditions Warrant And When Approved By The Town Engineer Or Director Of Public Works.



CONCRETE CURB REPLACEMENT CONNECTION DETAIL

Not To Scale

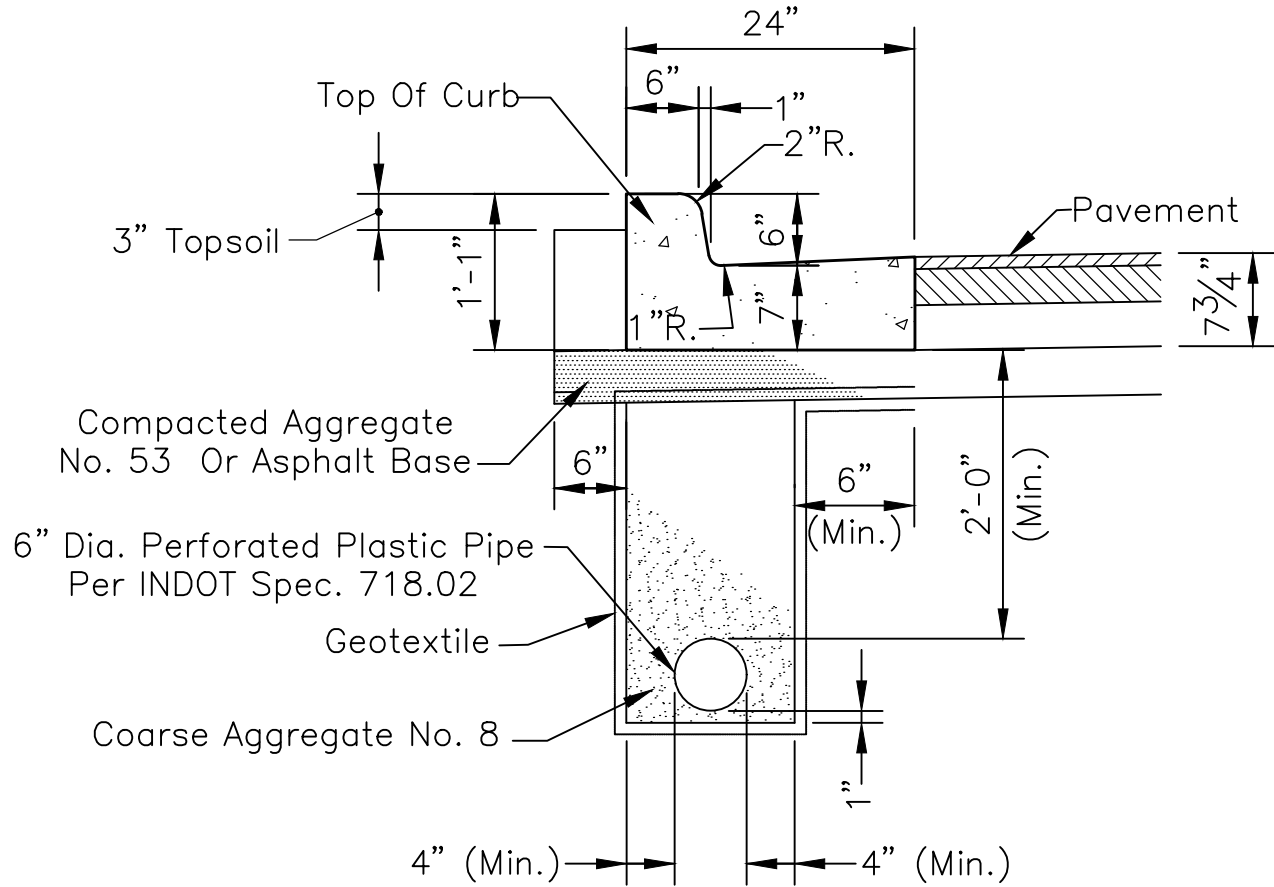


TYPE I

See Development Standards For Depressed Concrete Roll Curb If Desired At A Private Drive That Intersects A Public Road With Type I Curb.

2' CONCRETE ROLL CURB & GUTTER

Scale: 3/4"=1'-0"

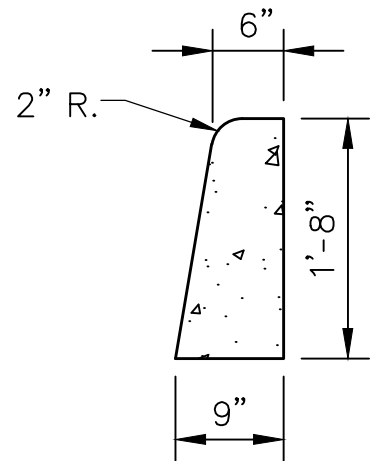


TYPE II

See Development Standards For Reinforced Concrete Gutter Which Is Required At All Private Drives That Intersect A Public Road With Type II Curb Or Similar.

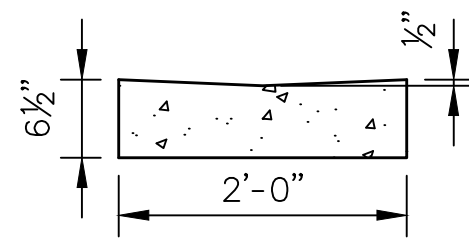
2' COMBINED CONCRETE CURB & GUTTER

Scale: 3/4"=1'-0"



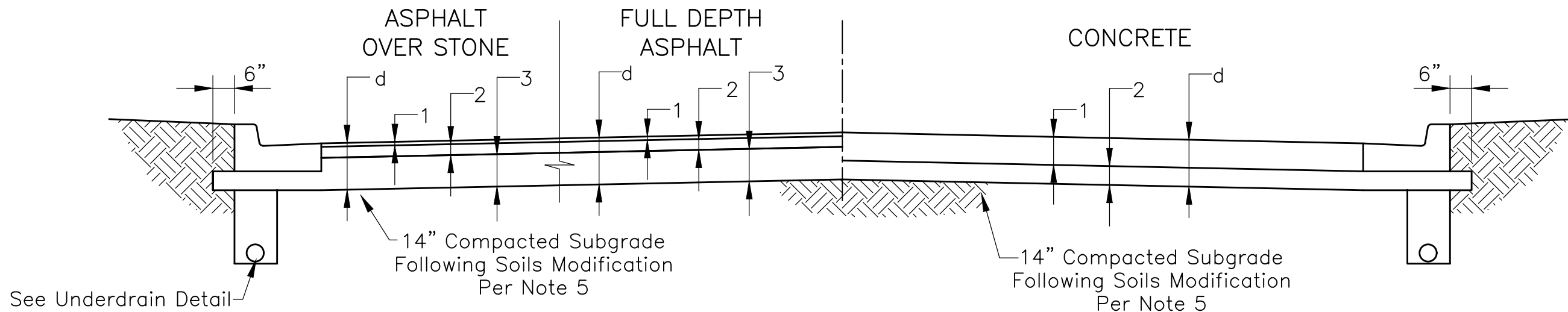
CONCRETE CURB DETAIL

Scale: 3/4"=1'-0"



SPECIAL CURB DETAIL

Scale: 3/4"=1'-0"



LOCAL RESIDENTIAL STREETS

- d=11"
- ① 165 lbs/sys, QC/QA-HMA, 2, PG 58S-28, Surface, 9.5mm
- ② 385 lbs/sys, QC/QA-HMA, 2, PG 58S-28, Inter., 19.0mm
- ③ 6" Compacted Aggregate No. 53 (2 Lifts)

- d=10"
- ① 6", PCCP
- ② 4" Compacted Aggregated No. 53 (See Note 2)

LOCAL RESIDENTIAL COLLECTOR AND LOCAL COMMERCIAL/INDUSTRIAL STREETS

- d=12"
- ① 220 lbs/sys, QC/QA-HMA, 2, PG 58S-28, Surface, 12.5mm
- ② 275 lbs/sys, QC/QA-HMA, 2, PG 58S-28, Inter., 19.0mm
- Over 385 lbs/sys, QC/QA-HMA, 2, PG 58S-28, Base, 25.0mm
- ③ 4" Compacted Aggregate No. 53

- d=10"
- ① 220 lbs/sys, QC/QA-HMA, 2, PG 58S-28, Surface, 12.5mm
- ② 275 lbs/sys, QC/QA-HMA, 2, PG 58S-28, Inter., 19.0mm
- ③ 275 lbs/sys, QC/QA-HMA, 2, PG 58S-28, Base, 19.0mm
- Over 330 lbs/sys, QC/QA-HMA, 2, PG 58S-28, Base, 25.0mm

- d=11"
- ① 7", PCCP
- ② 4" Compacted Aggregated No. 53

RESIDENTIAL/COMMERCIAL/INDUSTRIAL COLLECTOR AND SECONDARY ARTERIAL STREETS

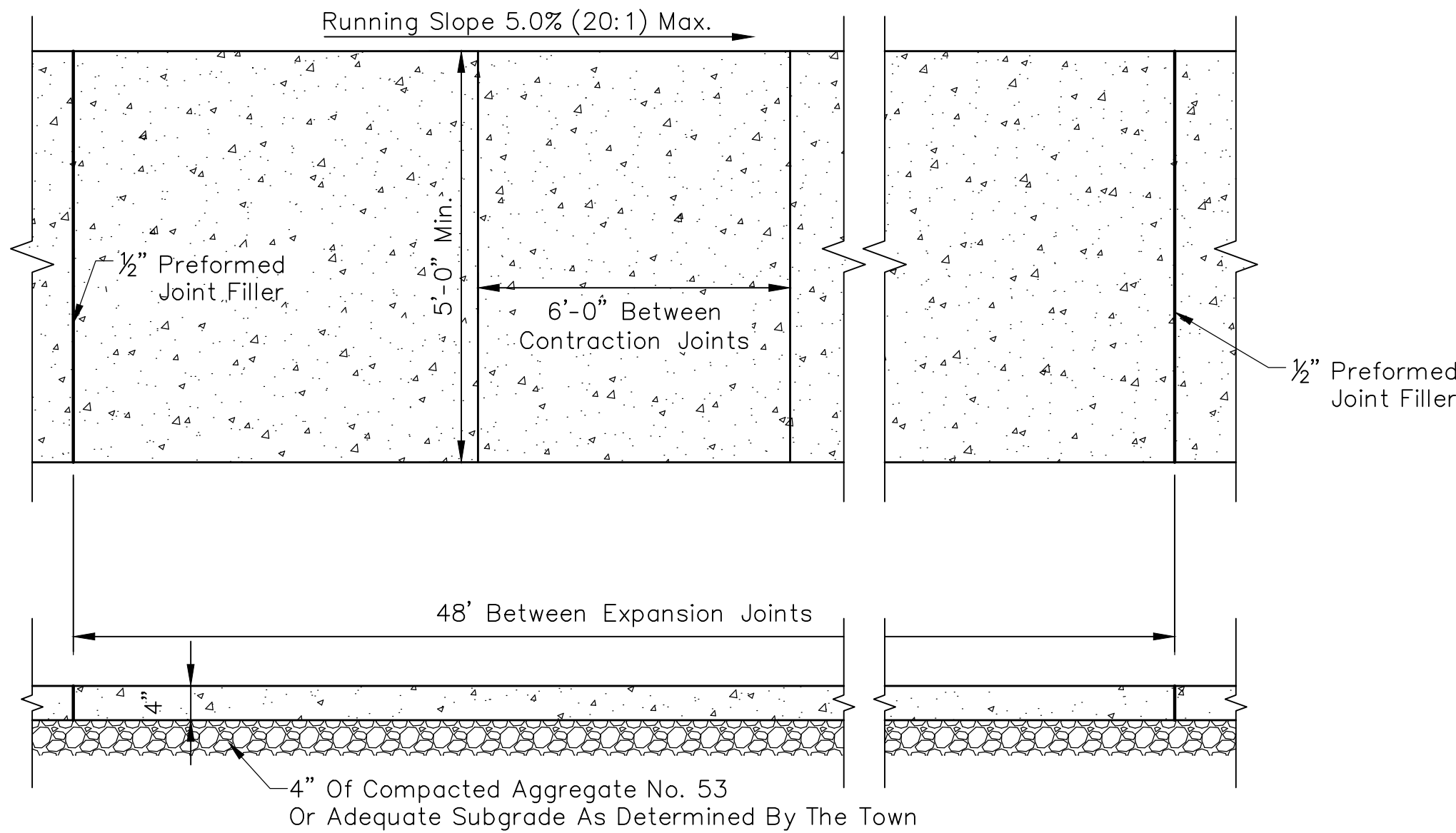
- d=13"
- ① 220 lbs/sys, QC/QA-HMA, 3, PG 58H-28, Surface, 12.5mm
- ② 275 lbs/sys, QC/QA-HMA, 3, PG 58H-28, Inter., 19.0mm
- Over 385 lbs/sys, QC/QA-HMA, 3, PG 58H-28, Base, 25.0mm
- ③ 5" Compacted Aggregate No. 53

- d=12"
- ① 220 lbs/sys, QC/QA-HMA, 3, PG 58H-28, Surface, 12.5mm
- ② 330 lbs/sys, QC/QA-HMA, 3, PG 58H-28, Inter., 19.0mm
- ③ 330 lbs/sys, QC/QA-HMA, 3, PG 58H-28, Base, 25.0mm
- Over 440 lbs/sys, QC/QA-HMA, 3, PG 58H-28, Base, 25.0mm

- d=11 1/2"
- ① 7.5", PCCP
- ② 4" Compacted Aggregated No. 53

PAVEMENT CONSTRUCTION

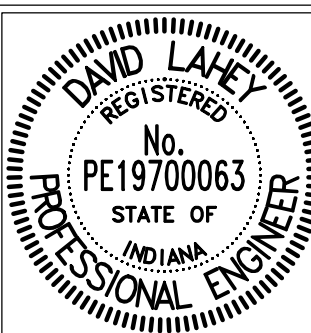
Not To Scale



TYPICAL SIDEWALK DETAIL

Not To Scale

REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Loh</i>	DESIGN ENGINEER	01/01/2015	DATE
APPROVED	<i>David Loh</i>	EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES	01/01/2015	DATE
APPROVED	<i>John J. Loh</i>	DIRECTOR OF TRANSPORTATION	01/01/2015	DATE

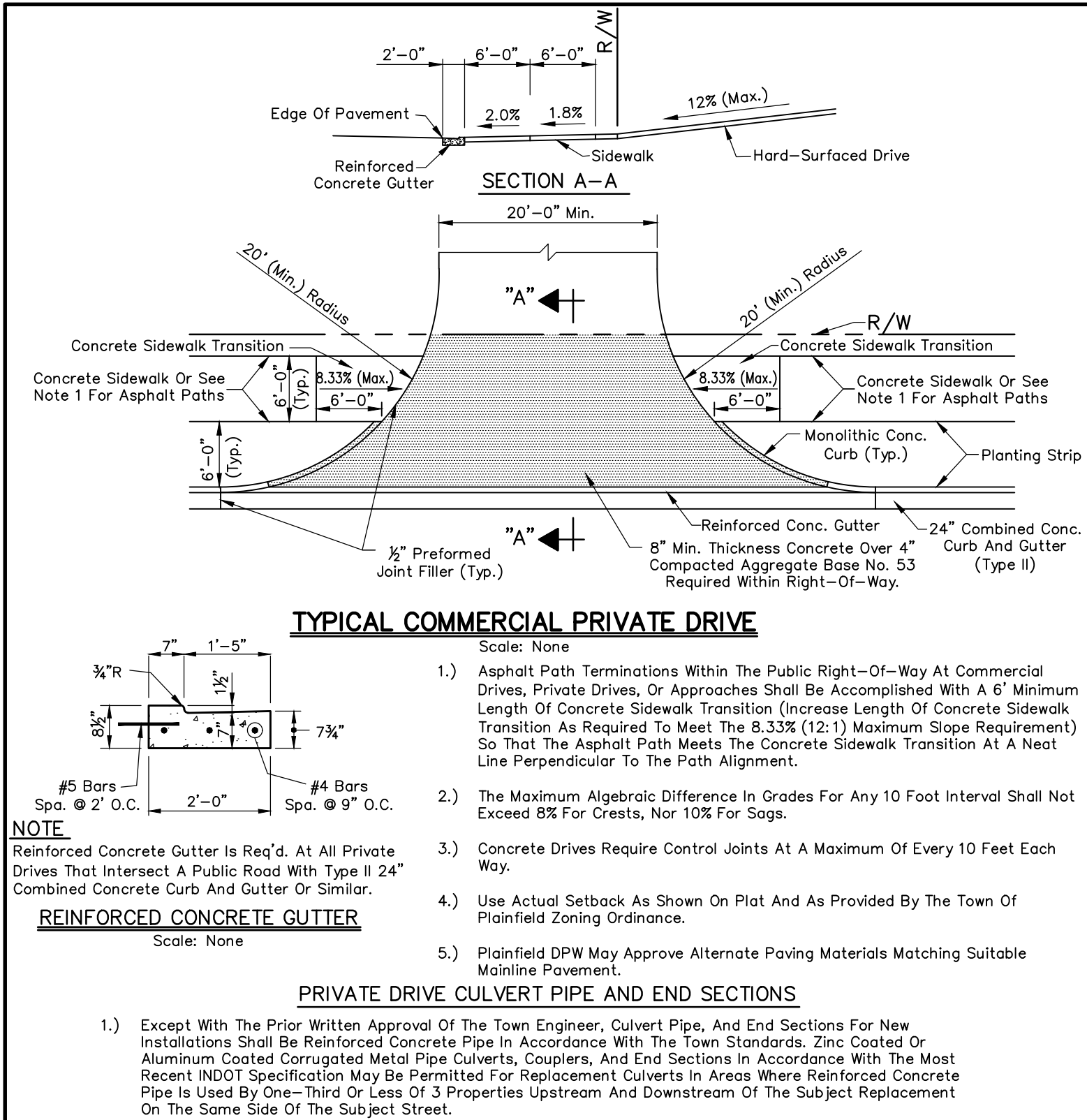
TOWN OF PLAINFIELD

PAVEMENT,  
CURB & SIDEWALK DETAILS &  
NOTES

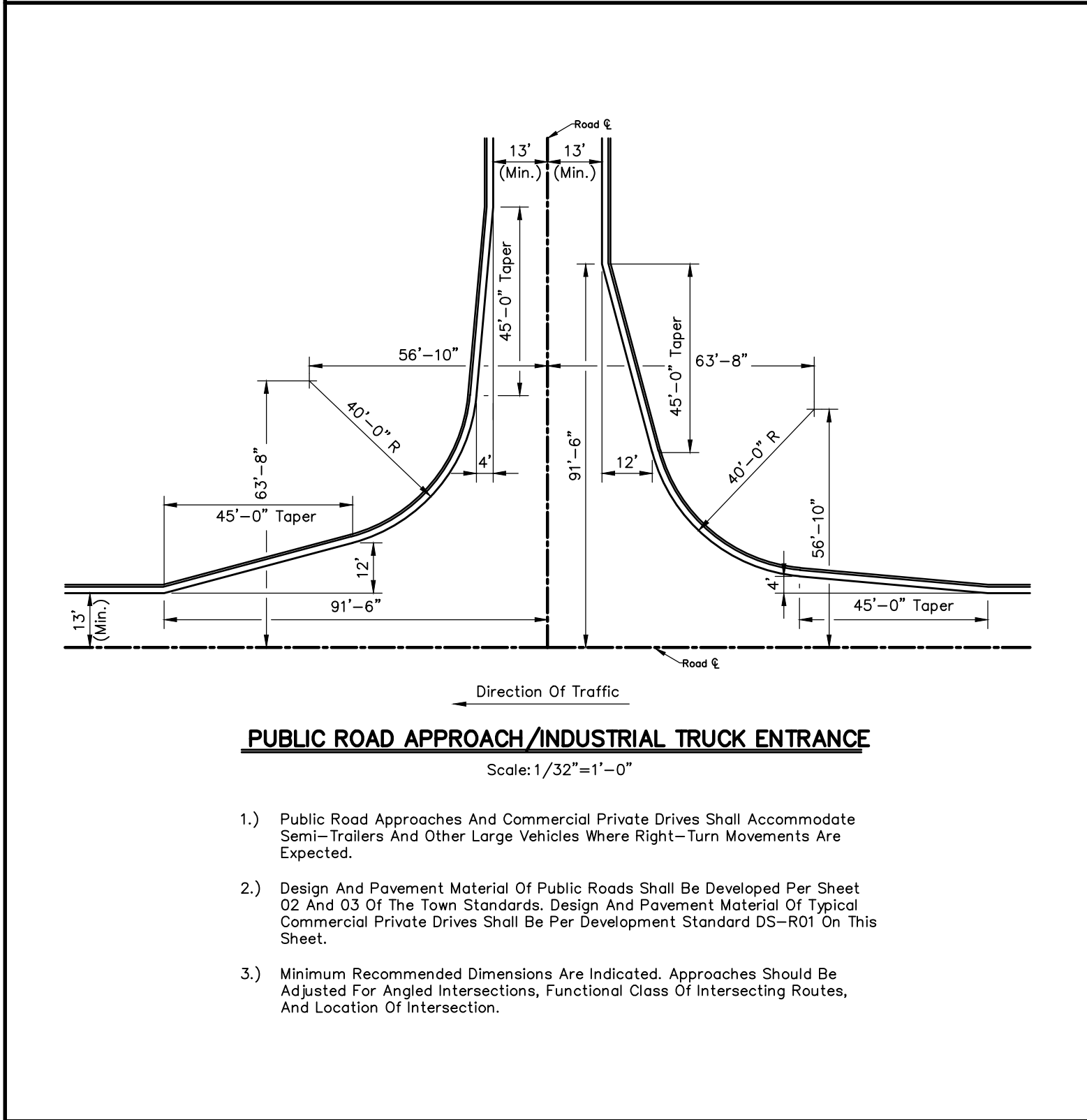
SHEET

03  
OF  
29

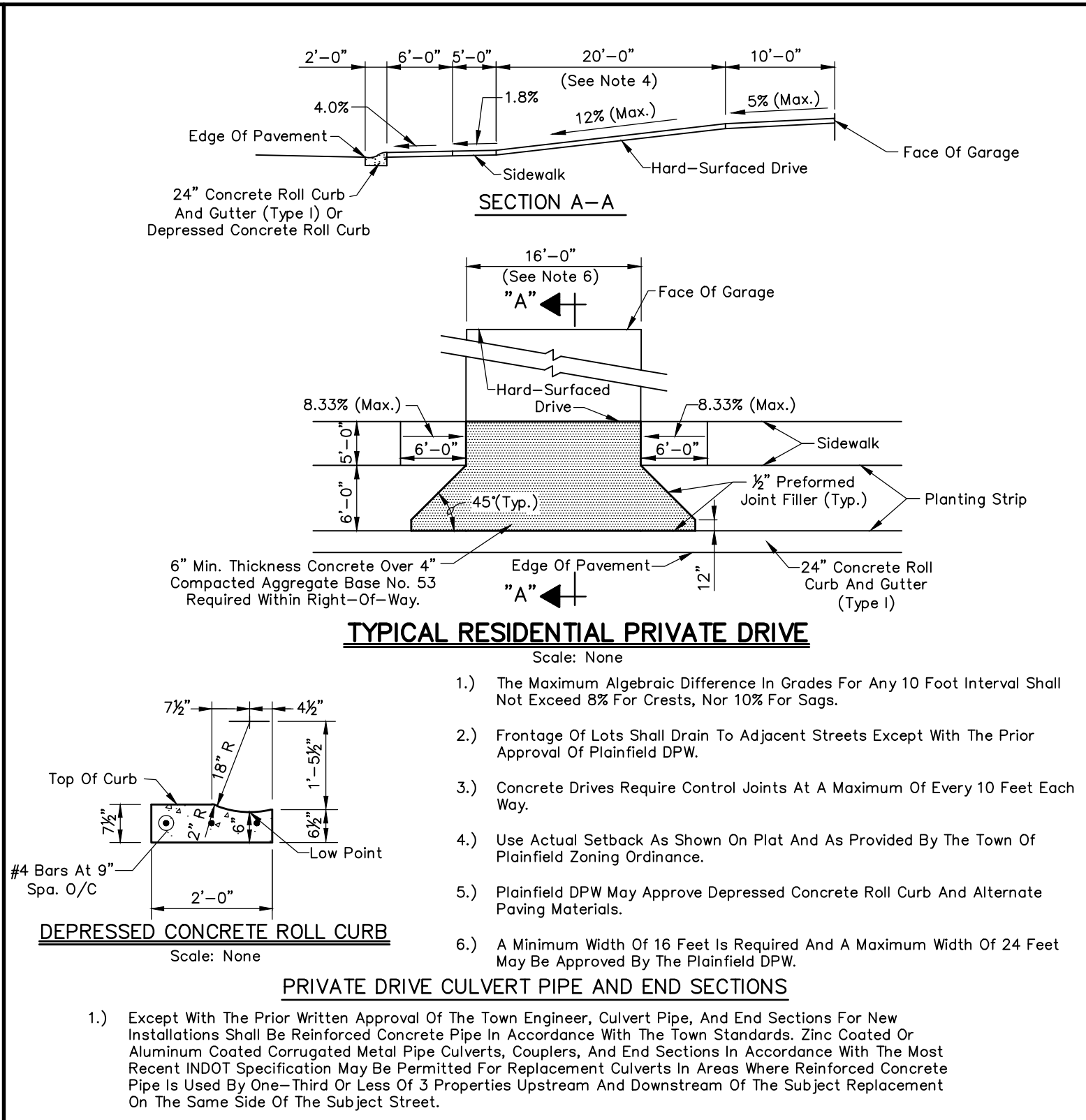




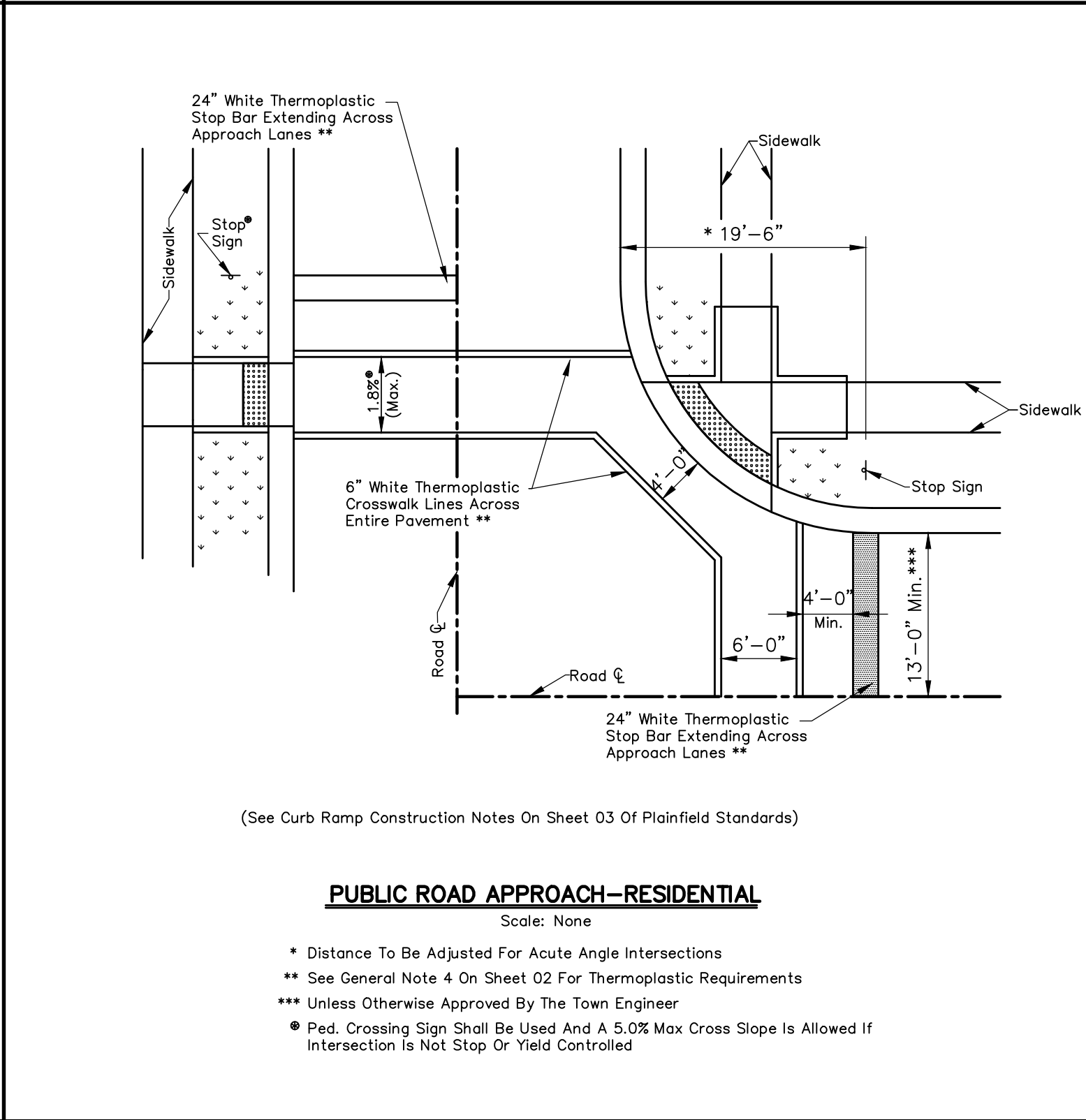
DEVELOPMENT STANDARD - DETAIL DS-R01



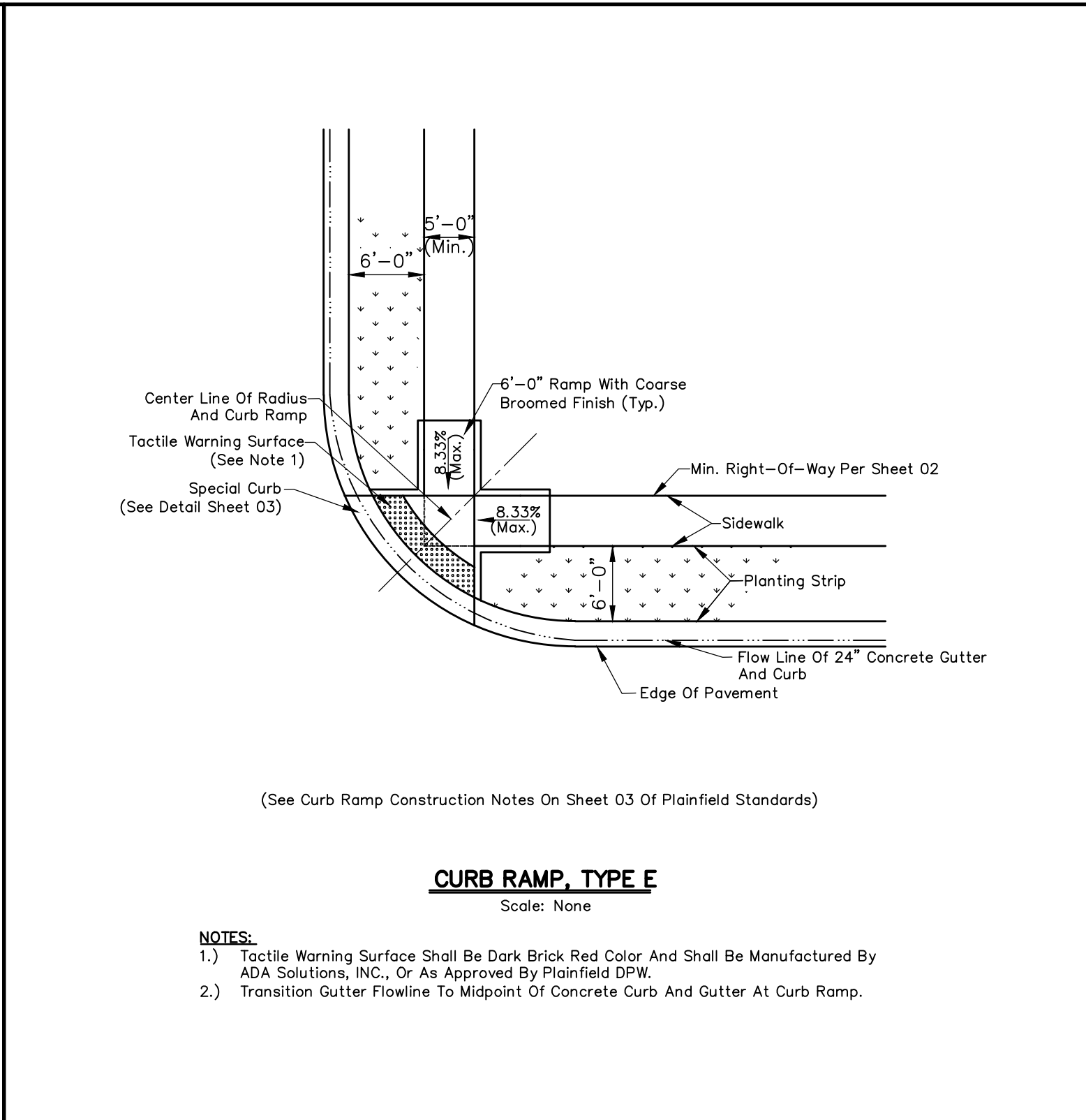
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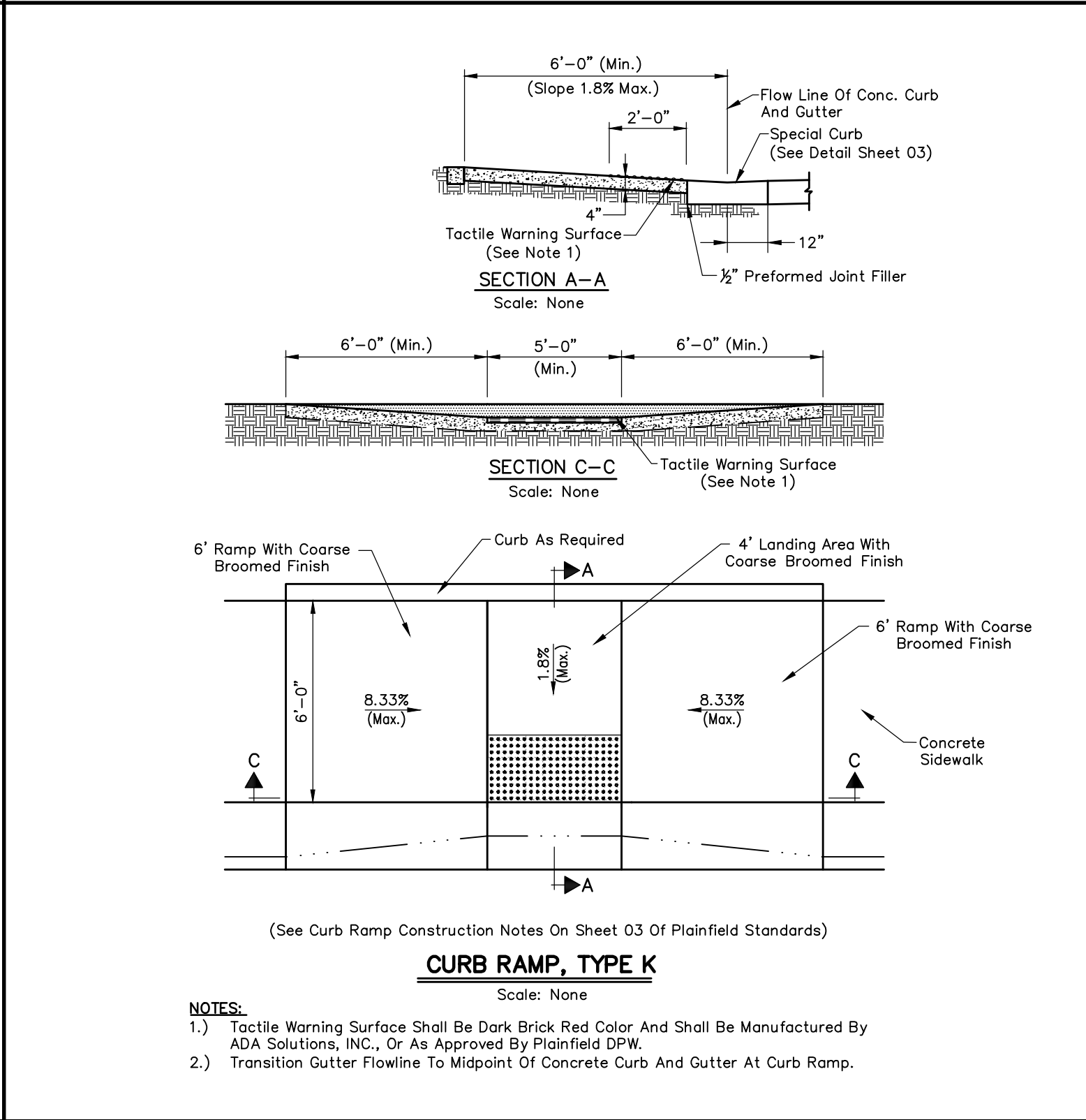
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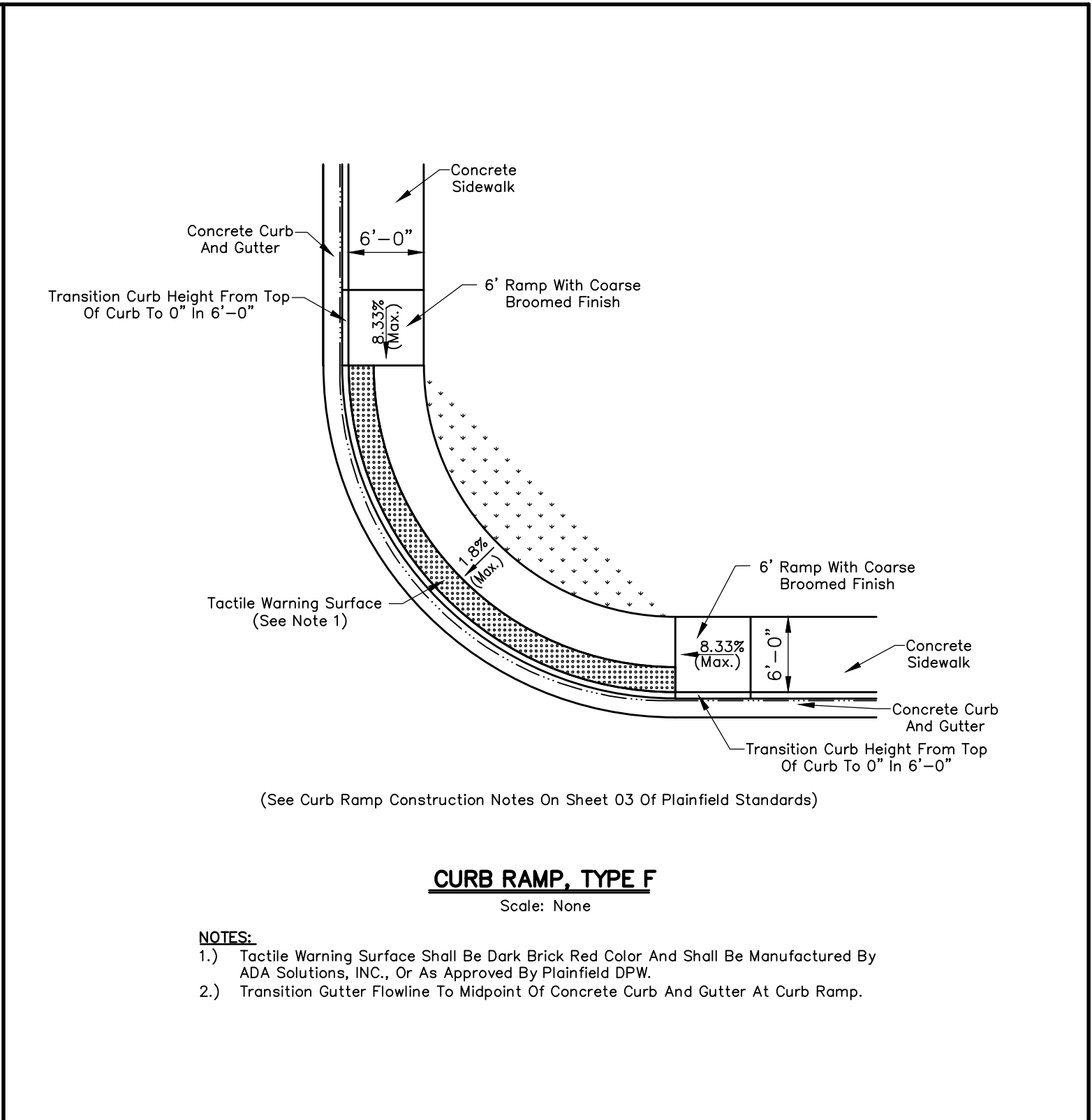
DEVELOPMENT STANDARD - DETAIL DS-R06



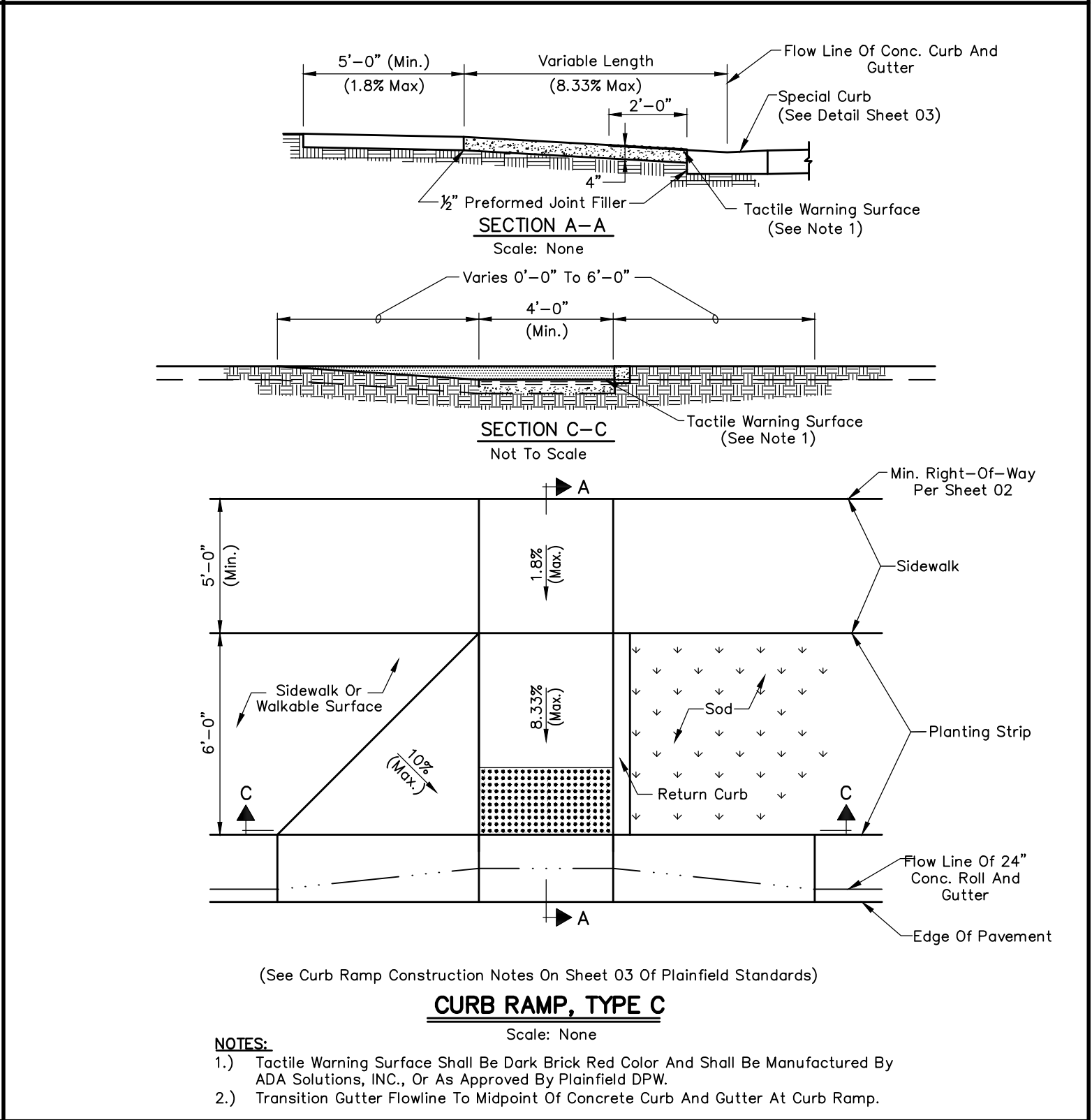
DEVELOPMENT STANDARD - DETAIL DS-R03



DEVELOPMENT STANDARD - DETAIL DS-R07

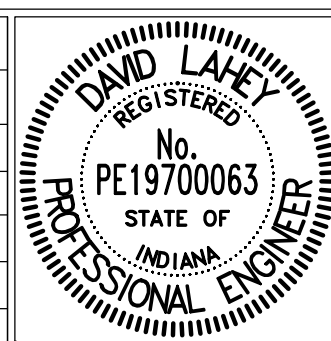


DEVELOPMENT STANDARD - DETAIL DS-R04



DEVELOPMENT STANDARD - DETAIL DS-R08

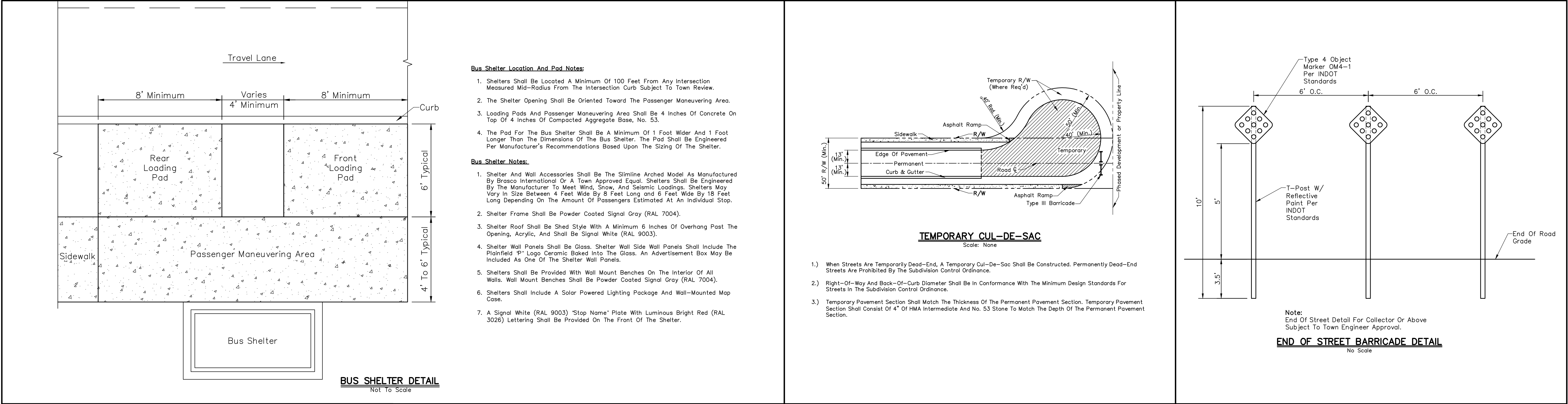
REVISIONS		
Rev. No.	Description	Date



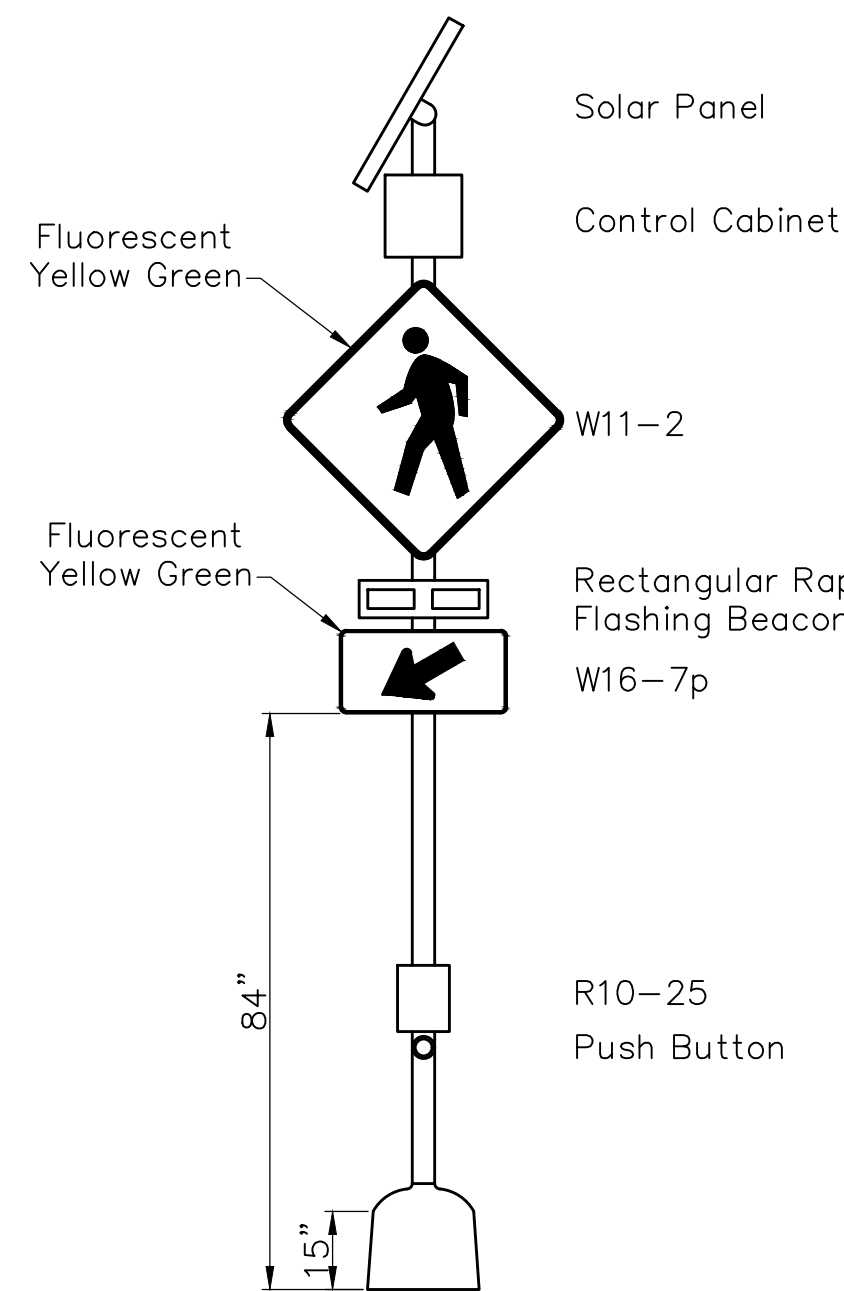
RECOMMENDED FOR APPROVAL	<i>David Loh</i>	DATE	01/01/2015
APPROVED	<i>David Loh</i>	DATE	01/01/2015
APPROVED	<i>John J. J. J.</i>	DATE	01/01/2015

TOWN OF PLAINFIELD	SHEET 04 OF 29
ROADWAY (R) DEVELOPMENT STANDARDS	







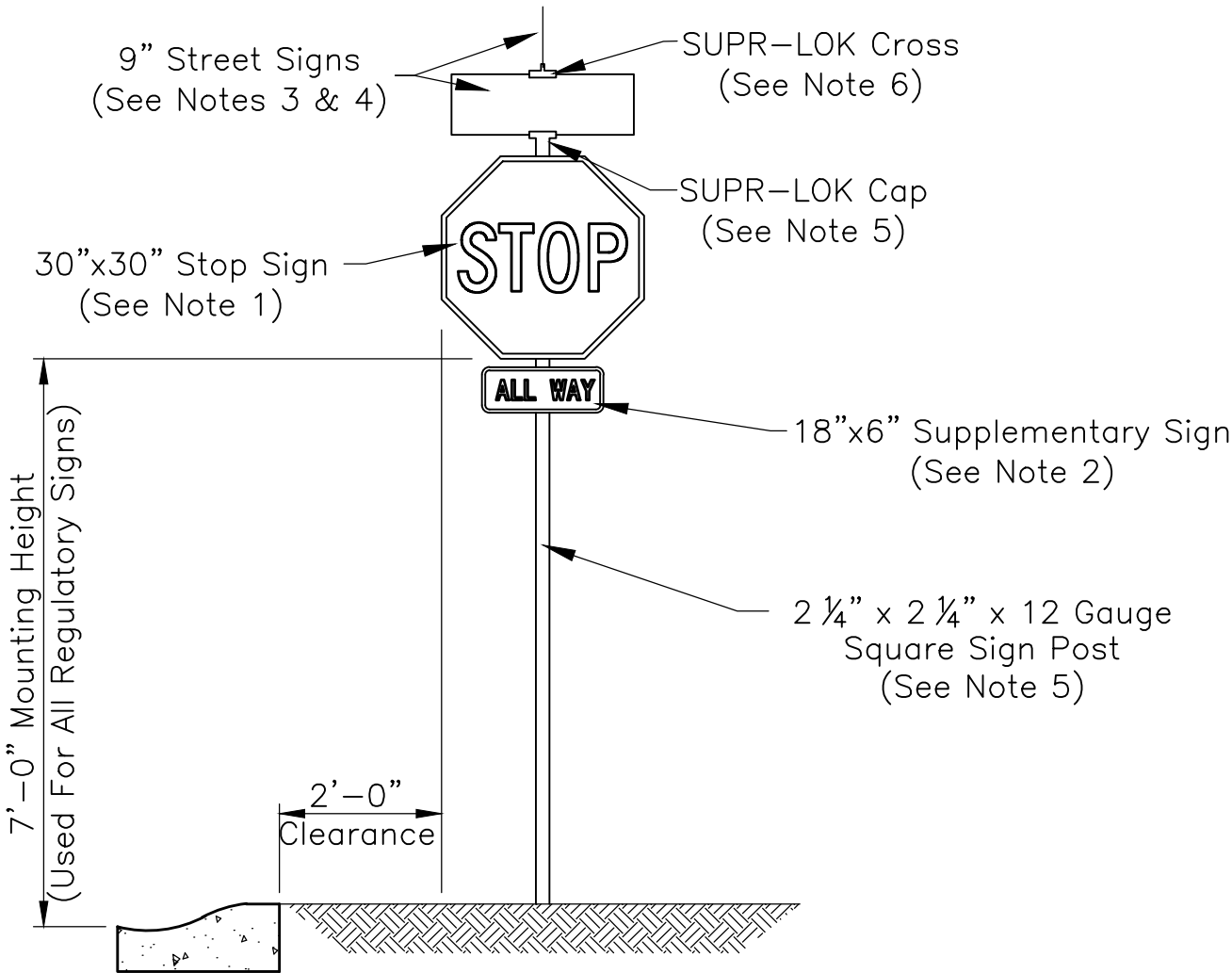


RECTANGULAR RAPID FLASHING BEACON ASSEMBLY

Scale: None

Notes:

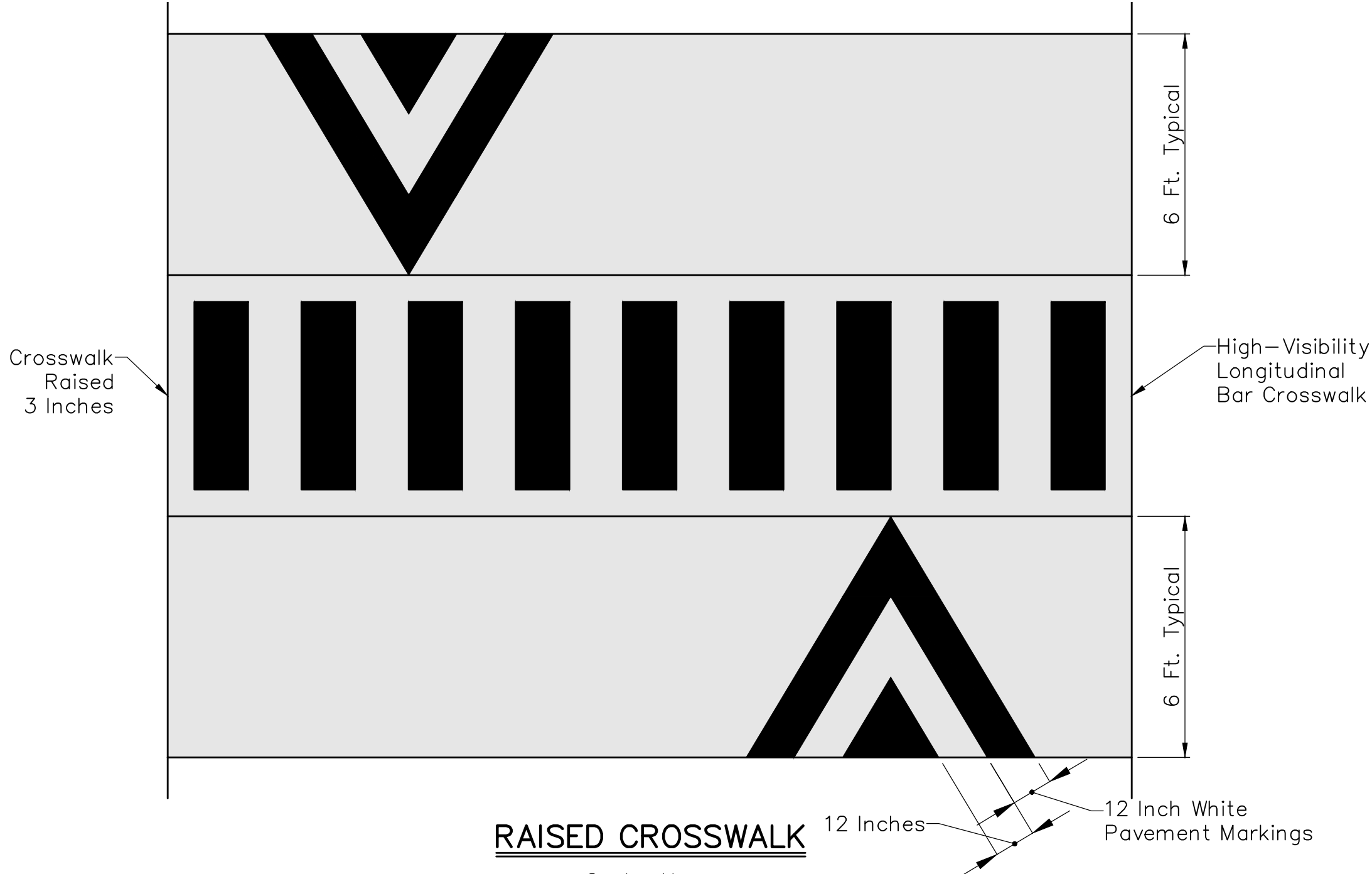
- 1.) Push Button And R10-25 Sign Shall Be Installed On Reverse Side Of Assembly.
- 2.) All Pedestrian Warning Signs Shall Have A Black Legend And Border On A Fluorescent Yellow-Green Background.
- 3.) The Center On The Push Button Shall Be Installed 42" Vertically Above The Accessible Pedestrian All Weather Surface.
- 4.) Push Buttons Shall Be Installed Within A 10" Horizontal Reach Of The Accessible Pedestrian All Weather Surface.
- 5.) Town Of Plainfield May Require Pole Painting Depending On Location.



TYPICAL REGULATORY/WARNING SIGN REQUIREMENTS

Scale: None

- 1.) Stop Sign (R1-1) Shall Be High Intensity And In Accordance With Most Recent Indiana Manual On Uniform Traffic Control Devices. Unless Otherwise Detailed On This Sheet, Other Regulatory Signs Shall Be A Minimum Of 18"x24".
- 2.) A Multi-Way Stop Intersection Requires An "ALL WAY" (R1-3P) Supplementary Sign 18" Wide By 6" Tall In Accordance With Said Manual.
- 3.) Streets Shall Be So Signed At Non-Signalized Intersections With Two Such Street Sign Assemblies Typically Required. Separate 12' Square Sign Post For Street Signs Permitted Only At Signalized Intersections.
- 4.) Street Signs Shall Be 9" Tall Extruded Aluminum (6063-T6) Green Background With White Letters.
- 5.) Regulatory Signs, Other Than Stop Signs, Shall Be Mounted On 12' - 2 1/4" x 2 1/4" x 12 Gauge Square Sign Posts. SUPR-LOK Cap Shall Be Model #97SQX. Regardless If Material For Posts Is Other Than As Shown Hereon, Mounting Height Shall Be 7'-0" From Roadway Edge Of Pavement
- 6.) SUPR-LOK Cross Shall Be Model #990X. For Non-Urban Intersections, Stop Sign To Be Placed A Minimum Of 6' From Cross-Street.
- 7.) For Urban Intersections See Handicap Ramp Detail On Sheet 4 Of The Town Standards.



RAISED CROSSWALK

Scale: None



NOTES:

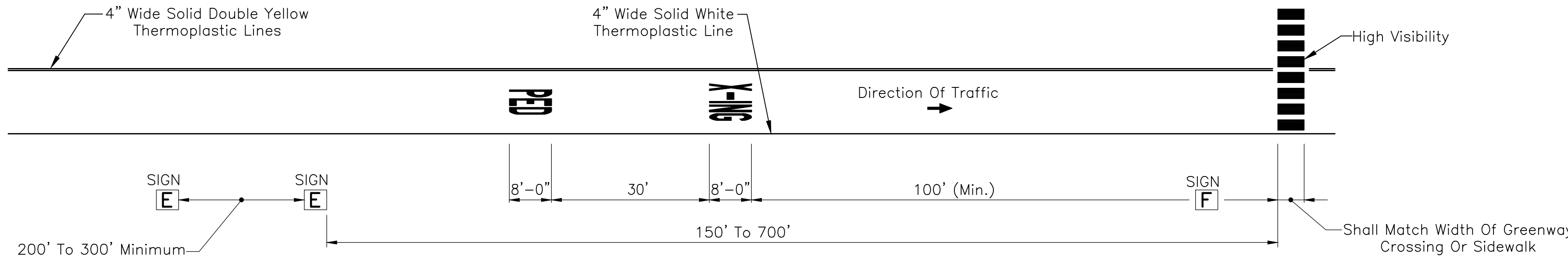
- 1.) \* Mounting Height From Roadway Edge Of Pavement. (Typ. 2.)
- 2.) All Black Lettering Is Scotchcal 7725 (Or Equal)

REGULATORY/WARNING SIGN DETAILS

Scale: None

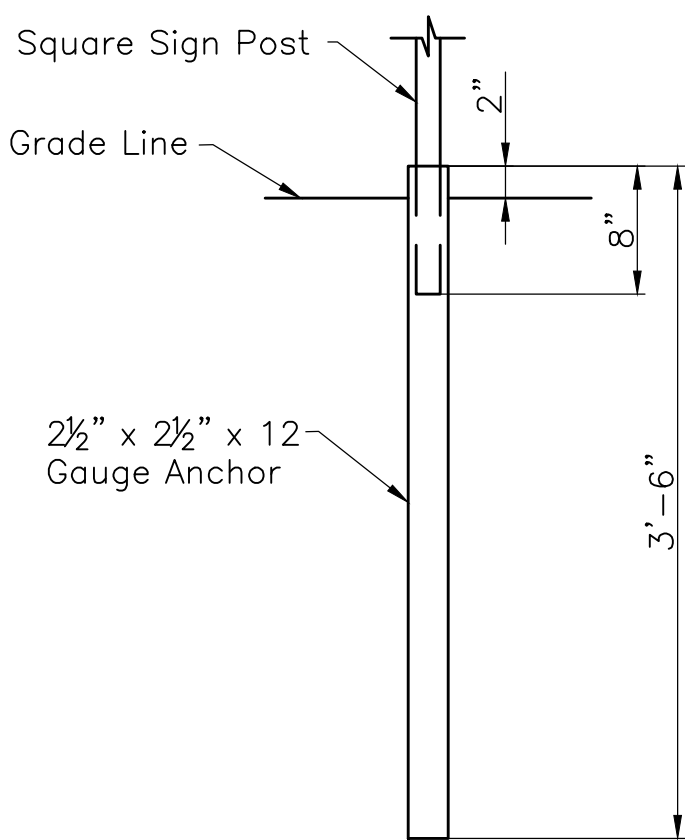
GENERAL NOTES:

- 1.) All Regulatory Signs Shall Be High Intensity And In Accordance With The Indiana Manual On Uniform Traffic Control Devices, Most Recent Edition.
- 2.) All Pavement Markings Shall Be White Thermoplastic And Span Across Approach Lanes.



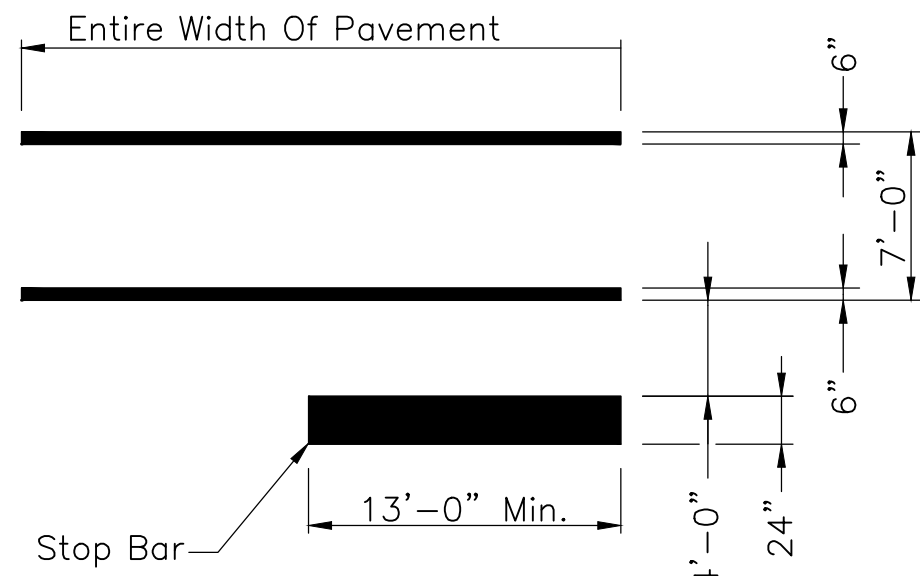
PEDESTRIAN CROSSING APPROACH DETAIL - COLLECTOR OR ABOVE

Not To Scale



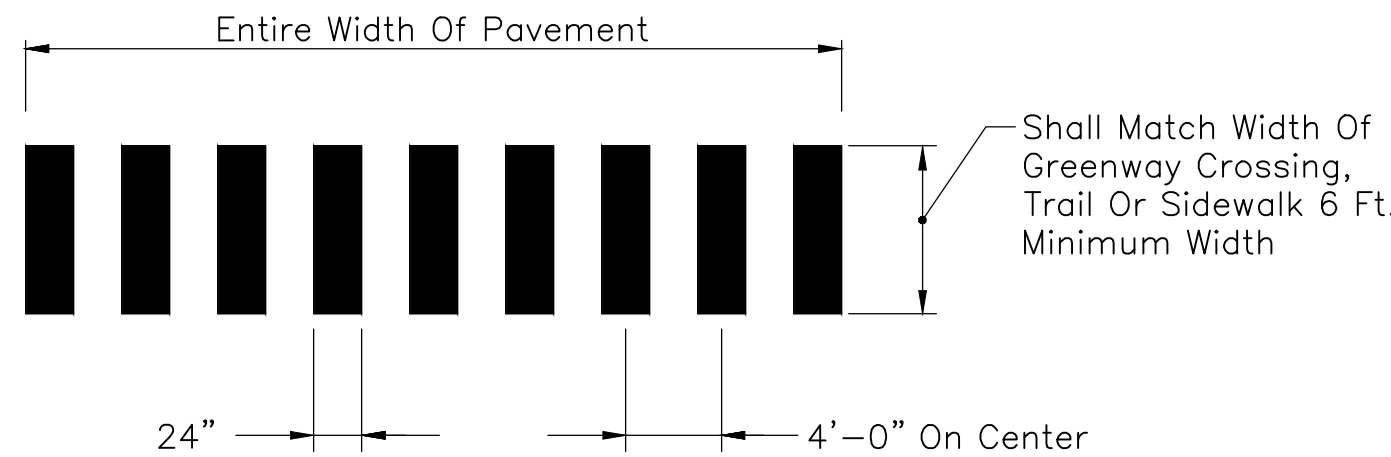
UNREINFORCED ANCHOR BASE

Scale: None



INTERSECTION CROSSWALK DETAIL

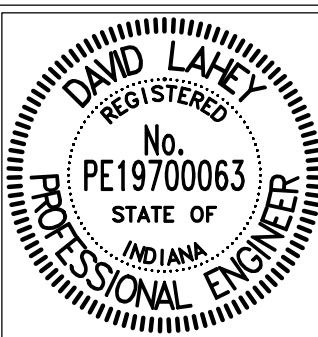
Scale: 1/8" = 1'-0"



HIGH-VISIBILITY LONGITUDINAL BAR CROSSWALK DETAIL

Scale: 1/8" = 1'-0"

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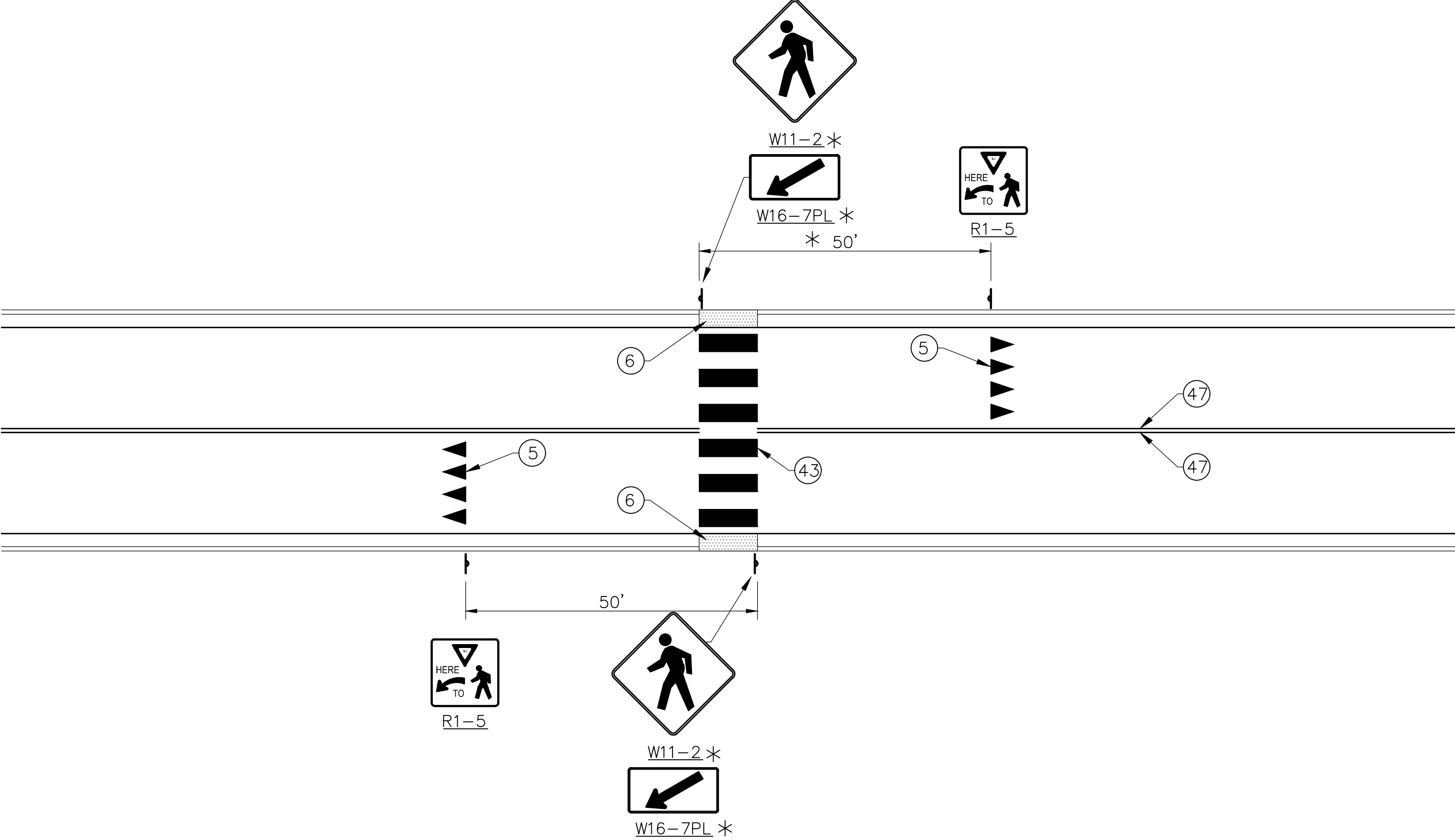


RECOMMENDED FOR APPROVAL	<i>David Loh</i>	01/01/2015
DESIGN ENGINEER		DATE
APPROVED	<i>David Loh</i>	01/01/2015
EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES		DATE
APPROVED	<i>John F. Hightower</i>	01/01/2015
DIRECTOR OF TRANSPORTATION		DATE

TOWN OF PLAINFIELD
MISCELLANEOUS DETAILS AND NOTES

SHEET
06
OF
29

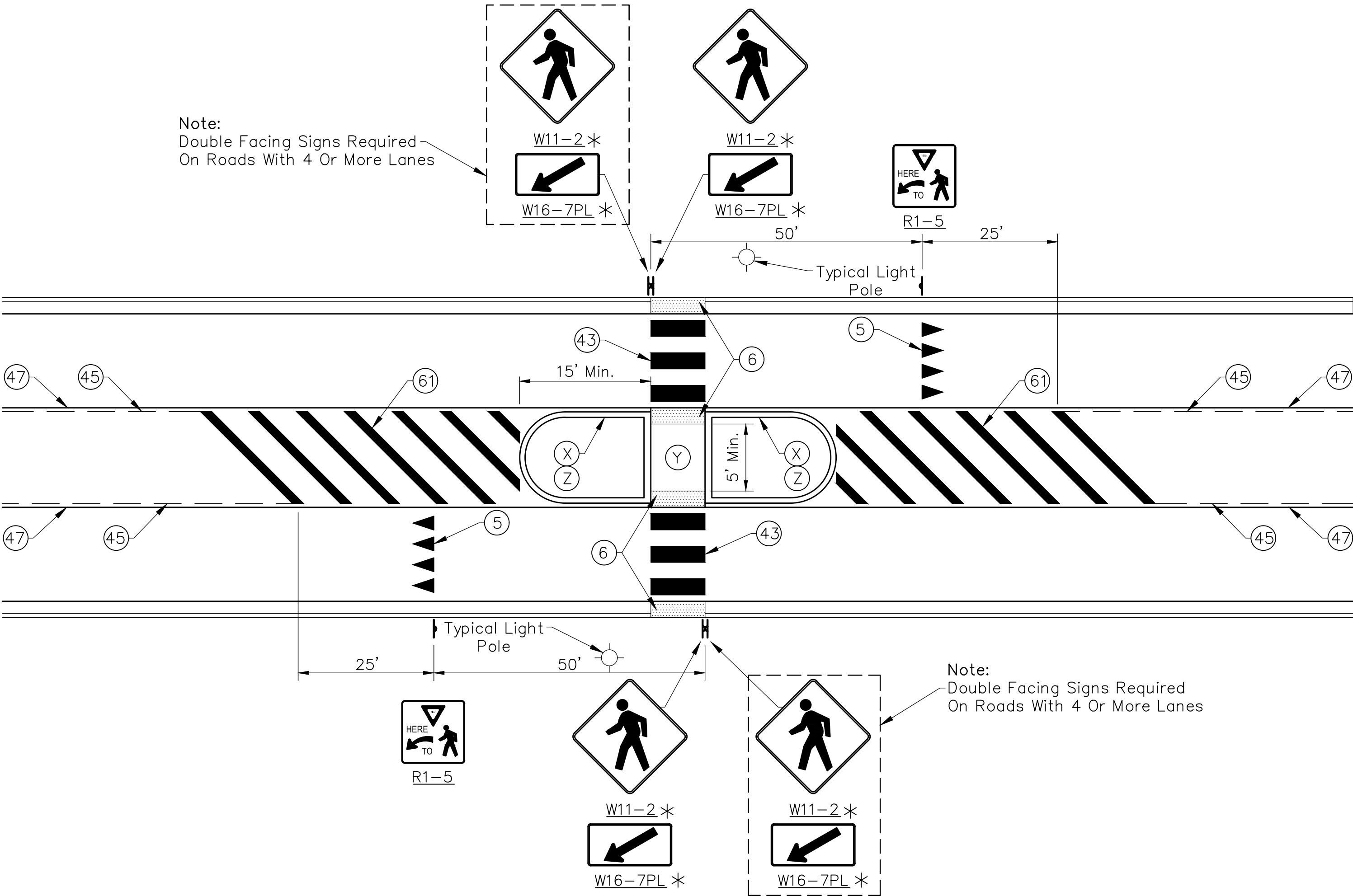




**UNSIGNALIZED MIDBLOCK CROSSWALK DETAIL**  
Not To Scale  
\* Fluorescent Yellow Green

**LEGEND**

- (5) Shark Tooth Yield Triangle 2'W x3'H
- (6) Tactile Warning Surface Shall Be Dark Brick Red Color And Shall Be Manufactured By ADA Solutions, INC., Or As Approved By Plainfield DPW.
- (43) Line, Thermo., Solid, White, 24"
- (45) Line, Thermo., Broken, Yellow, 4"
- (46) Line, Thermo., Solid, White, 4"
- (47) Line, Thermo., Solid, Yellow, 4"
- (61) Line, Thermo., Solid 45°, Yellow 12" (10' Spacing)
- (X) 18" Standard Curb
- (Y) 8" Concrete Sidewalk
- (Z) Low Height Shrub Vegetation In 18" Of Topsoil Covered With 3" Black Mulch
- Sign



**UNSIGNALIZED MIDBLOCK CROSSWALK WITH PEDESTRIAN ISLAND DETAIL**  
Not To Scale  
\* Fluorescent Yellow Green

REVISIONS		
Rev. No.	Description	Date

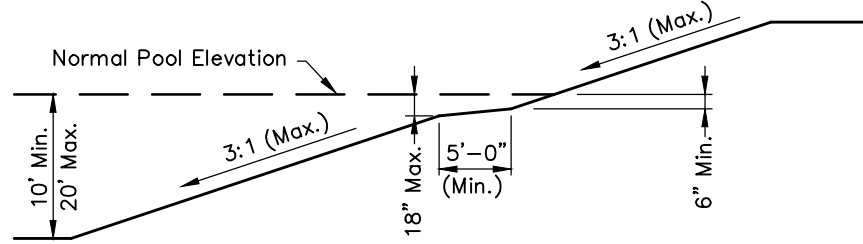


RECOMMENDED FOR APPROVAL	<i>David Lacey</i>	01/01/2015
DESIGN ENGINEER		DATE
APPROVED	<i>David Lacey</i>	01/01/2015
EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES		DATE
APPROVED	<i>John J. Fingleton</i>	01/01/2015
DIRECTOR OF TRANSPORTATION		DATE

TOWN OF PLAINFIELD
MISCELLANEOUS DETAILS AND NOTES

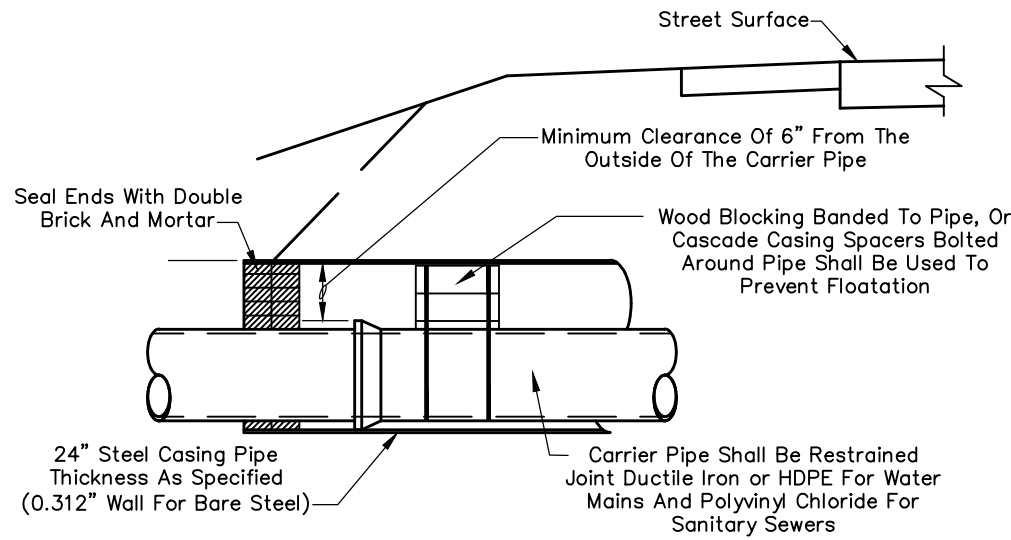
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07  
OF  
29





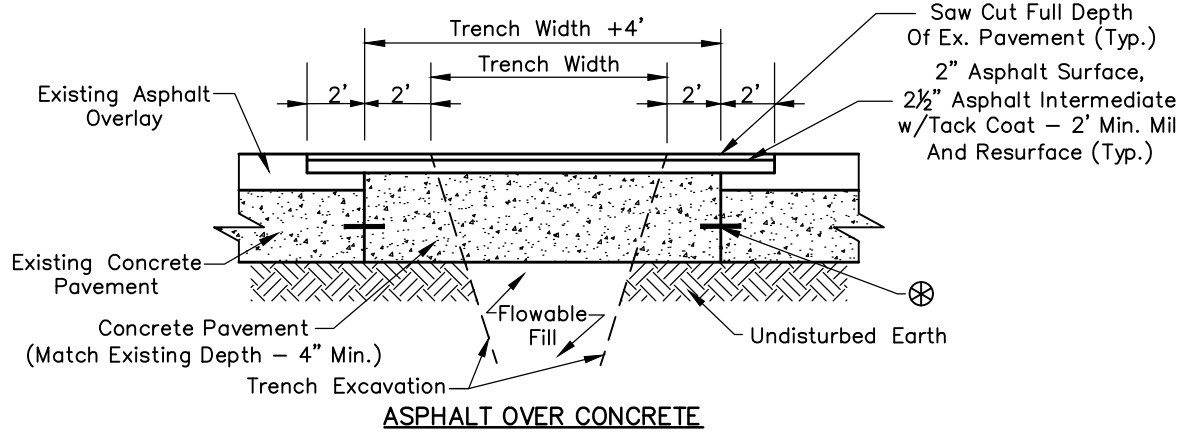
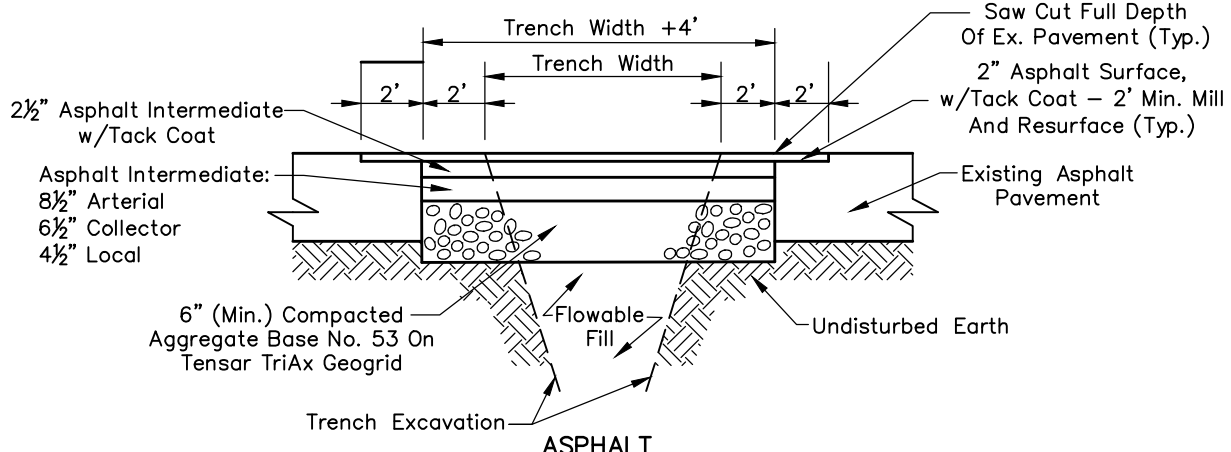
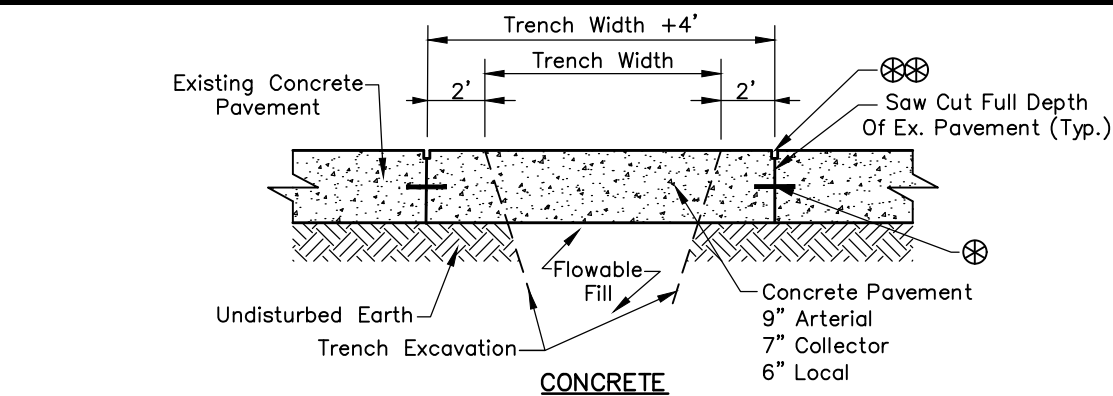
**TYPICAL DETENTION POND SECTION**  
Not To Scale

- NOTES:**
- 1.) Dry Bottom Basins Shall Be Subject To The Maximum Of 3:1 Slope Above The Basin Floor. The Longitudinal Slope Shall Be Subject To General Note 1 As Set Out On Sheet 09. The Transverse Grade Shall Be 2% Minimum.
  - 2.) Emergency Overflow Facilities Such As A Weir Or Spillway Shall Be Provided For The Release Of Exceptional Storm Runoff Or In Emergency Conditions Should The Normal Discharge Devices Become Totally Or Partially Inoperative.
  - 3.) Plainfield DPW May Approve Alternate Detention Pond/Basin Sections.



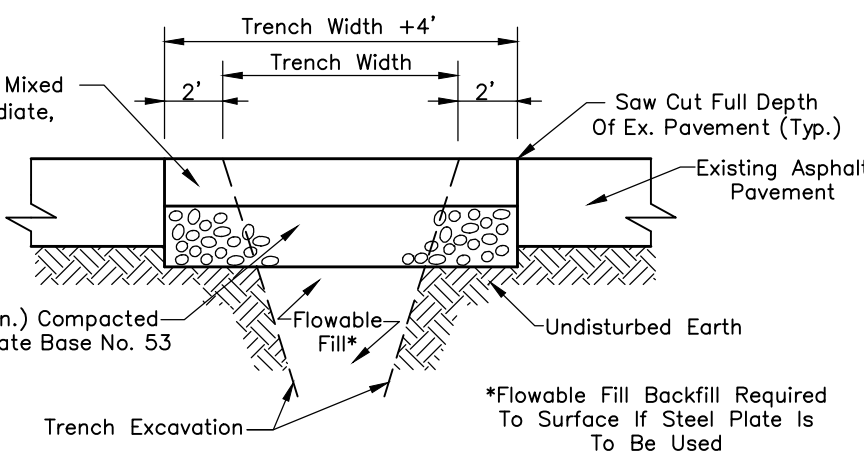
**TYPICAL STREET CASING DETAIL FOR UP TO 12" CARRIERS**  
Not To Scale

- NOTES:**
- 1.) Bored Or Jacked Crossings Require Intimate Knowledge Of Site Conditions; Therefore, Construction Is Subject To Certified Special Provisions Prepared By The Design Engineer.
  - 2.) Casings Depicted Hereon Do Not Necessarily Comply With INDOT Permit Requirements, But Are Intended To Be Used For Crossings Of Public Roads Under The Jurisdiction Of The Town Of Plainfield When Open Cut Of Such Roads Is Not Permitted.
  - 3.) Refer To Appropriate Plainfield Standards For Carrier Pipe Requirements.



**PAVEMENT RECONSTRUCTION DETAILS**  
Not To Scale

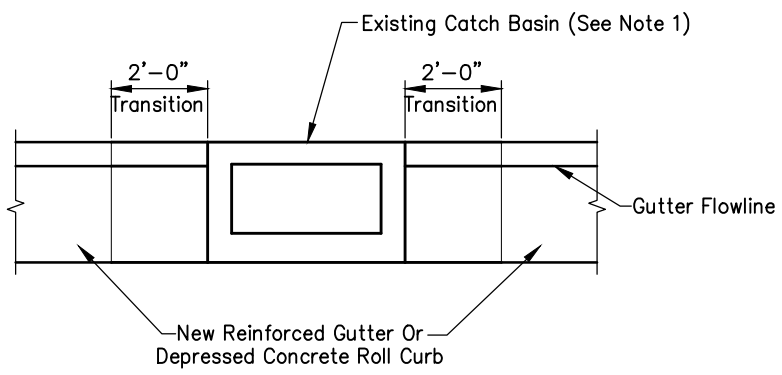
- NOTES:**
- 1.) All Concrete Shall Be Air Entrained, 6 Bag Per Cubic Yard With 4,000 PSI Minimum 28 Day Strength. Concrete Surface Shall Be Broom Finished Perpendicular To Traffic Flow.
  - 2.) Refer To INDOT Standard Drawing E506-CCPP-01 For Dowel Bar Spacing and Diameter And E503-CCPJ-08 For Retrofit Tie Bar Spacing and Diameter.
  - 3.) Refer To INDOT Standard Drawing E503-CCPJ-03 For Joint Seal Details. Joint Seals Are Not Required If Concrete Pavement Is Overlay.



**TEMPORARY ASPHALT PATCH**  
**TEMPORARY PAVEMENT PATCH DETAIL**  
Not To Scale

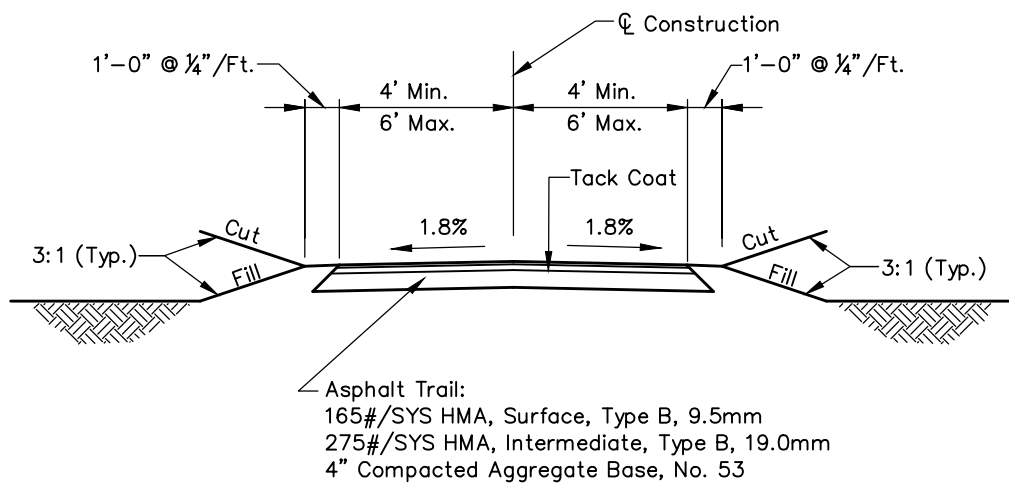
- NOTES:**
- 1.) Steel Plate Required Over Trench To Open Roadway To Traffic. Pavement Reconstruction Or Temporary Asphalt Patch To Be Placed Within 48 Hours.
  - 2.) Cold Mixed Asphalt (CMA) Shall Not Be Used When The Ambient Temperature Is Less Than 40°F. Use Flowable Fill To Surface.

DEVELOPMENT STANDARD - DETAIL DS-G01



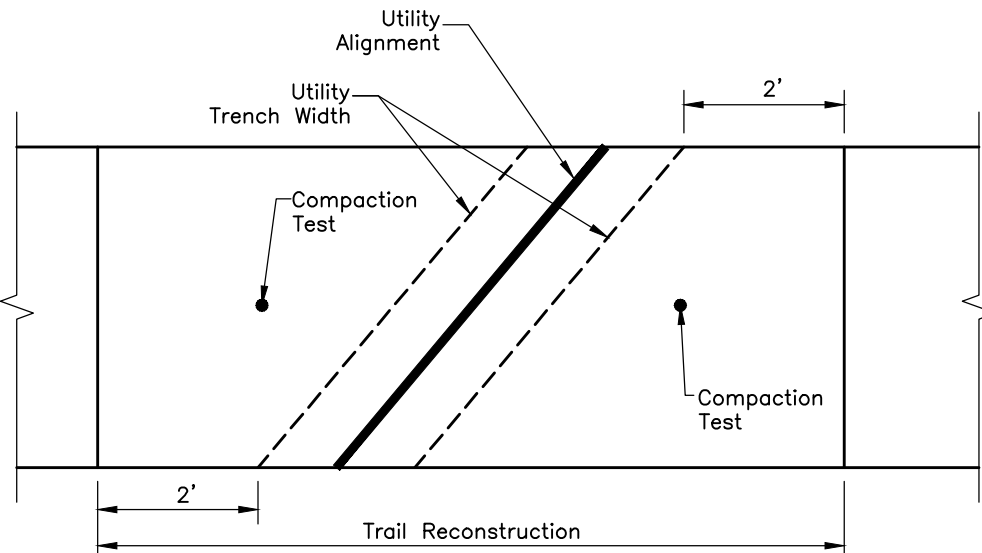
**EXISTING CATCH BASIN MODIFICATION**  
Scale: 1/4"=1'-0"

- NOTES:**
- 1.) Contractor To Verify Existing Casting Size To Determine Replacement.
  - 2.) Existing Catch Basin Within Limits Of New Approach That Can Not Be Relocated Due To Existing Gutter Flow, As Approved By Plainfield DPW.
  - 3.) As Required To Accept: Neenah R-3405 For Type A Inlet Or, Neenah R3454-B For Type B Inlet Or, Neenah R3287-5 For Type C Inlet, Or Town Approved Equal. Adjust Casting To Grade.



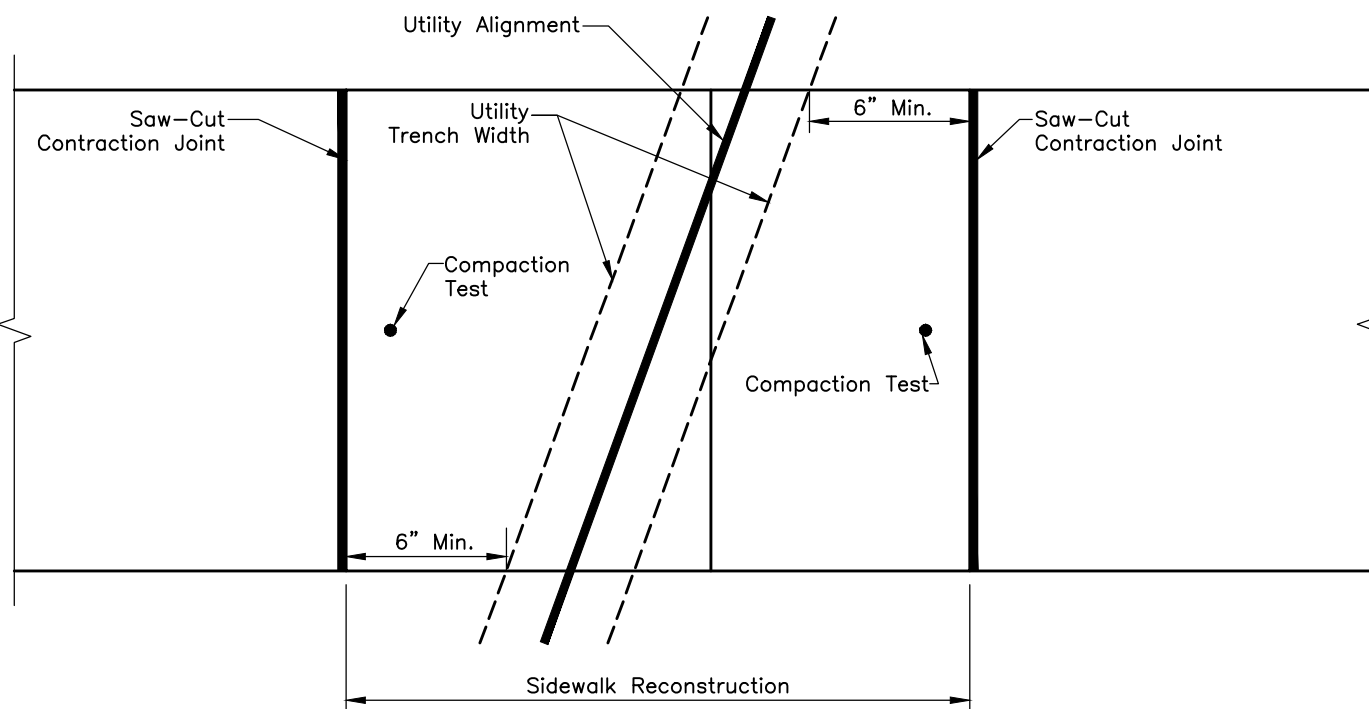
**TYPICAL SIDEWALK/TRAIL CROSS SECTION**  
Not To Scale

- NOTES:**
- 1.) Cross Slope Shall Be 1.8% Maximum For Crowns, Transitions, And Superelevations.



**EXISTING TRAIL RECONSTRUCTION**  
Not To Scale

- NOTES:**
- 1.) Full Depth Saw-Cut 2 Feet On Either Side Of The Outer Limits Of The Utility Trench And Remove Pavement. Saw-Cut Should Be Made Perpendicular To The Trail's Centerline.
  - 2.) Backfill Utility Trench With Flowable Fill Per Pavement Reconstruction Detail DS-G03.
  - 3.) Aggregate Subbase Compaction Adjacent To The Utility Trench Shall Not Be Less Than 95% Of The Maximum Dry Density As Determined By AASHTO T99. One Compaction Test On Each Side Of The Utility Trench Shall Be Performed.
  - 4.) Replace Asphalt Per Typical Trail Cross Section Detail DS-G06 Making Sure To Match Existing Grades.



**EXISTING SIDEWALK RECONSTRUCTION**  
Not To Scale

- NOTES:**
- 1.) Full Depth Saw-Cut Nearest Contraction Joints Outside Of Utility Trench And Remove Existing Sidewalk.
  - 2.) Backfill Utility Trench With Flowable Fill Per Pavement Reconstruction Detail DS-G03.
  - 3.) Aggregate Subbase Compaction Adjacent To The Utility Trench Shall Not Be Less Than 95% Of The Maximum Dry Density As Determined By AASHTO T99. One Compaction Test On Each Side Of The Utility Trench Shall Be Performed.
  - 4.) Replace Preformed Joint Filler If Removed During Sidewalk Removal.
  - 5.) If Utility Alignment Follows The Sidewalk Joint Take Adjacent Sidewalk Panels Out.
  - 6.) If Utility Trench Encroaches Within 6" Inches Of Contraction Joint Take Adjacent Sidewalk Panel Out.
  - 7.) Replace Sidewalk Per Typical Sidewalk Detail On Sheet 03 Making Sure To Match Existing Grades.

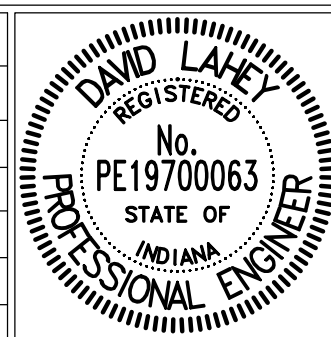
DEVELOPMENT STANDARD - DETAIL DS-G05

DEVELOPMENT STANDARD - DETAIL DS-G06

DEVELOPMENT STANDARD - DETAIL DS-G07

DEVELOPMENT STANDARD - DETAIL DS-G08

REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Loh</i>	DATE	01/01/2015
APPROVED	<i>David Loh</i>	DATE	01/01/2015
APPROVED	<i>John J. J. J.</i>	DATE	01/01/2015

TOWN OF PLAINFIELD

GENERAL (G)  
DEVELOPMENT STANDARDS

SHEET

08  
OF  
29



STORM SEWER GENERAL NOTES AND AS-BUILT DRAWINGS

- Storm Sewer Greater Than 18 Inches Shall Be Reinforced Concrete Pipe, Alternate Materials Subject To DPW Approval.
- Storm Sewer Pipe Of Other Material Or Material Not Meeting These Specifications Shall Require The Prior Written Approval Of Plainfield DPW.
- The Contractor Shall Submit Information To The Town Engineer Showing Conformance With These Specifications Upon Request.
- To Get Relief From The Town's Inlet And Manhole Requirements, A Structural Best Management Practice Or Isolation From The Town's System Is Required. The Town Standards For Bedding Of Pipe And Pipe Material For Storm Sewers Are Required Regardless.
- The Centerline Of Storm Water Quality Structures Shall Be Located As Required So As To Be Within 15' From Edge Of Pavement. Structure Cone Sections Shall Be Rotated Towards The Street.
- As-Built Drawings Shall Be Submitted To Plainfield DPW. GPS Collected Coordinates Shall Depict Actual Horizontal And Vertical Locations Of Utility Assets Such As: Manholes, Catch Basins, End Sections, Outfalls, And BMPs.

STORM SEWER REINFORCED CONCRETE PIPE

- Reinforced Concrete Pipe Shall Be Class **III**, **IV**, Or **V** As Specified In ASTM C76.
- Reinforced Elliptical Concrete Pipe Shall Be Class **HE-II** Or **HE-IV** As Specified In ASTM C507.
- Lift Holes Are Not Allowed For Pipe Less Than 24 Inches In Diameter. A Maximum Of Two Lift Holes Are Allowed For Pipe 24 Inches In Diameter Or Larger. Lift Holes Shall Be Repaired According To Most Recent INDOT Standard Specifications.
- Fittings And Specialties Shall Be In Accordance With The Specifications For The Type Of Pipe Being Used.
- Each Pipe Section Shall Be Marked With Date Of Manufacture, Size And Class Of Pipe, Specification Designation, Manufacturer And Plant Identification.
- Pipe Shall Be Furnished With A Bell Or Groove On One End Of A Unit Of Pipe And A Spigot Or Tongue On The Adjacent End Of The Adjoining Pipe. All Joints Shall Have A Groove On The Spigot For Placement Of A Rubber "O"-Ring Or Profile Gasket In Accordance With ASTM C443. The Gasket Shall Be A Continuous Ring Which Fits Snugly Into The Annular Space Between The Overlapping Surfaces Of The Assembled Pipe Joint.

STORM SEWER POLYVINYL CHLORIDE (PVC) PIPE

- Pipe Diameters Of 12 Inches Through 15 Inches Shall Meet Or Exceed All The Requirements Of ASTM D3034, And Shall Have A Minimum Cell Classification Of 12454. Reference Should Be Made To ASTM D1784 For A Summarization Of Cell Class Properties. Pipe Diameters Greater Than 15 Inches Shall Meet Or Exceed All Requirements Of ASTM F679, And Shall Have A Minimum Cell Classification Of 12454. PVC Ribbed Sewer Pipe Shall Meet Or Exceed All Requirements Of ASTM F794, And Shall Have A Minimum Cell Classification Of 12454.
- The Minimum Wall Thickness Of Pipe 12 Inches Through 15 Inches In Diameter Shall Conform To SDR-26, Type PSM, As Specified In ASTM D3034. The Minimum Wall Thickness For Pipe Diameters Greater Than 15 Inches Shall Conform To PS 115 As Specified In ASTM F679. PVC Pipe Shall Have A Minimum Pipe Stiffness Of 115 Pounds Per Square Inch For Each Diameter When Measured At Five Percent Deflection And Tested In Accordance With ASTM D2412.
- Pipe Joints Shall Have A Bell Wall, Gasket Groove, And Spigot Which Are Integral With The Pipe. The Assembly Of Joints Shall Be In Accordance With The Pipe Manufacturer's Recommendations And ASTM D3212. No Solvent Cement Joints Shall Be Allowed. Gasket Material Shall Be Constructed Of Styrene Butadiene Or Butyl Rubber And Meet The Requirements Of ASTM F477.
- Each Pipe Section Shall Be Marked With Name Of Manufacturer, Trademark Or Tradename, Nominal Pipe Size, Production/Extrusion Code, Material And Cell Classification, And ASTM Number.
- Installation Shall Be In Accordance With Recommended Practice ASTM D2321.

STORM SEWER HIGH DENSITY POLYETHYLENE (HDPE) CORRUGATED PIPE

- Requirements For Test Methods, Dimensions, And Markings Are Those Found In AASHTO Specifications M-252 And M-294.
- Pipe And Fittings Shall Be Made Of Polyethylene Compounds Which Meet Or Exceed The Requirements Of Type **III**, Category 4 Or 5, Grade P33 Or P34, Class C Per ASTM D1248.
- Minimum Pipe Stiffness Values Shall Be In Accordance With AASHTO Specifications M-294.
- The HDPE Corrugated Pipe Shall Have An Integrally Formed Smooth Interior. Male And Female Pipe Ends Which Allow The Construction Of Overlapping Gasket Joints Shall Be Made In Conformance With ASTM D3212. Neoprene Gaskets Shall Meet ASTM F477.
- Installation Shall Be In Accordance With Recommended Practice ASTM D2321.

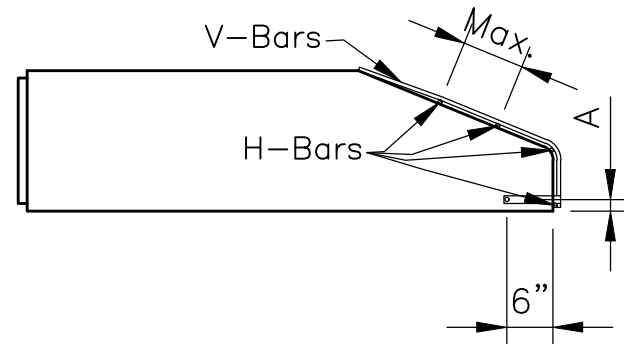
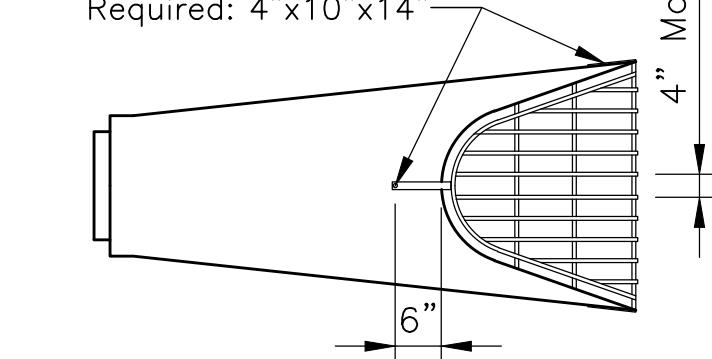
STORM SEWER CORRUGATED POLYPROPYLENE (PP) PIPE

- 12-inch through 18-inches Pipe Shall Be Smooth Interior And Annular Exterior Corrugated Polypropylene (PP) Pipe Meeting The Requirements Of ASTM F2764, ASTM F2881 or AASHTO M330 Type S (Double-Wall) Or D (Triple-Wall), For Respective Diameters.
- Material For Pipe And Fitting Production Shall Be An Impact Modified Copolymer Meeting The Material Requirements Of ASTM F2764, ASTM F2881 And AASHTO M330, For Respective Pipe Diameters.
- Watertight Joints Shall Be Bell-And-Spigot Meeting The Watertight Requirements Of ASTM D3212. Gaskets Shall Comply With The Requirements Of ASTM F477. Gaskets Shall Be Installed By The Pipe Manufacturer And Covered With A Removable Wrap To Ensure The Gasket Is Free From Debris. A Joint Lubricant Supplied By The Manufacturer Shall Be Used On The Gasket And Bell During Assembly.
- Fittings Shall Conform To ASTM F2764, ASTM F2881 Or AASHTO M330, With The Exception Of Meeting The Watertight Joint Performance Requirements Of ASTM D3212. Gasketed Bell And Spigot Connections Shall Utilize A Spun-on, Welded Or Integral Bell And Spigot With Gaskets Meeting ASTM F477.
- Each Pipe Section Shall Be Marked With Nominal Pipe Size, Class Size And Wall, Date Of Manufacture, Trademark or Tradename and ASTM Specification.
- Installation Shall Be In Accordance With ASTM D2321 And Manufacture's Recommended Installation Guidelines.

STORM SEWER DEFLECTION TESTING AND TELEVISING

- Deflection Testing Is Required For All Mainline Flexible Pipe And Plainfield DPW Shall Be Given 24 Hour Written Notice Of Deflection Testing. An Allowable Deflection Of 5 Percent Inside Pipe Diameter Will Be Acceptable After All Backfilling Has Been In Place For 30 Days. A Nine-Point "Go-No-Go" Mandrel Shall Be Used For The Deflection Test. A Proving Ring Shall Be Provided For Each Mandrel. All Pipe Exceeding The Allowable Deflection Shall Be Televised To Determine The Extent Of Replacement Or Rerouting Required. The Reworked Section Shall Be Retested 30 Days After Completion. Contractor Shall Bear All Testing Costs. The "Go-No-Go" Mandrel Shall Be Manually Pulled Without The Use Of Mechanical Devices.
- Televising Is Required For All Pipe Installations. Plainfield DPW Shall Be Given 24 Hour Written Notice Of Televising. A Camera Equipped With Remote Control Devices To Adjust Light Intensity And 1,000 Linear Feet Of Sewer Cable Shall Be Provided. The Camera Shall Transmit A Continuous Image To The Television Monitor As It Is Being Pulled Through Pipe. The Image Shall Be Clear Enough To Enable The Town Of Plainfield Representative And Others Viewing The Monitor To Easily Evaluate The Interior Condition Of The Pipe. The Camera Shall Stamp The Video Tape With Linear Footage And Project Number, And An Audio Voice-Over Shall Be Made During The Inspection Identifying Problems. Contractor Shall Bear All Televising Costs.
- The Pipe Shall Be Thoroughly Cleaned Before Installing Camera And Commencing Televising.
- If Any Pipe And/Or Joint Is Found To Be Leaking In Such A Way As Soil Migration Is Likely In The Sole Judgment Of The Town, The Contractor Shall Repair That Portion Of The Work To The Satisfaction And Approval Of The Town Of Plainfield.

Bolt To Apron 6" From Edge Of Concrete 3 Bolt Plates Required: 4"x10"x14"



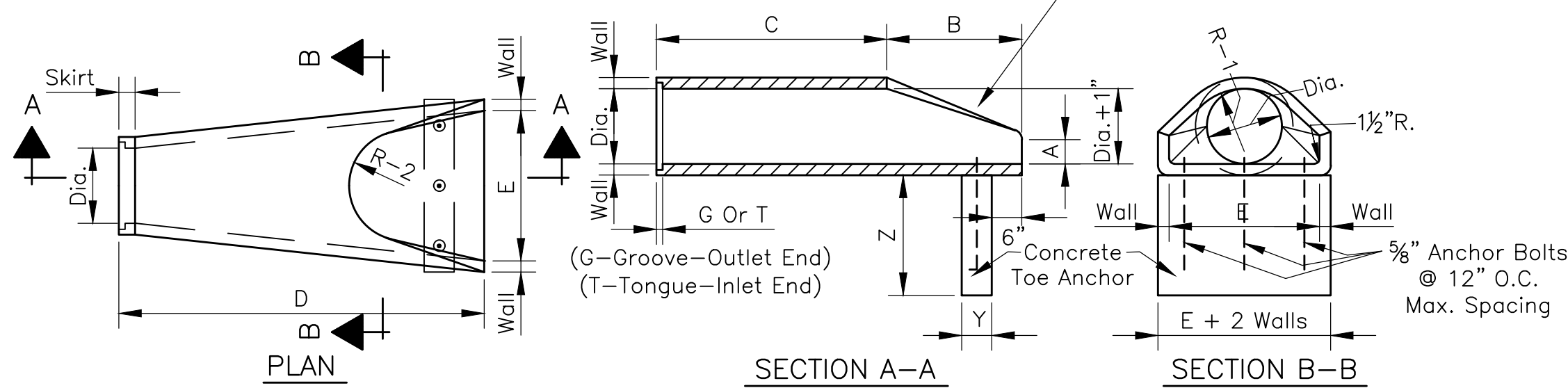
APRON SIZE	V-BAR SIZE (ø)	H-BAR SIZE (ø)	No. OF H-BARS	BOLT DIA.	"A" DIM
12	1/2	3/8	3	1/2	4
15	1/2	3/8	3	1/2	4 1/2
18	1/2	3/8	4	1/2	4 1/2
21	1/2	3/8	4	1/2	5
24	3/4	3/4	4	1/2	5
27	3/4	3/4	4	1/2	5 1/2
30	3/4	3/4	4	1/2	5 1/2
36	3/4	1	4	3/4	8
42	3/4	1	4	3/4	8
48	3/4	1	5	3/4	8
54	3/4	1 1/2	5	3/4	8
60	3/4	1 1/2	5	3/4	8
66	3/4	1 1/2	5	3/4	8
72	3/4	1 1/2	5	3/4	9
84	3/4	1 1/2	5	3/4	10
90	3/4	1 1/2	5	3/4	10

ANIMAL GUARD

Scale: None

- NOTES:
- Animal Guard Is Not Required For Culvert Crossings

End Section End Treatment (Per Animal Guard Detail) Involving Horizontal Minimum No. 12 Gauge Hot Dipped Galvanized Steel Tubes Of Suitable Diameter And Suitably Affixed To Sloping Portion Of The End Section Shall Be Provided

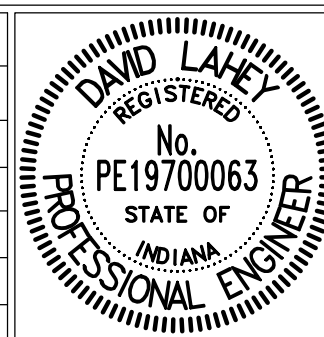


DIA.	WALL	G or T	WT. SEC.	A	B	C	D	E	DIA.+1"	R-1	R-2	SKIRT	Y	Z
12	2	1 1/2	530	4	24	48 7/8	72 7/8	24	13	10 1/16	9	3 1/2	12	24
15	2 1/4	2	740	6	27	46	73	30	16	12 1/2	11	3 1/2	12	24
18	2 1/2	2 1/2	990	9	27	46	73	36	19	15 1/2	12	4	12	24
21	2 3/4	2 1/2	1280	9	35	38	73	42	22	16 1/8	13	4	12	36
24	3	2 1/2	1520	9 1/2	43 1/2	30	73 1/2	48	25	16 11/16	14	4 1/2	18	36
27	3 1/4	2 1/2	1930	10 1/2	48	25 1/2	73 1/2	54	28	17 3/4	14 1/2	4 1/2	12	36
30	3 1/2	3	2190	12	54	19 3/4	73 3/4	60	31	18 5/16	15	5	12	36
33	3 3/4	3 3/8	3150	13 1/2	58 1/2	39 1/4	97 3/4	66	34	23 3/4	17 1/2	5 1/2	18	36
36	4	3 1/2	4100	15	63	34 3/4	97 3/4	72	37	24 1/16	20	5 1/2	18	36
42	4 1/2	3 3/4	5380	21	63	35	98	78	43	27 1/4	22	5 1/2	24	36
48	5	4 1/4	6550	24	72	26	98	84	49	28 1/8	22	5 3/4	24	36
54	5 1/2	4 3/4	8040	27	65	35	100	90	55	32 7/8	24	6 1/4	30	36
60	6	5	8750	30	60	39	99	96	61	36 3/4	24	6 3/4	30	36
66	6 1/2	5 1/2	10630	24	78	21	99	102	67	35 11/16	24	7 1/4	30	36
72	7	6	12520	34	78	21	99	108	73	38 5/8	24	7 3/4	36	36
78	7 1/2	6 1/2	14430	24	78	21	99	114	79	41 15/16	24	8 1/2	36	36
84	8	7	16350	24	78	21	99	120	85	44 13/16	24	9	39	36

PRECAST CONCRETE PIPE END SECTION

Scale: None

REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Lacey</i>	01/01/2015
DESIGN ENGINEER		DATE
APPROVED	<i>James J. Bero</i>	01/01/2015
EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES		DATE
APPROVED	<i>James Lacey</i>	01/01/2015
DIRECTOR OF PUBLIC WORKS		DATE

TOWN OF PLAINFIELD

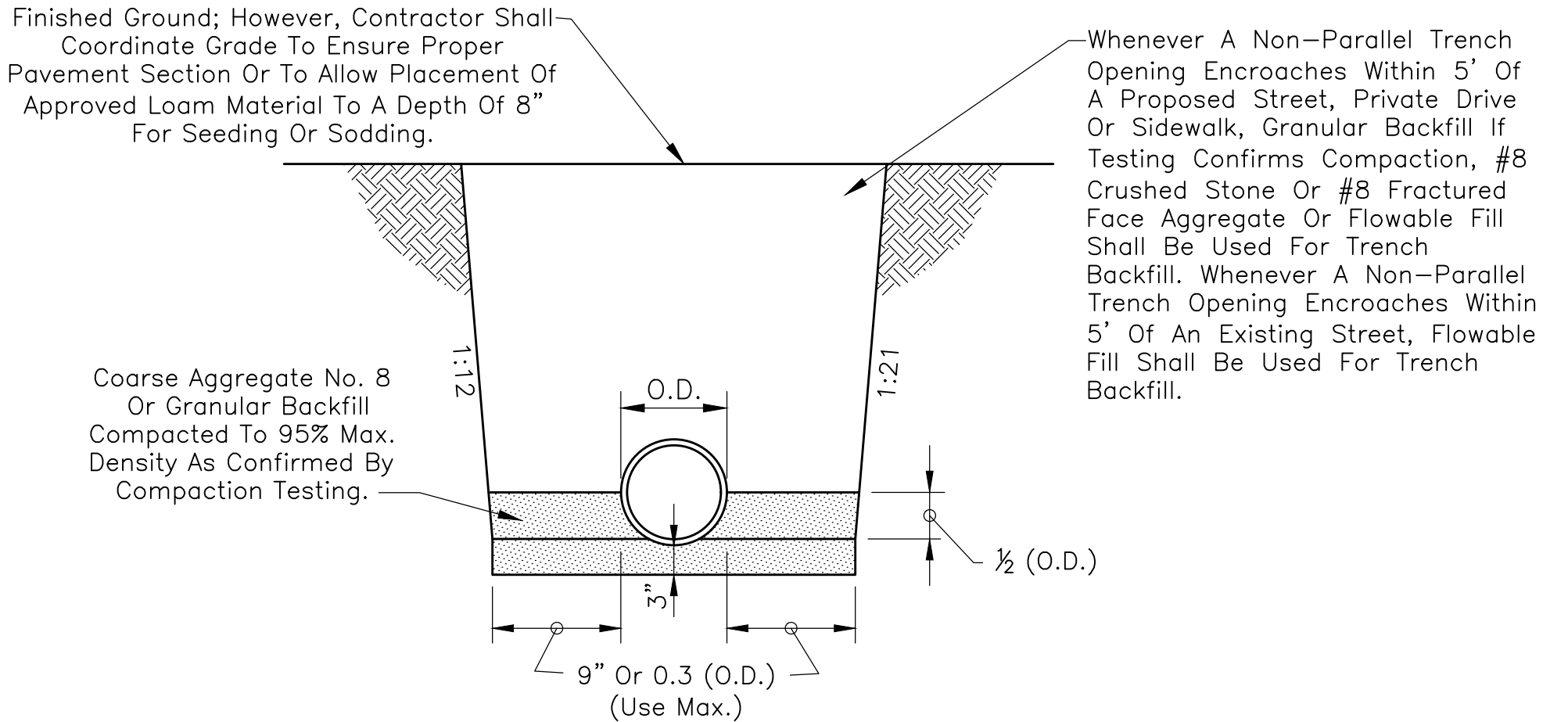
STORM SEWER BEDDING  
DETAILS AND NOTES

SHEET

09

OF

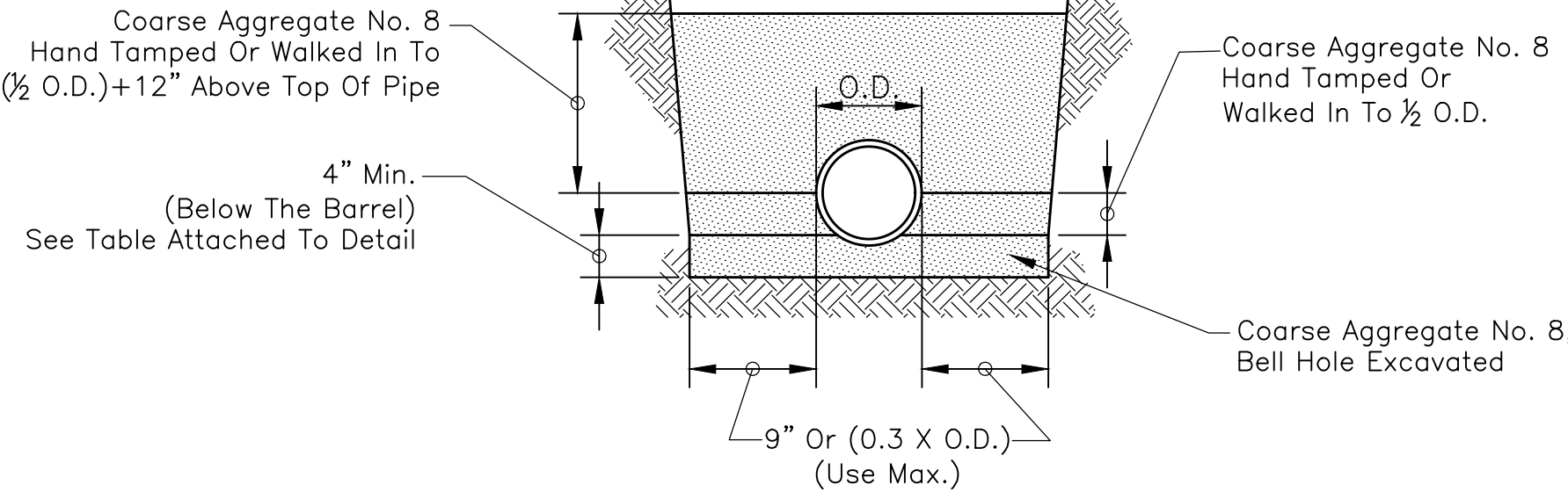
29



RCP PIPE BEDDING DETAIL

Scale: None

Structure Backfill According To INDOT Specification 211- When Trench Opening Encroaches Within 5' Of An Existing Or Proposed Street Or Sidewalk. Approved Backfill Material Outside Of B-Borrow Backfill Limits. Approved Backfill Material May Be Used Under Proposed Sidewalks Provided Sidewalks Are Constructed 6 Months After Backfilling Of Trenches Up To 6' Deep, 8 Months For Trenches 6'-10' Deep, 10-12 Months For Trenches Greater Than 10' Deep.



Pipe Size	12" TO 15"	18" And Over
Bedding Below The Pipe Barrel	O.D./4 Min.=4"	O.D./4 Min.=8"

FLEXIBLE (PVC, PP OR HDPE) PIPE BEDDING DETAIL

Scale: None

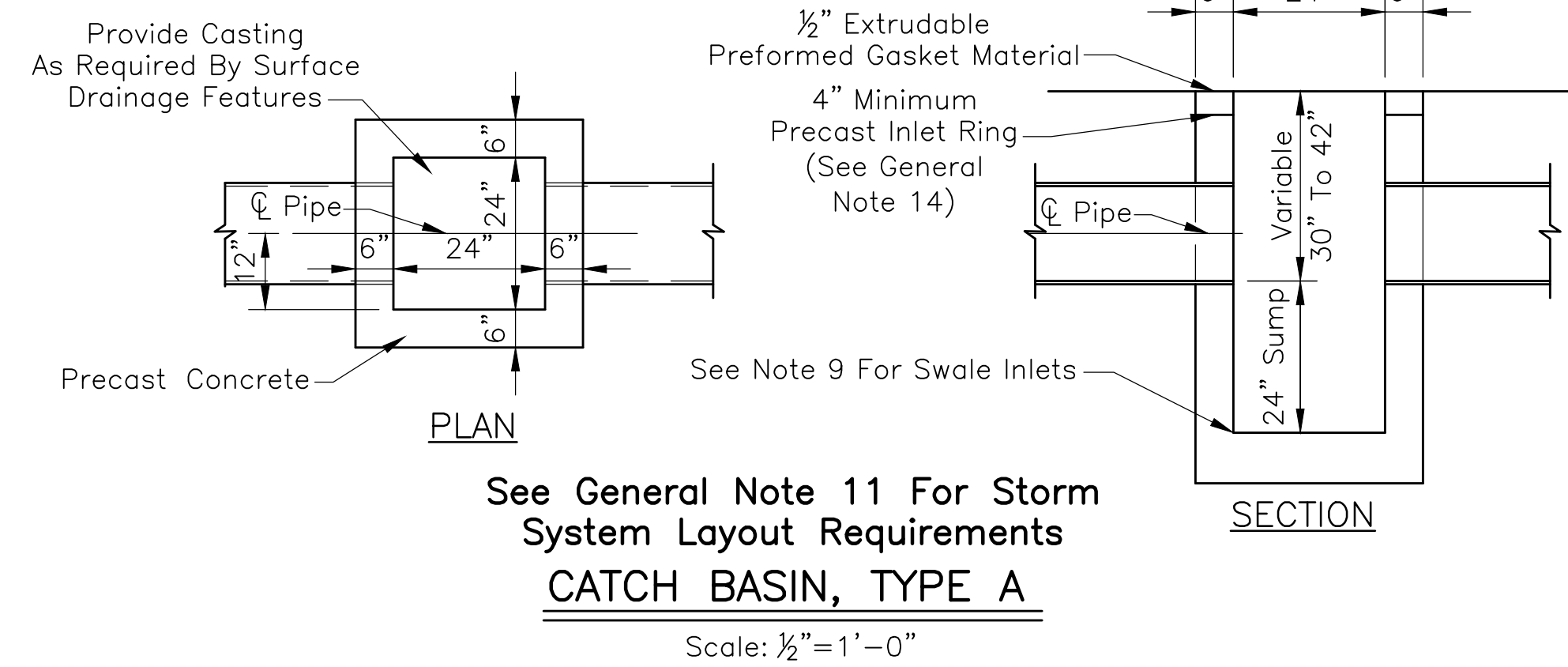
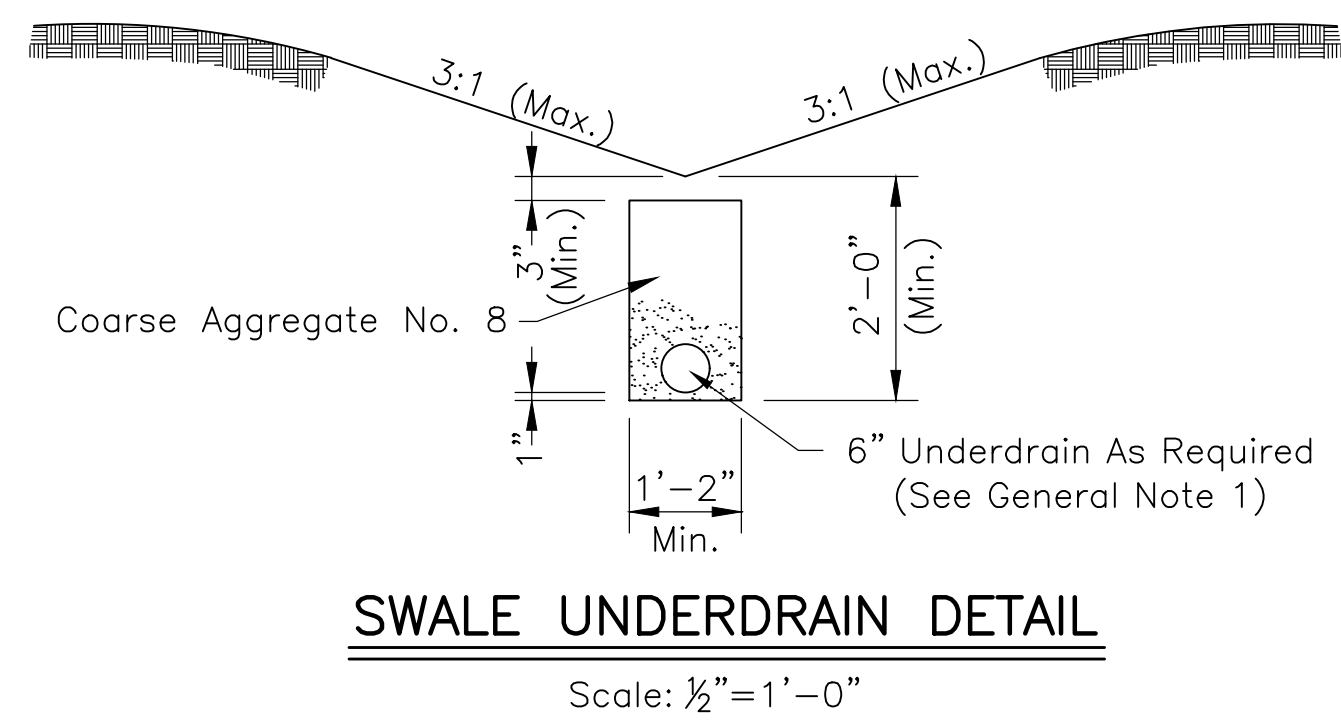
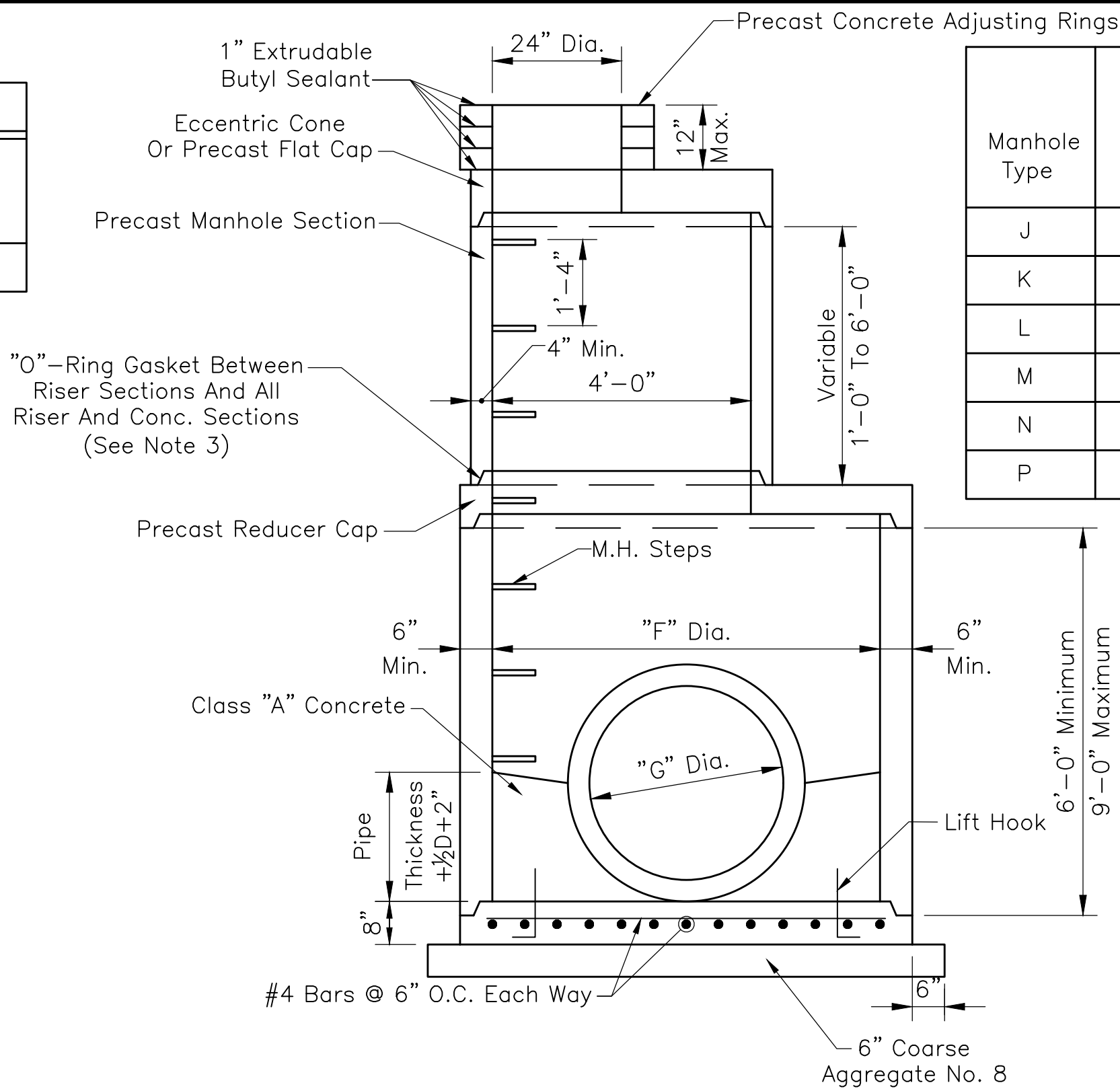
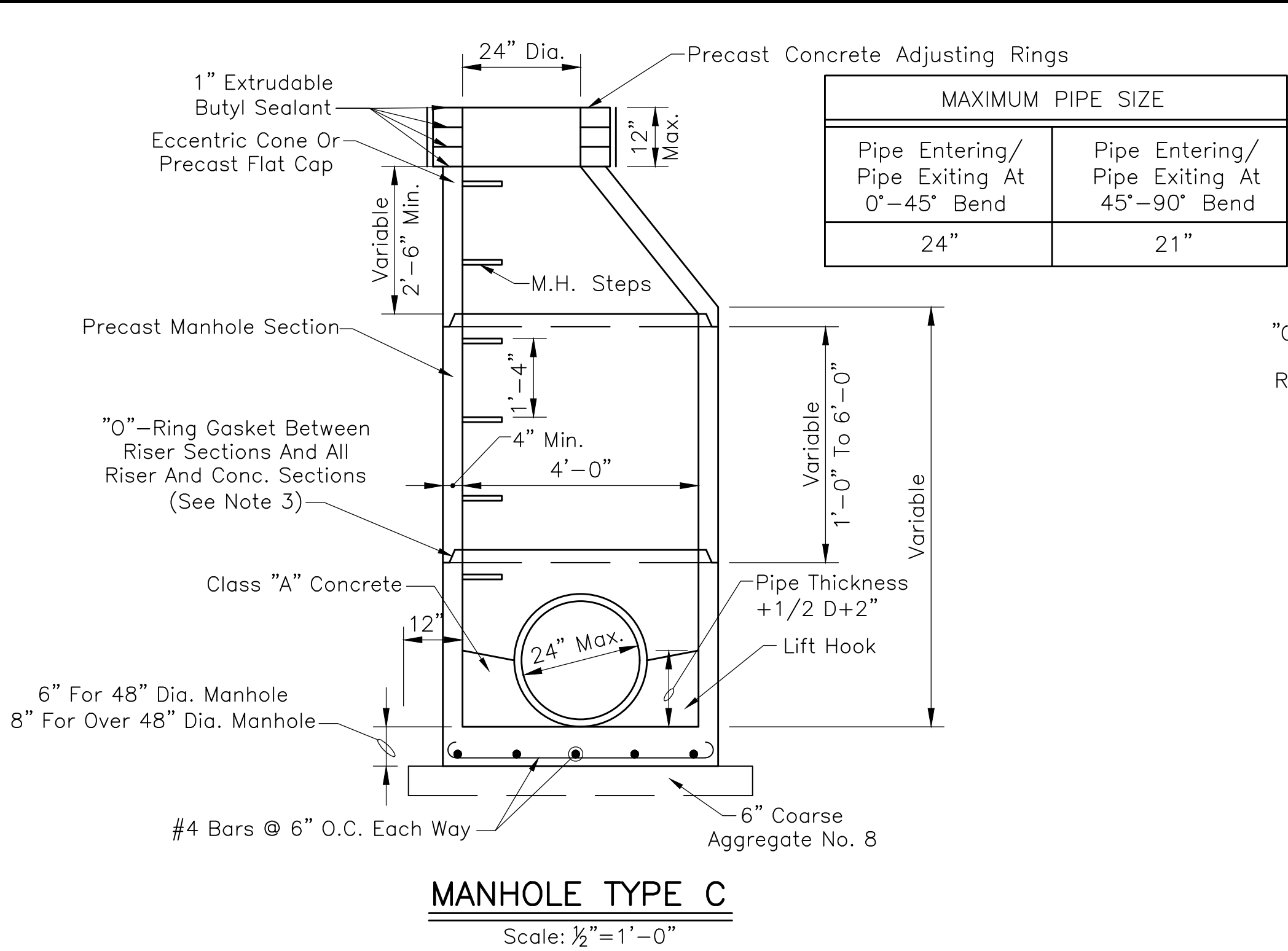
NOTES:

- Special Consideration Should Be Made For Shallow Depth Flexible Pipe Where Flotation Is A Possibility.
- Anti-Flotation Measures Should Be Considered Per Manufacturers Recommendation.

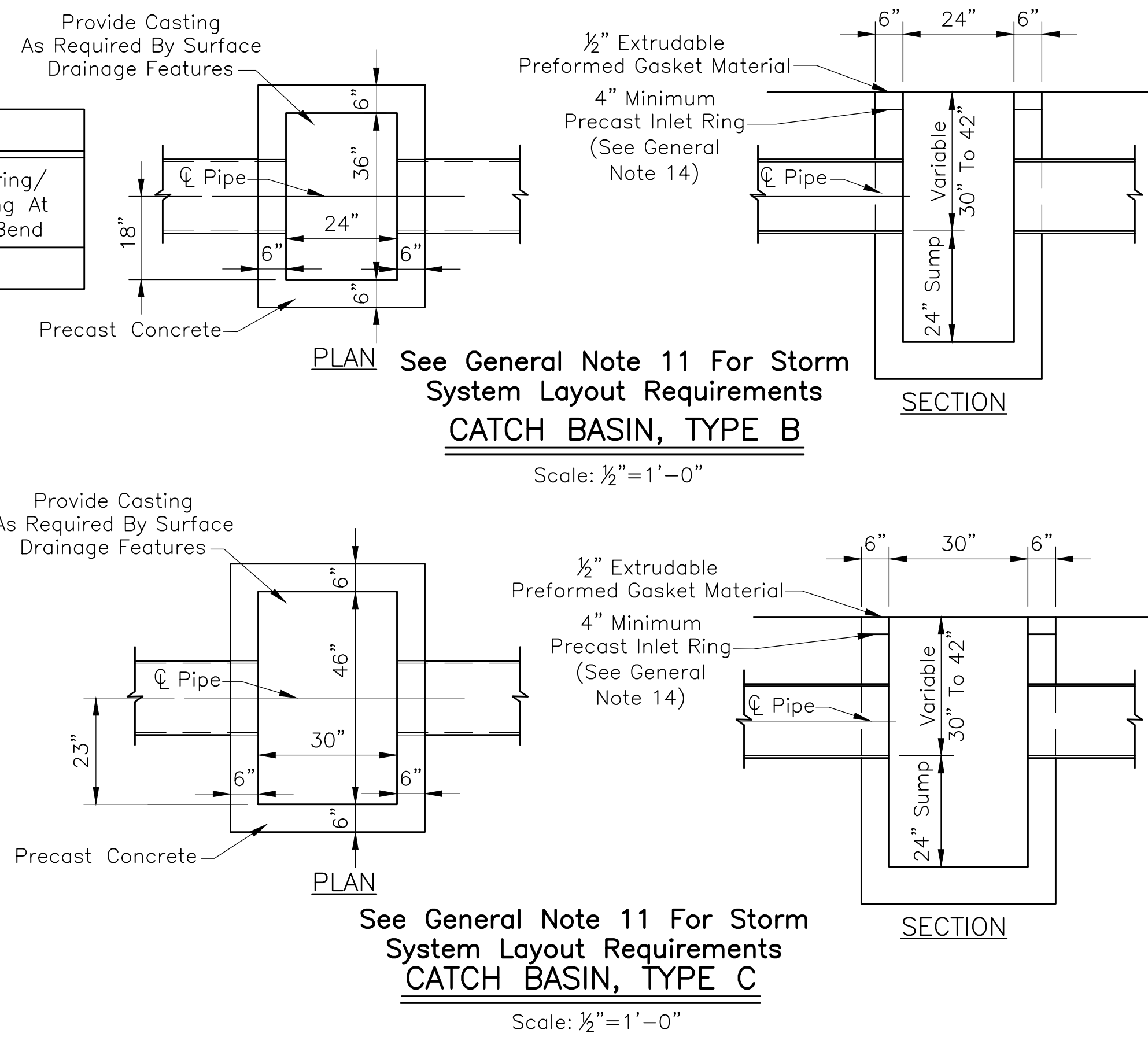
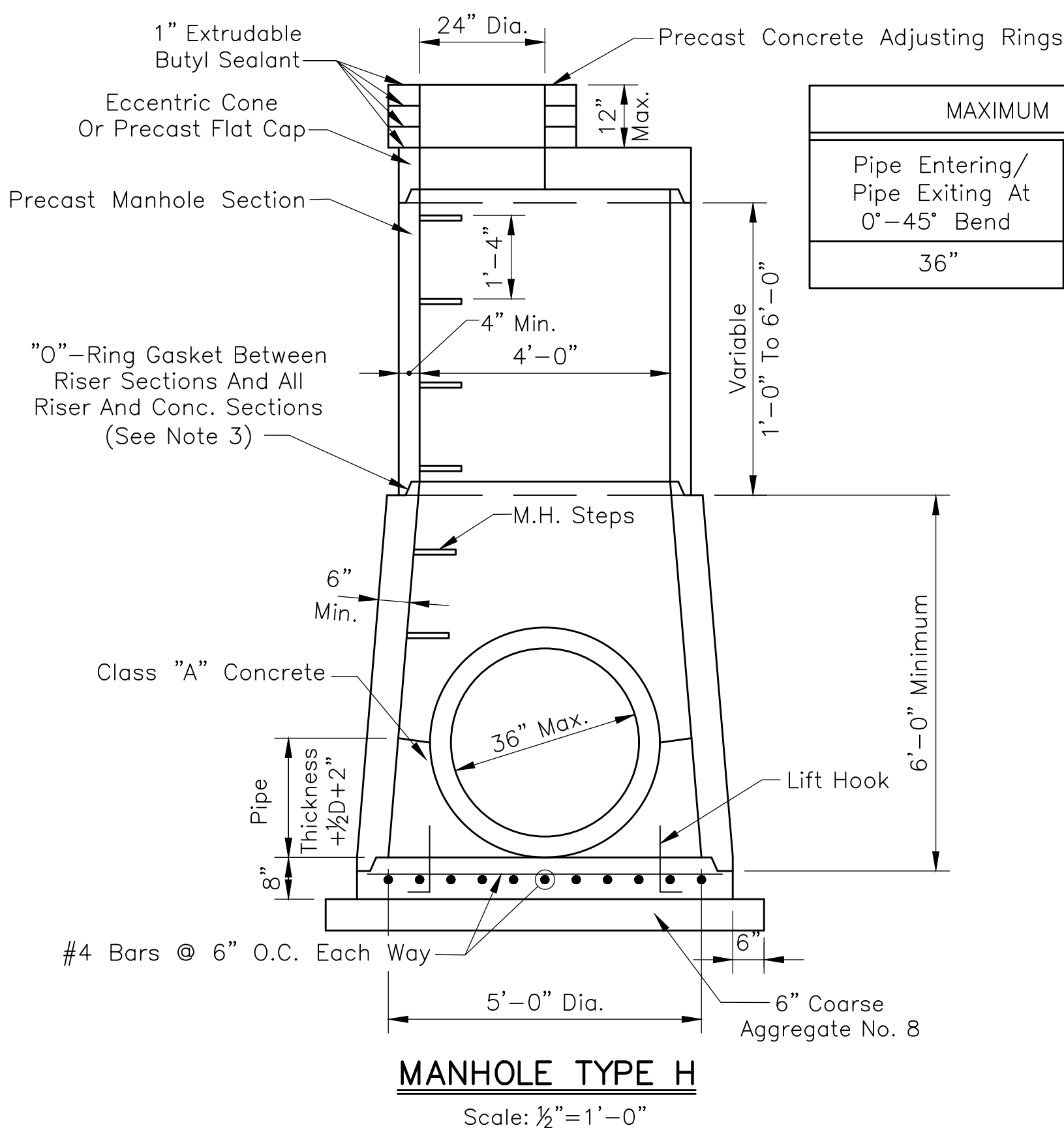
NOTES:

- Precast Flared Reinforced Concrete Pipe End Sections Shall Be Used At Exposed Pipe Ends. Concrete Toe Anchors Shall Be Required. Plastic Pipe Shall Require A Full Length Section Of Reinforced Concrete Pipe Jointed By A Concrete Collar Prior To The Precast Concrete Pipe End Section.
- Revetment Riprap In Accordance With The Most Recent INDOT Channel Design Guide Set On Geotextile In Accordance With The Most Recent INDOT Standard Specifications Shall Be Required At Inlet And Outlet Precast Flared Reinforced Concrete Pipe End Sections.
- Pipe End Sections Shall Have Appropriately Designed Riprap Outlet Protection. Refer To Outlet Protection Detail On Sheet 18.

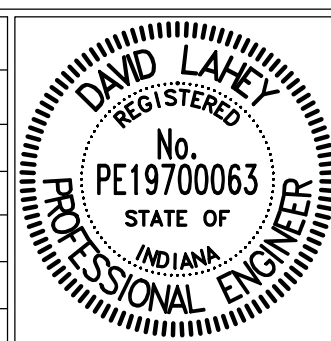




- GENERAL NOTES**
- Swales Shall Be Constructed With A Minimum 0.3 Percent Profile Grade Provided That A 6 Inch Diameter Underdrain Is Provided For Residential Swales And Commercial/Industrial Swales With Less Than A 0.5 Percent Profile Grade. See Detail On This Sheet.
  - Type J, K, L, M, N, And P Manholes As Detailed Hereon Require A Certain Minimum Depth. In Cases Where The Depth Of The Storm Sewer Is Not Sufficient To Meet The Minimum Depth As Required By The Detail, "F" Diameter Manhole Section May Be Used Throughout The Depth Of The Manhole.
  - Manholes Shall Conform To ASTM C478. Joints Shall Conform To ASTM C443. The Use Of Cast-In-Place Concrete Structures Shall Require The Prior Written Approval Of The Town Engineer. Regardless Of The Type Of Casting Used, The Casting Shall Be Centered Over The Manhole Steps.
  - Manhole Steps Shall Be Neenah R-1981-J, M.A. Industries PS 1-PF, Or As Approved By Plainfield DPW.
  - For Drainage Of Roll Curb And Gutter, Type **I**, Provide As Per Development Standard Detail DS-D01 Or As Approved By Plainfield DPW.
  - For Drainage Of Combined Curb And Gutter, Type **II**, Provide As Per Development Standard Detail DS-D02 Or As Approved By Plainfield DPW. For Additional Capacity As Directed By The Engineer, Provide As Per Development Standard Detail DS-D03 Or As Approved By Plainfield DPW. Manholes Shall NOT Directly Drain Type **II** Curb.
  - For Drainage Of Open Pavement Areas Without Curbing At An Inlet, Provide As Per Development Standard Detail DS-D04 Or As Approved By Plainfield DPW.
  - For Drainage Of Open Pavement Areas Without Curbing At A Manhole, Provide As Per Development Standard Detail DS-D05 Or As Approved By Plainfield DPW.
  - Castings For Use On Inlets Or Manholes Which Drain Swales Or Dry Bottom Detention Basins Shall Be As Per Development Standard Detail DS-D06 Or As Approved By Plainfield DPW.
  - Castings For Manholes Which Do Not Drain Surface Water Shall As Per Development Standard Detail DS-D07 Or As Approved By Plainfield DPW.
  - Mainline Pipe Shall NOT Connect To Catch Basins. Catch Basin Connections Occur At A Manhole. Mainline Pipe Is Any Pipe Downstream Of A Single Set Of Two Catch Basins Or Any Pipe Larger Than Or Equal To 15 Inch Diameter. Pipe Less Than Or Equal To 15 Inch Diameter Which Drains One Swale Inlet May Be Connected To Catch Basins When The Invert Depth Of Such Catch Basin Is Not Greater Than Shown On The Catch Basin Detail. A 10'-15' Offset Is Required For Inlet Pipes Parallel To Mainline Pipe. It Is Noted That On Commercial Sites No Pipe Is Considered Mainline Pipe Until It Enters The Public R-O-W. Further, On Commercial Sites Precast Concrete Structures, As Detailed By Outside Sources, May Be Used Subject To The Providing Of A Suitable Transition So That Castings Prescribed For Use Within Plainfield Are Used, And Subject To Storm Sewer General Note 4 On Sheet 7.
  - Catch Basins Require Back Plaster Inside And Out. Castings May Be Adjusted As Much As 1 1/2" Using Cretex PenngROUT Or As Approved By Plainfield DPW. Special Adjustment Up To 6" Using Precast Adjusting Ring With 1/2" Butyl Rubber Gasket May Be Used If Approved By Plainfield DPW.
  - All Castings Shall Be Per Sheet No. 10 Of The Town Standards.
  - All Inlets And Catch Basins Shall Have A Minimum Of 3" Allowed For Riser Rings Or Adjustment; Manholes Shall Have A Minimum Of 4".
  - When A Structure Encroaches Within 5' Of A Roadway, Or At The Discretion Of Plainfield DPW, It Shall Be Backfilled With Coarse Aggregate No. 8.



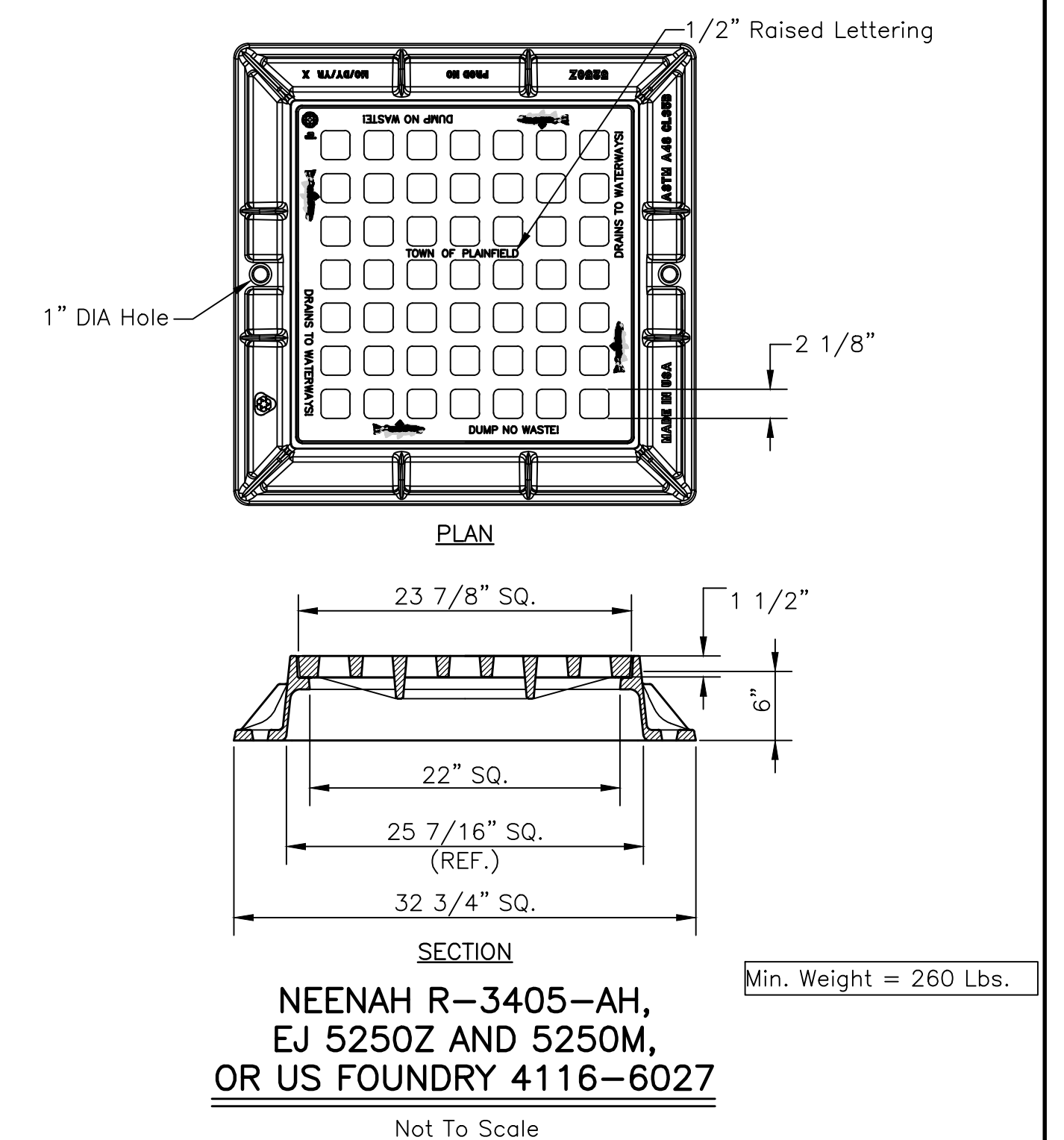
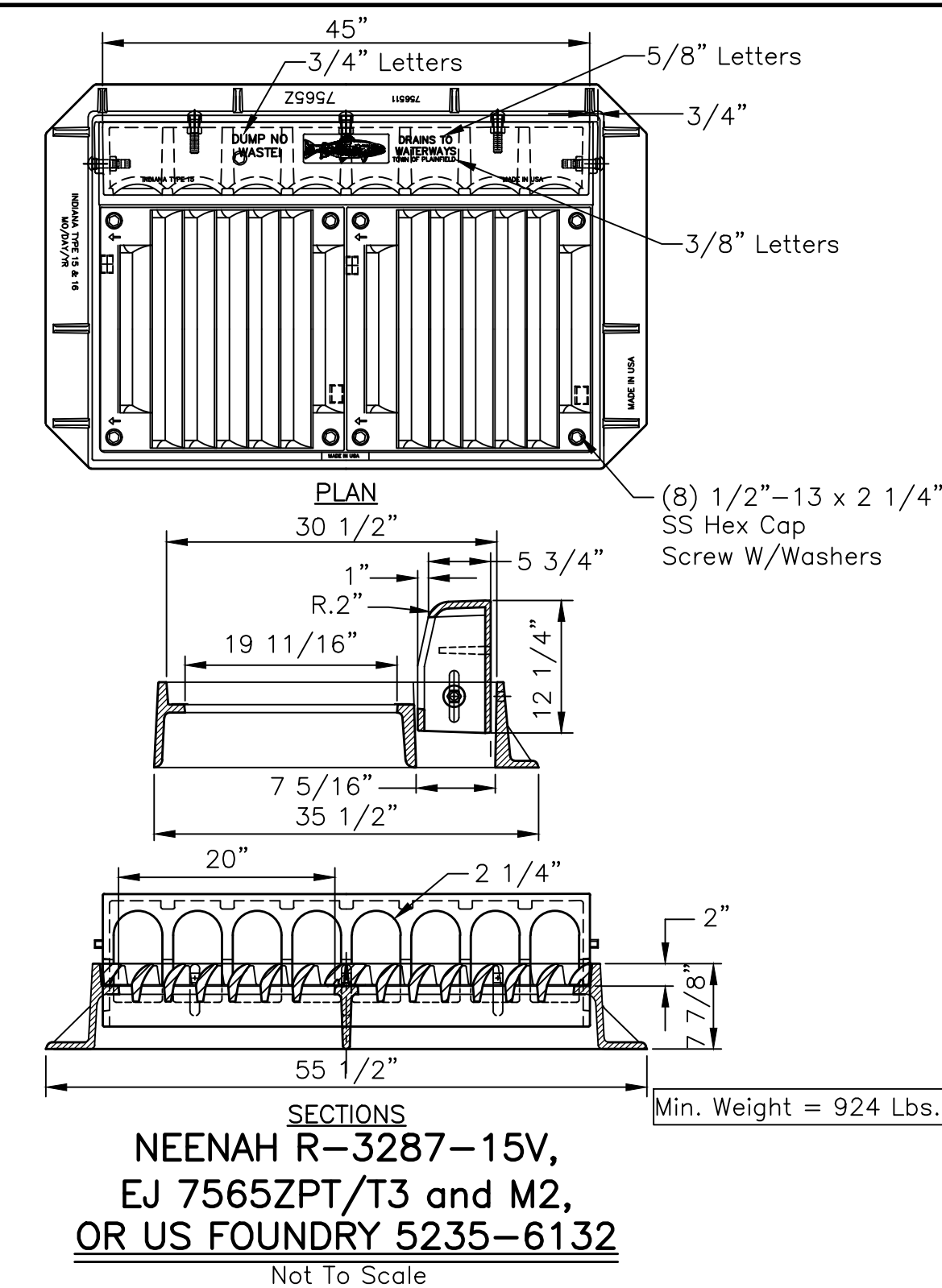
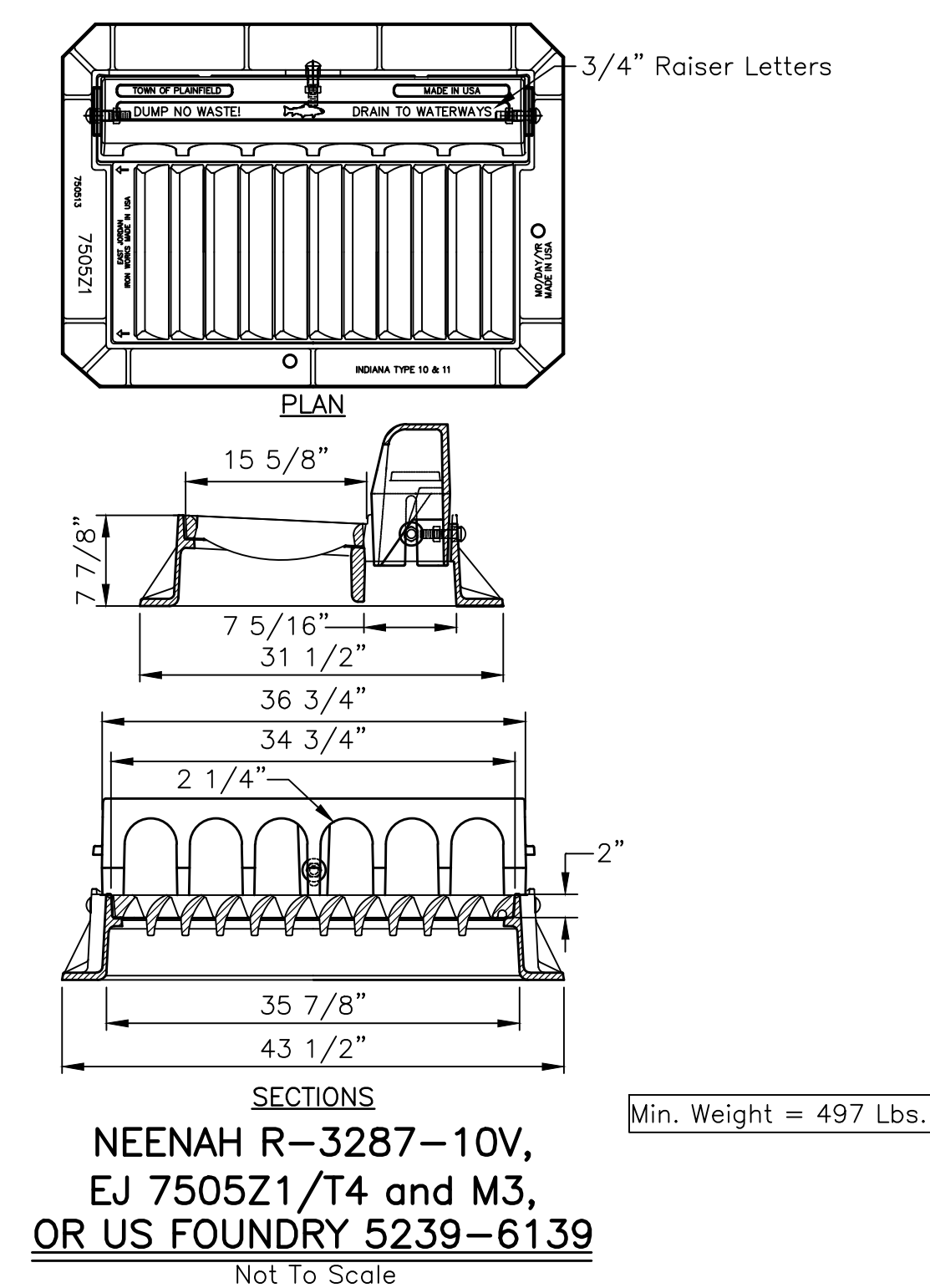
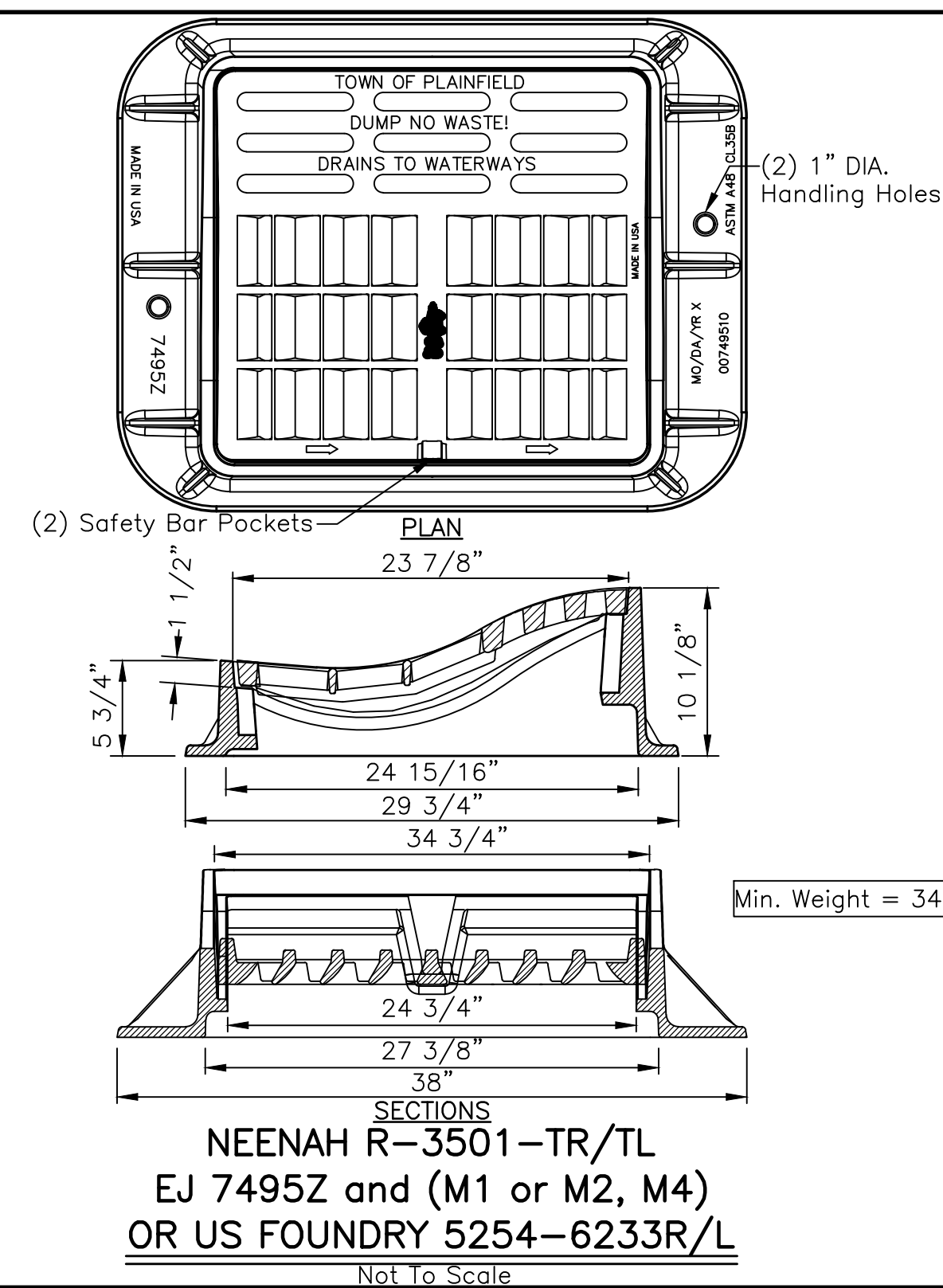
REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Lacy</i>	01/01/2015
DESIGN ENGINEER		DATE
APPROVED	<i>James J. B. B.</i>	01/01/2015
EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES		DATE
APPROVED	<i>James L. L.</i>	01/01/2015
DIRECTOR OF PUBLIC WORKS		DATE

TOWN OF PLAINFIELD	SHEET 10 OF 29
STORM SEWER DETAILS AND NOTES	



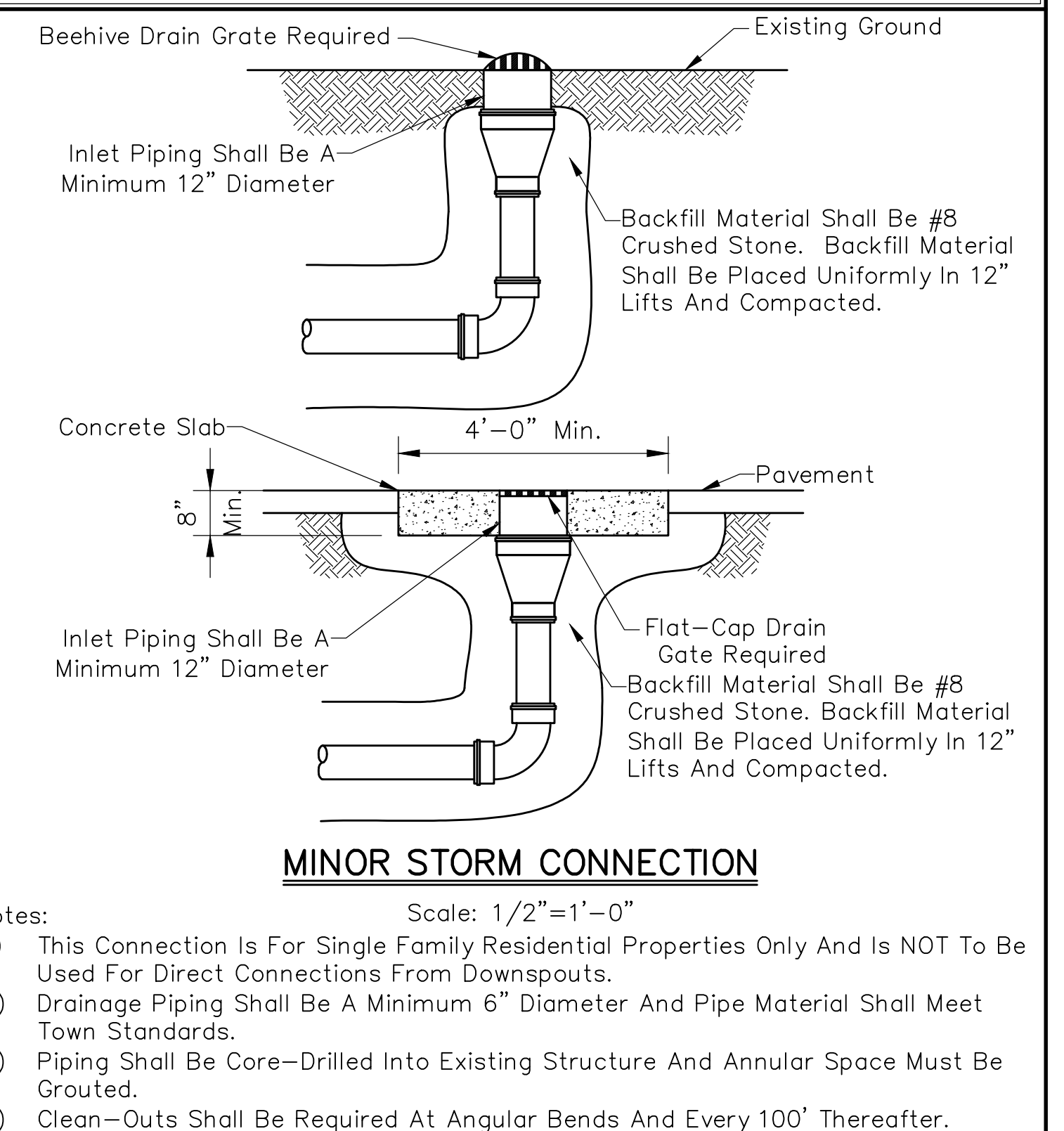
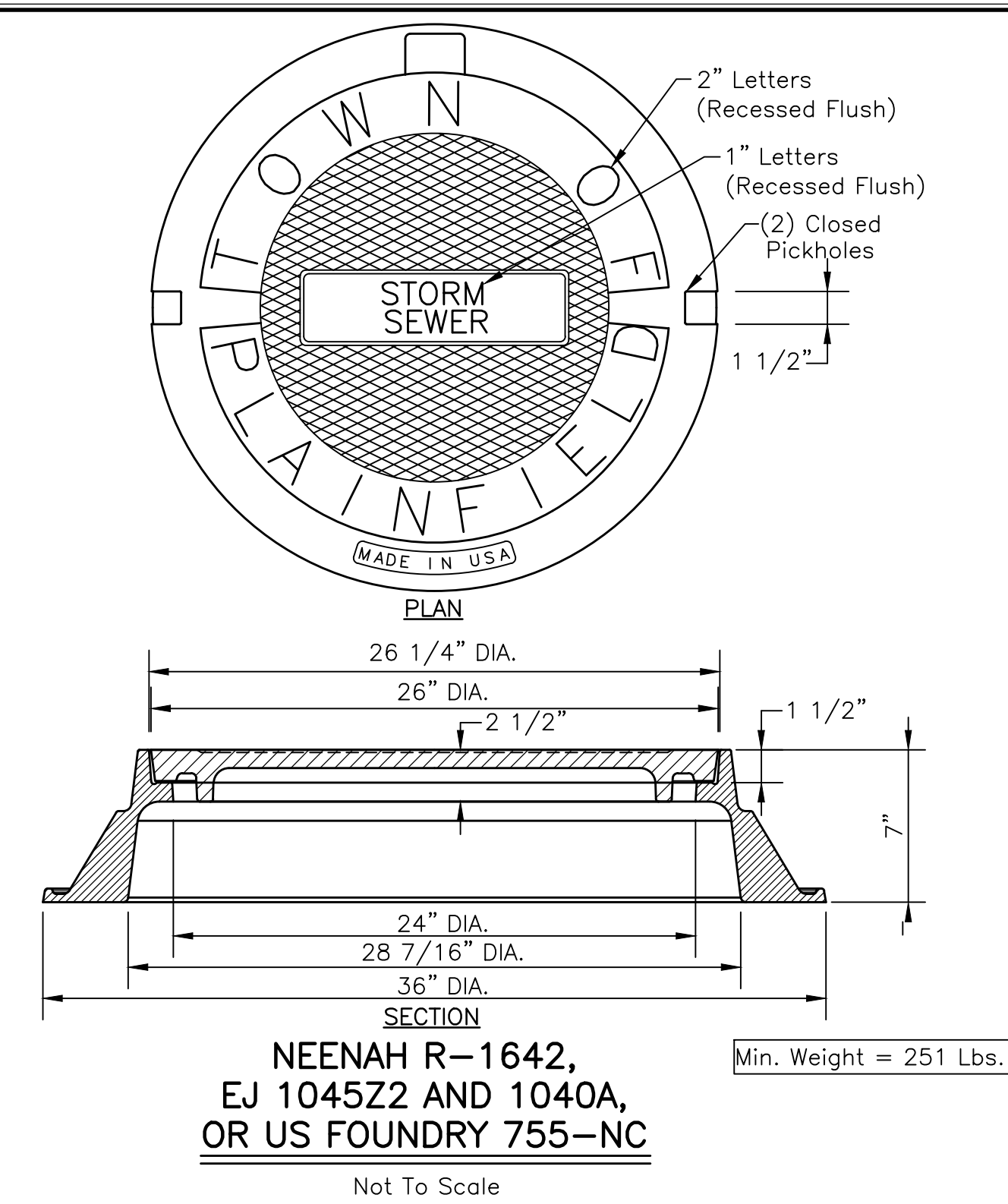
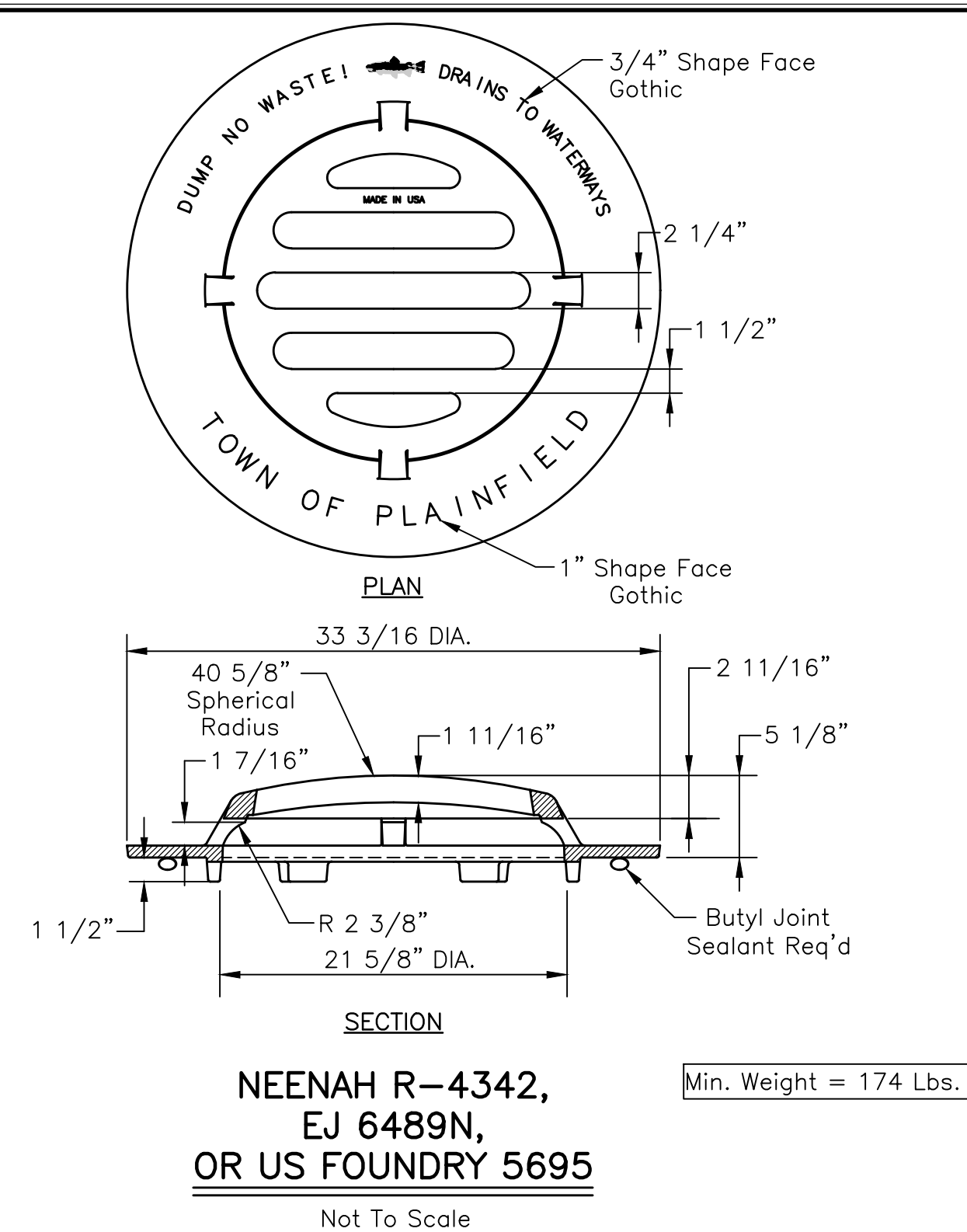
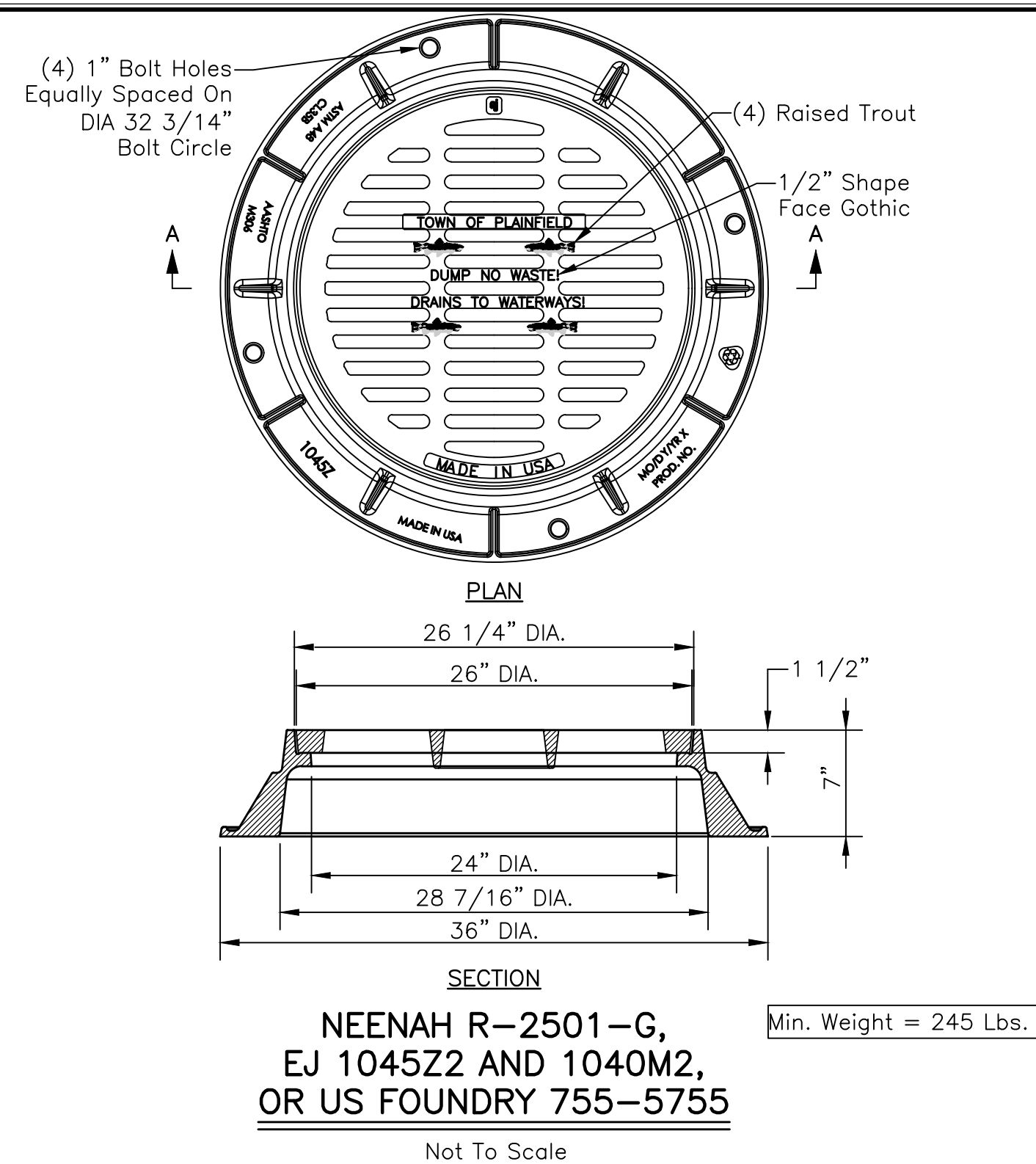


DEVELOPMENT STANDARD – DETAIL DS-D01

DEVELOPMENT STANDARD – DETAIL DS-D02

DEVELOPMENT STANDARD – DETAIL DS-D03

DEVELOPMENT STANDARD – DETAIL DS-D04



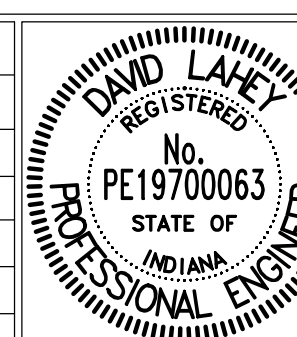
DEVELOPMENT STANDARD – DETAIL DS-D05

DEVELOPMENT STANDARD – DETAIL DS-D06

DEVELOPMENT STANDARD – DETAIL DS-D07

DEVELOPMENT STANDARD – DETAIL DS-D08

REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Luker</i> DESIGN ENGINEER	<i>01/01/2001</i> DATE
APPROVED	<i>Simon J. O'Broon</i> EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES	<i>01/01/2001</i> DATE
APPROVED	<i>Jean Carville</i> DIRECTOR OF PUBLIC WORKS	<i>01/01/2001</i> DATE

TOWN OF PLAINFIELD

## STORM DRAINAGE (D) DEVELOPMENT STANDARDS

SHEET  
11  
OF  
29



WATER MAIN MATERIALS

- 1.) All Pipe Provided For Use In The Town Of Plainfield Water System Shall Be Of U.S. Production Manufactured By American, U.S. Pipe, Or Town Approved Equal. All Fittings Provided For Use In The Town Of Plainfield Water System Shall Be Of U.S. Production Manufactured By U.S. Pipe, Tyler, American, Or As Approved By Plainfield DPW.
- 2.) Ductile Iron Pipe For Water Mains Shall Be Centrifugally Cast And Shall Conform To The Latest Revision Of ANSI A21.5 And AWWA C151. Ductile Iron Pipe With Push-On Or Mechanical Joints Shall Be Special Thickness Class 50. The Pipe Shall Be Provided With A Minimum Laying Length Of 18 Feet.
- 3.) Ductile Iron Fittings 3 Inches Through 48 Inches Shall Conform To The Latest Revision Of ANSI A21.10 And AWWA C110. Ductile Iron Compact Fittings 3 Inches Through 16 Inches Shall Conform To The Latest Revision Of ANSI A21.53 And AWWA C153. Fittings In, And Within 2 Feet Of, Structures Shall Be Flanged. All Other Fittings Shall Be Mechanical Joint Type.
- 4.) Ductile Iron Pipe Coatings Shall Conform To The Latest Revision Of ANSI A21.51, AWWA C151, ANSI A21.4, And AWWA C104. Interior Pipe Lining Shall Be Cementious Mortar With Asphaltic Seal Coat. Exterior Pipe Coating Shall Be Standard Asphaltic Coating, Except Exposed Piping Within Structures Shall Receive Shop Priming Compatible With Finish Coat.
- 5.) Mechanical Joints And Accessories Shall Conform To The Latest Revision Of ANSI A21.10 And AWWA C110. Rubber Gaskets Shall Be Vulcanized Synthetic Rubber And Shall Conform To The Latest Revision Of ANSI A21.11 And AWWA C111.
- 6.) Flanged Joints Shall Conform To The Latest Revision Of ANSI A21.15 And AWWA C115. Rubber Gaskets Shall Be Either Ring Or Full Face And Shall Be 1/8" Thick. Bolts And Nuts Shall Conform To ANSI B18.2.1 And ANSI B18.2.2.
- 7.) Push-On Joints Shall Conform To The Latest Revision Of ANSI A21.11 And AWWA C111. Rubber Gaskets Shall Be Vulcanized Synthetic Rubber And Shall Conform To The Latest Revision Of ANSI A21.11 And AWWA C111.
- 8.) Service Tubing To Customer Shall Be Copper Water Tube, Type K, Soft Temper For 3/4" Through 2" For Underground Service, Conforming To ASTM B88, ASTM B251, And AWWA C800. Pipe Shall Be Marked With The Manufacturer's Name Or Trademark And Mark Indicative Of The Type Of Pipe. Outside Diameter Of The Pipe And Minimum Weight Per Foot Of Pipe Shall Not Be Less Than Listed In ASTM B251, Table II.
- 9.) Gate Valves Shall Be In Accordance With AWWA C515 Having Fused Epoxy Coating Inside And Outside Assembled With S.S. Bolts And Shall Be American Flow Control Series 2500. Consult Plainfield DPW For Valves Larger Than 16 Inches. Valves Shall Pass A 250 PSI Factory Test. Valve Boxes Shall Be Furnished With Posi-Caps To Align Box Over Stem.

WATER MAIN PRESSURE AND LEAKAGE TESTING

- 1.) The Town Of Plainfield Shall Be Given 24 Hour Written Notice Of The Required Pressure And Leakage Test To Be Performed By The Contractor. The Pressure And Leakage Test Shall Be Performed In Accordance With The Basic Provisions Of AWWA C600. The Testing Procedure Shall Assume A 100 PSIG Working Pressure. The Test Pressure Shall Not Be Less Than 1.25 Times The Working Pressure At The Highest Point Along The Test Section But Not Less Than 1.5 Times The Assumed Working Pressure At The Point Of Testing. Test Pressure Shall Not Exceed Pipe Or Thrust Restraint Design Pressures Or Rated Pressure Of The Valves. The Test Pressure Shall Not Vary By More Than +5 PSI For The 2 Hour Test Duration.
- 2.) Valves Shall Not Be Operated In Either Direction At Differential Pressures Exceeding The Rated Valve Working Pressure.
- 3.) The Pressure And Leakage Test Shall Be Performed Following The General Form Of The Following:

A. Record Time And Line Pressure Prior To Start Of Test.

B. Pump Water Into New Main Until Pressure Reaches 150 PSIG, Stop Pumping When Pressure Reaches 150 PSIG, Record Time And Line Pressure.

C. Contractor Shall Remain At Site For One Hour. The Test Shall Be Voided If Any Adjustments Are Made To The Main, Test Equipment, Or Appurtenances. Tightening Of Fittings On Test Equipment Is Allowed. Following The One Hour Period, Record Time And Line Pressure.

D. Pump Water Into New Main From A Calibrated Container Of Water Until Pressure Reaches 150 PSIG, Stop Pumping When Pressure Reaches 150 PSIG, Record Time, Line Pressure, And Amount Of Water Pumped To The Nearest 1/100 Gallon. The Calibrated Container Shall Have Markings At 1/10 Gallon Increments.

E. Repeat Steps C And D One Additional Time.
- 4.) A Test Section Of Water Main Is Considered Satisfactory If It Meets The Following:

Main Size (Inches)	Allowable Leakage (Gal./Hr./1000 Ft.)
6	0.50
8	0.66
10	0.83
12	0.99
16	1.32

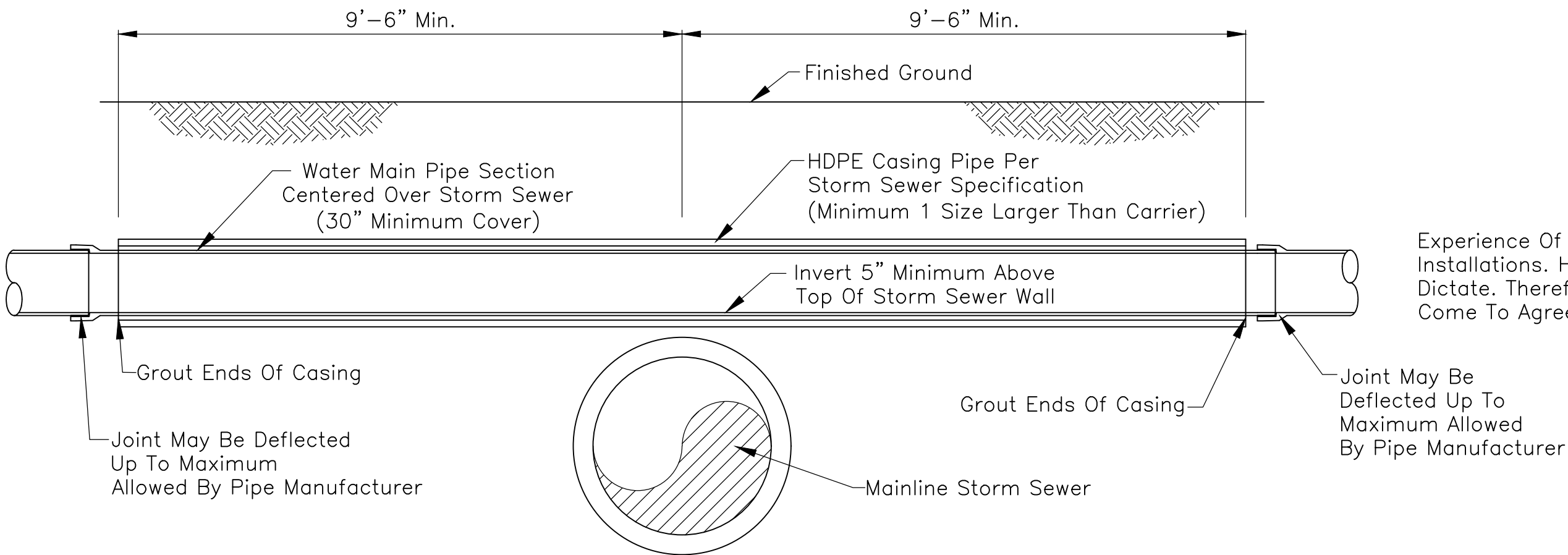
- 5.) If The Leakage From A Test Section Is Greater Than Permitted Under These Specifications, The Contractor Shall Locate And Repair The Defective Joints, Mains, And Appurtenances. The Pressure And Leakage Test Shall Then Be Repeated Until Satisfactory Results Are Obtained. All Labor And Materials Required To Meet The Requirements Of The Pressure And Leakage Test Shall Be At The Expense Of The Contractor.

WATER MAIN GENERAL NOTES AND AS-BUILT DRAWINGS

- 1.) Provide A Valve On All Runs And Branches Per The Connection Details On Sheet 12 Of The Plainfield Standards Even When Such Runs Or Branches Are Stubs For Future Extension.
- 2.) Storm Pipe Conflicts Require Special Attention In That Proposed Water Mains Shall Pass Over Proposed Mainline Storm Pipe. Such Situations May Require Upsizing Of Downstream Storm Pipes To Enable Flatter Slopes To The Point Of Conflict Such That 30 Inch Minimum Cover Is Maintained Over The Water Main. Vertical Water Main Fittings Shall Not Be Used. All Water Main Crossings Of Storm Pipe Shall Be Shown On Storm Sewer Profiles. When It Is Necessary To Decrease Water Main Cover To Less Than 54 Inches, Inlet Pipes That May Conflict With The Water Main Shall Be Laid At Such Slope To Pass Below The Water Main.
- 3.) Water Mains Shall Follow The Alignment Of The Road Centerline And Shall Remain 3 1/2 Feet Behind The Back Of Curb On One Side Of The Street Without Alternating From Such Side.
- 4.) All Water Pipe Shall Be Installed In Accordance With AWWA C600 And With A Minimum Depth Of Cover Of 54 Inches, Except As Provided By General Note No. 2.
- 5.) Terminate Dead End Mains With A Mainline Valve Followed By A Fire Hydrant Assembly. For Cul-De-Sacs, Eliminate Hydrant Assembly Tee And Terminate With 6" Valve And Fire Hydrant. As Directed Or Approved By The Plainfield DPW, Terminate Temporary Dead End Mains With A #2 Eclipse Post Hydrant With Tamper-Proof Options And Provide #492 Tamper-Proof Wrench With A Brass Street Elbow, Brass Nipple, Mueller B20283 Ball Curb Valve With Box, And A Brass Nipple Tapped Into Restrained Cap. See Development Standard DS-W05.
- 6.) Unless Unavoidable As Determined By Plainfield DPW, Double Tees Shall Not Be Permitted. Utilize A Cross At Intersection Of Four Water Mains With Cross Sized To Match The Largest Pipe.
- 7.) See Development Standard DS-W01 For Water Main Abandonment Procedure.
- 8.) As-Built Drawings Shall Be Submitted To Plainfield DPW. GPS Collected Coordinates Shall Depict Actual Horizontal And Vertical Locations Of Utility Assets Such As: System Valves, Hydrants, Blow-Offs, Air Release Valves, And Master Meters.

WATER MAIN DISINFECTION, BACTERIOLOGICAL TESTING



- 1.) The Town Of Plainfield Shall Be Given 24 Hour Written Notice Of The Required Disinfection, Flushing And Testing Procedures To Be Performed By The Contractor. All Newly Installed Water Mains Shall Be Disinfected In Accordance With ANSI/AWWA C-651. Liquid Chlorine, High-Test Calcium Hypochlorite (70 Percent Chlorine), Or High-Test Sodium Hypochlorite (14.7 Percent Chlorine) May Be Used To Provide An Initial Minimum Concentration Of 25 mg/L Of Free Chlorine In All Newly Installed Mains.
- 2.) A Minimum Concentration Of 10 mg/L Of Free Chlorine Shall Be Maintained In All Parts Of The Newly Installed Mains For 24 Hours Of Contact Time.
- 3.) Following The Initial 24 Hour Contact Time But Prior To 48 Hours Of Contact Time, All Treated Water Shall Be Properly Dechlorinated and Thoroughly Flushed From The Newly Laid Pipe At Its Extremity Until The Replacement Water Has A Chlorine Residual Of Less Than 1 mg/L.
- 4.) After Flushing, Water Samples Collected On Two Successive Days From The Treated Piping System, As Directed By The Town Of Plainfield, Shall Show Satisfactory Bacteriological Tests. Following Satisfactory Bacteriological Tests, Contractor Shall Submit 2 Copies Of The Results To Plainfield DPW And To IDEM Drinking Water Branch.
- 5.) The Taking Of Samples And The Testing Of Chlorine Residual Shall Be Carried Out By The Contractor At The Direction Of The Town Of Plainfield.

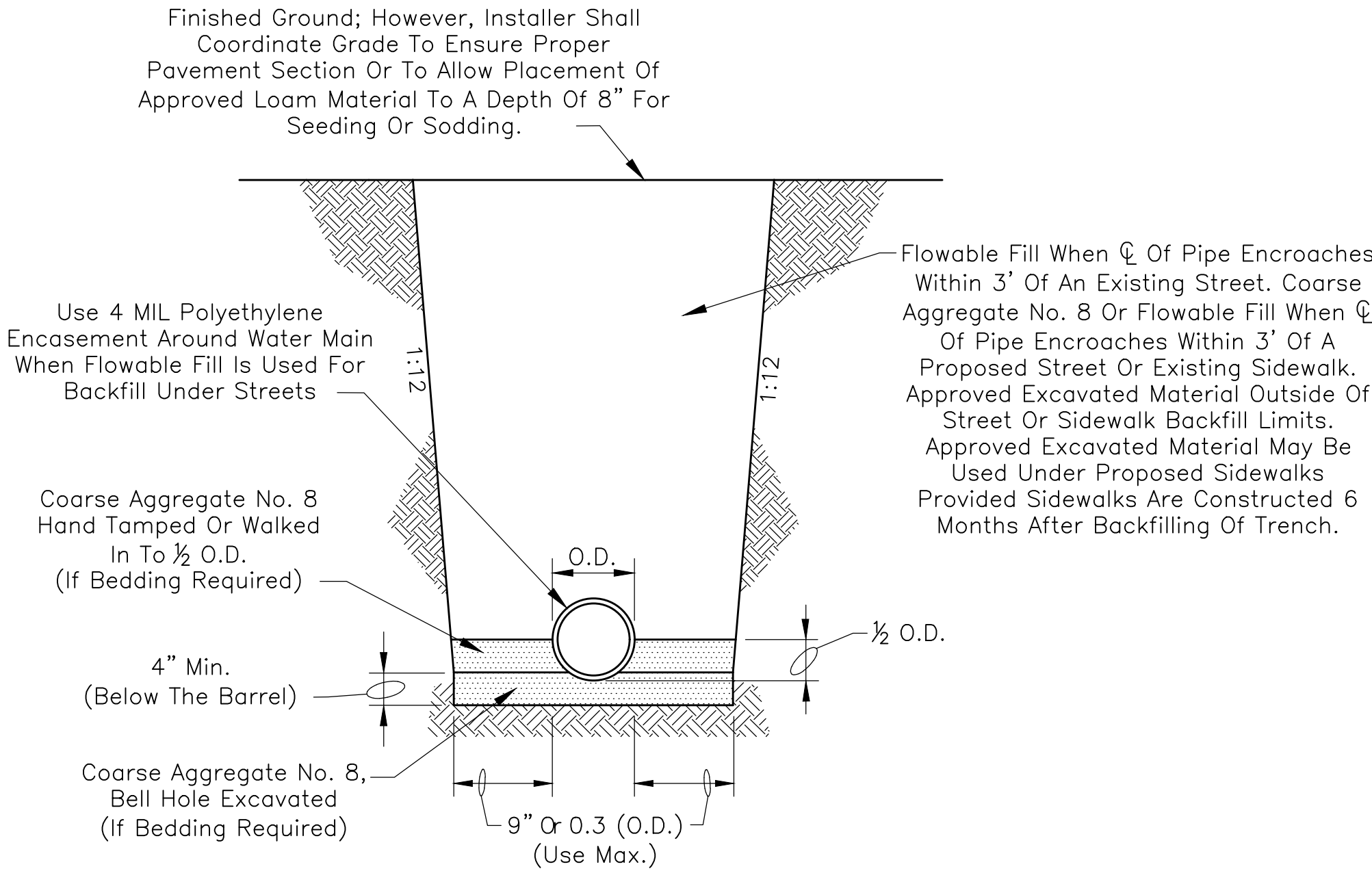


- 1.) HDPE Casing Pipe Required At All Water Main/Sewer Crossings Which Violate The 18 Inch Minimum Vertical Separation Required By Ten States Standards. Refer To Water Main General Notes.
- 2.) Fittings May Be Required To Reduce The Overall Length Of Shallower Than Normal Water Main As A Part Of The Special Storm Sewer Conflict Treatment.

SPECIAL STORM SEWER CONFLICT TREATMENT

Scale: None

REVISIONS				RECOMMENDED FOR APPROVAL			TOWN OF PLAINFIELD		SHEET	
Rev. No.	Description	Date					DESIGN ENGINEER			12
				APPROVED			EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES			OF
				APPROVED			DIRECTOR OF PUBLIC WORKS			29



D.I. PIPE BEDDING TABLE		
Pipe Size (in)	Bedding Depth Below Barrel (in)	Cubic Yards Of Bedding Per Foot Of Pipe (cys/ft)
4	4	0.038
6	4	0.053
8	4	0.070
10	4	0.088
12	4	0.108
14	4	0.131
16	5	0.164
18	5	0.191
20	6	0.230
24	7	0.306
30	8	0.432

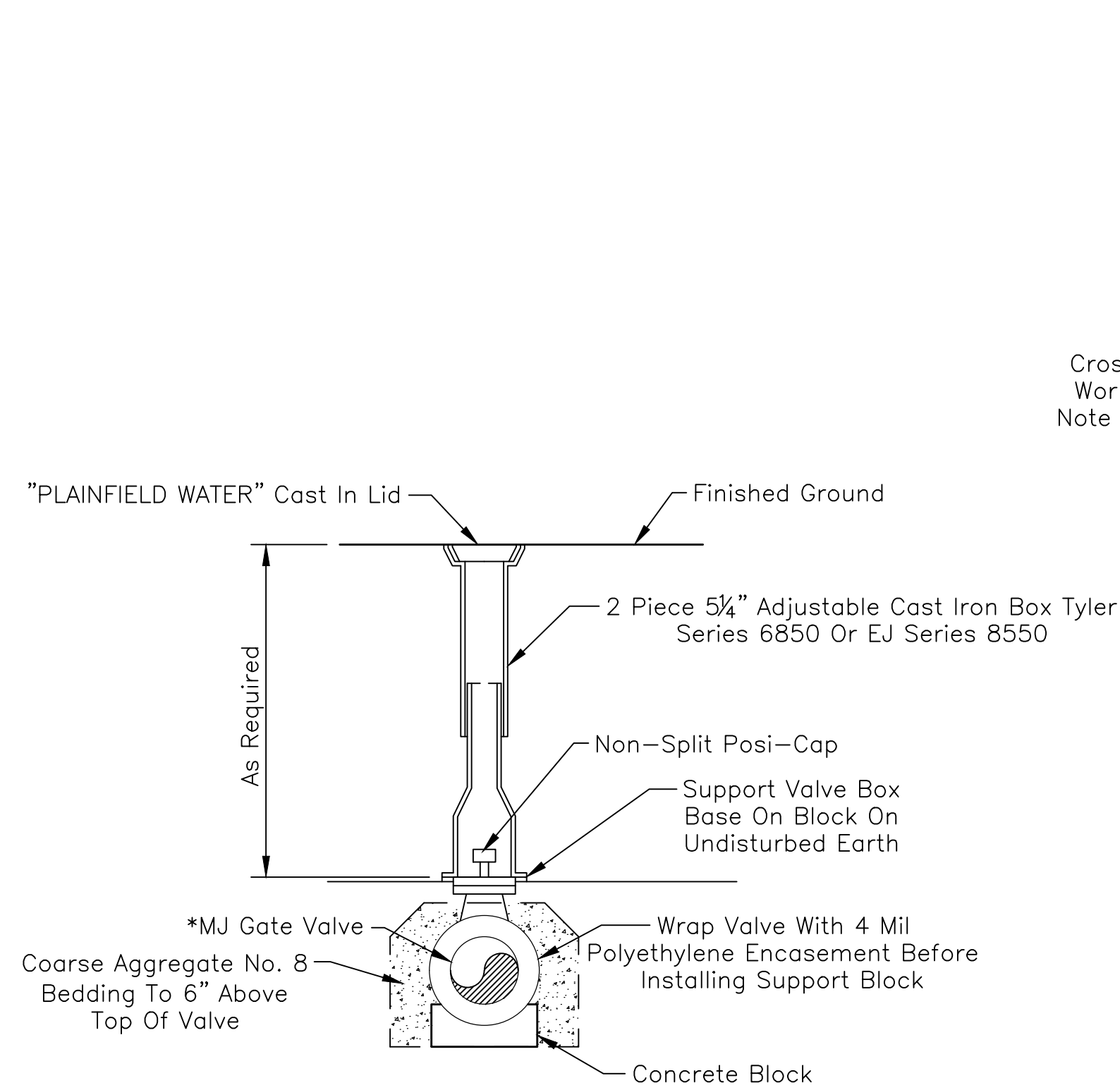
Note:  
Bedding Is Still Required Where Pipe Requires Structural Backfill (Flowable Fill, Coarse Aggregate No. 8, Etc.)

Experience Of Plainfield DPW Has Been That Bedding Has Not Been Required For Nearly All Installations. However, Plainfield DPW May Direct That Bedding Is Required As Trench Conditions Dictate. Therefore, It Is Recommended That The Project Owner And The Project Contractor Come To Agreement On Payment Methods In The Event That Bedding Is Required.

DI PIPE BEDDING DETAIL

Scale: None

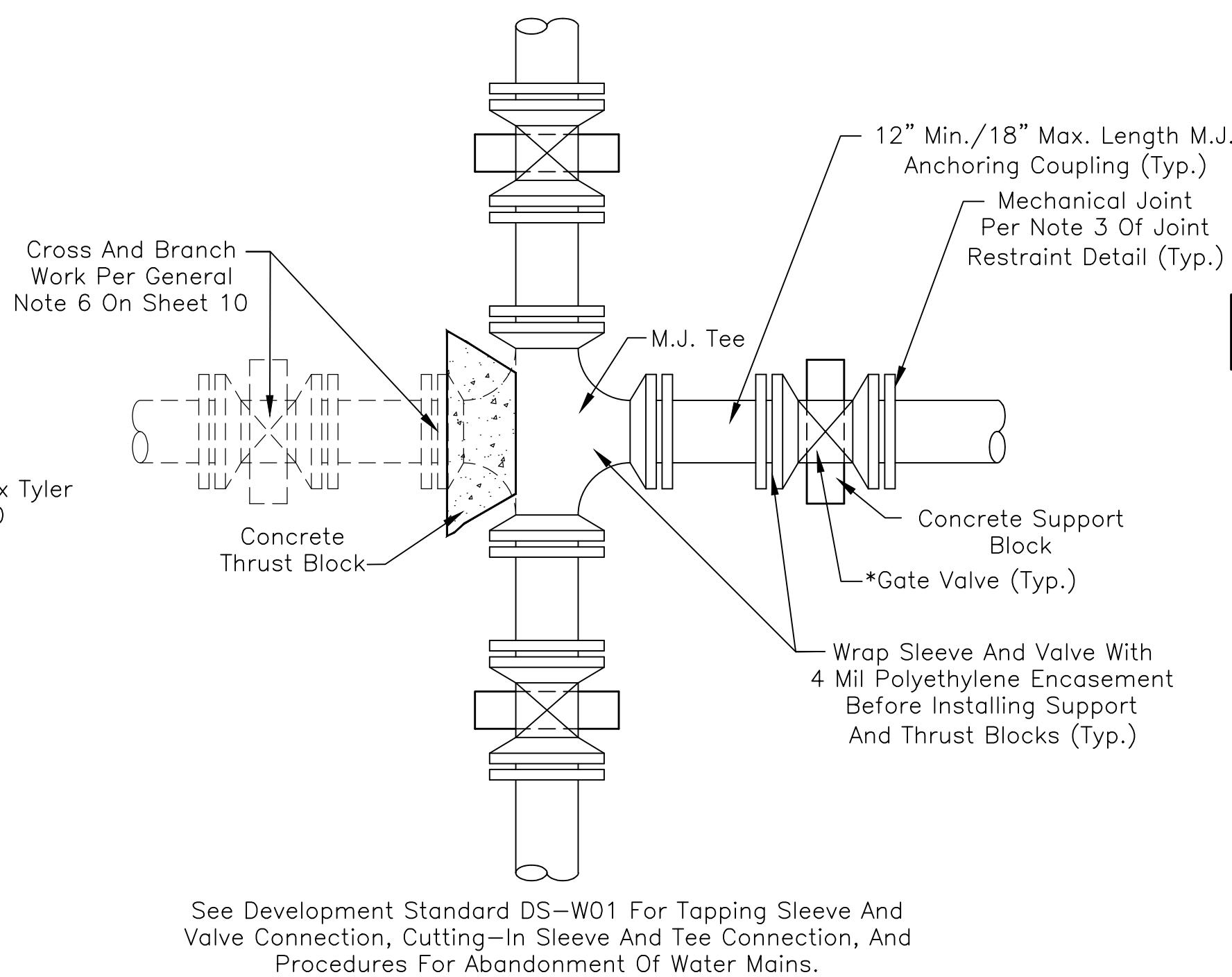




**TYPICAL VALVE INSTALLATION DETAIL**

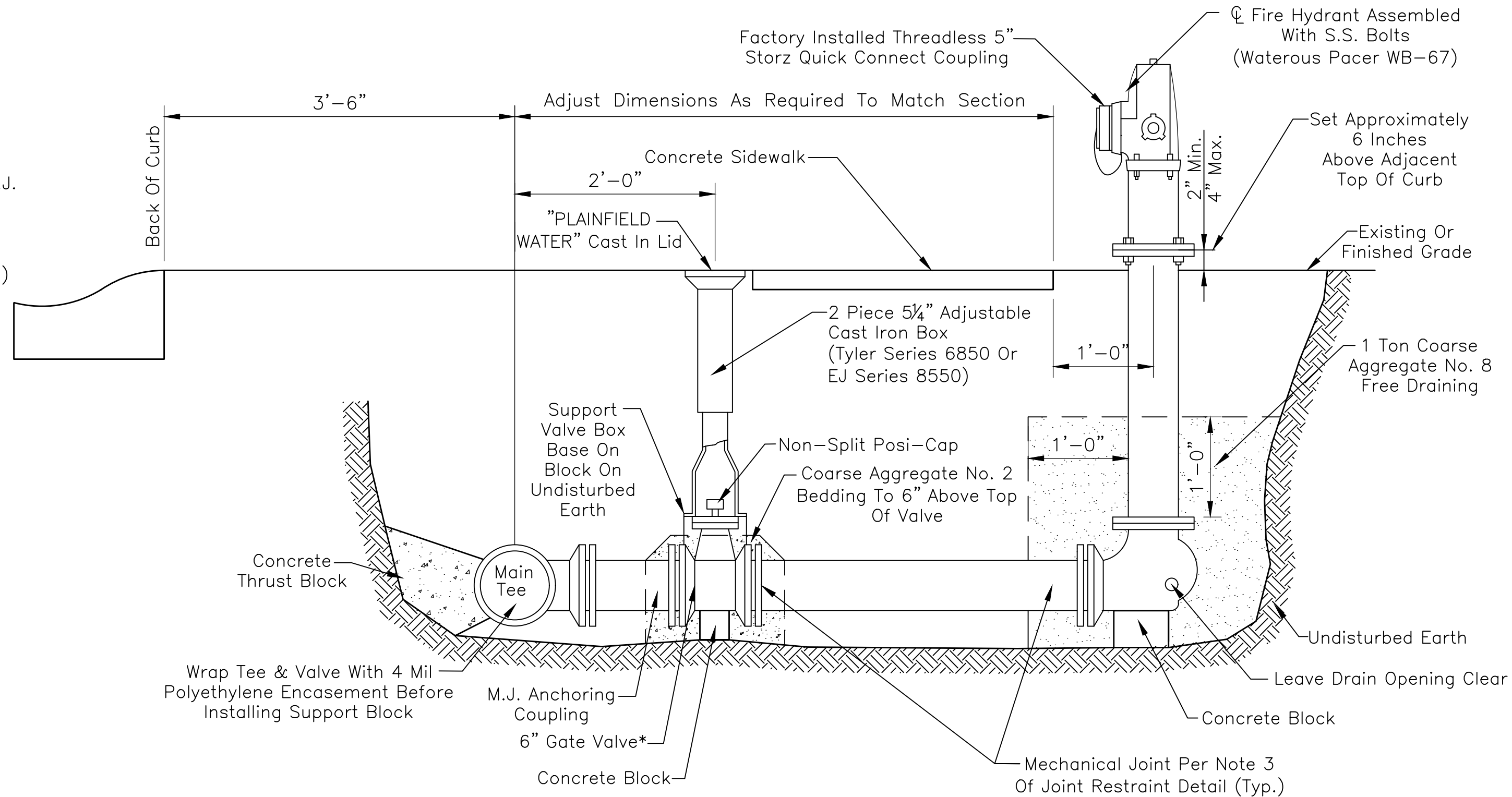
Scale: None

All Gate Valves Shall Be American Flow Control Series 2500 Assembled With Factory Installed Stainless Steel Bolts & With Coarse Aggregate No. 8 Bedding To 6" Above Top Of Valve.



**STANDARD NEW WORK BRANCH CONNECTION**

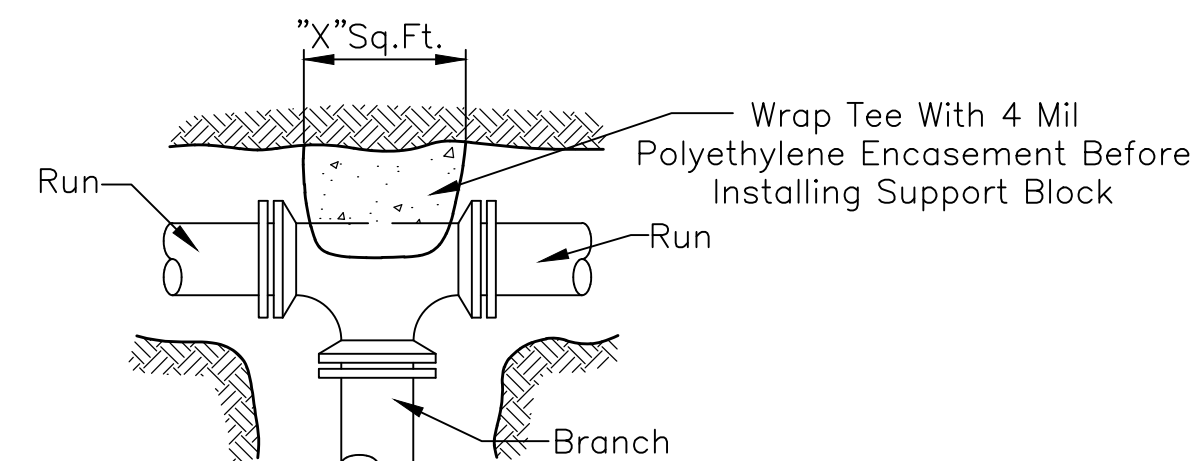
Scale: None



Note: Fire Hydrants Shall Be Painted Safety Yellow If Placed Behind Private Distribution Vault

**TYPICAL HYDRANT INSTALLATION DETAIL**

Scale: None



**TEE OR DEAD-END CROSS**

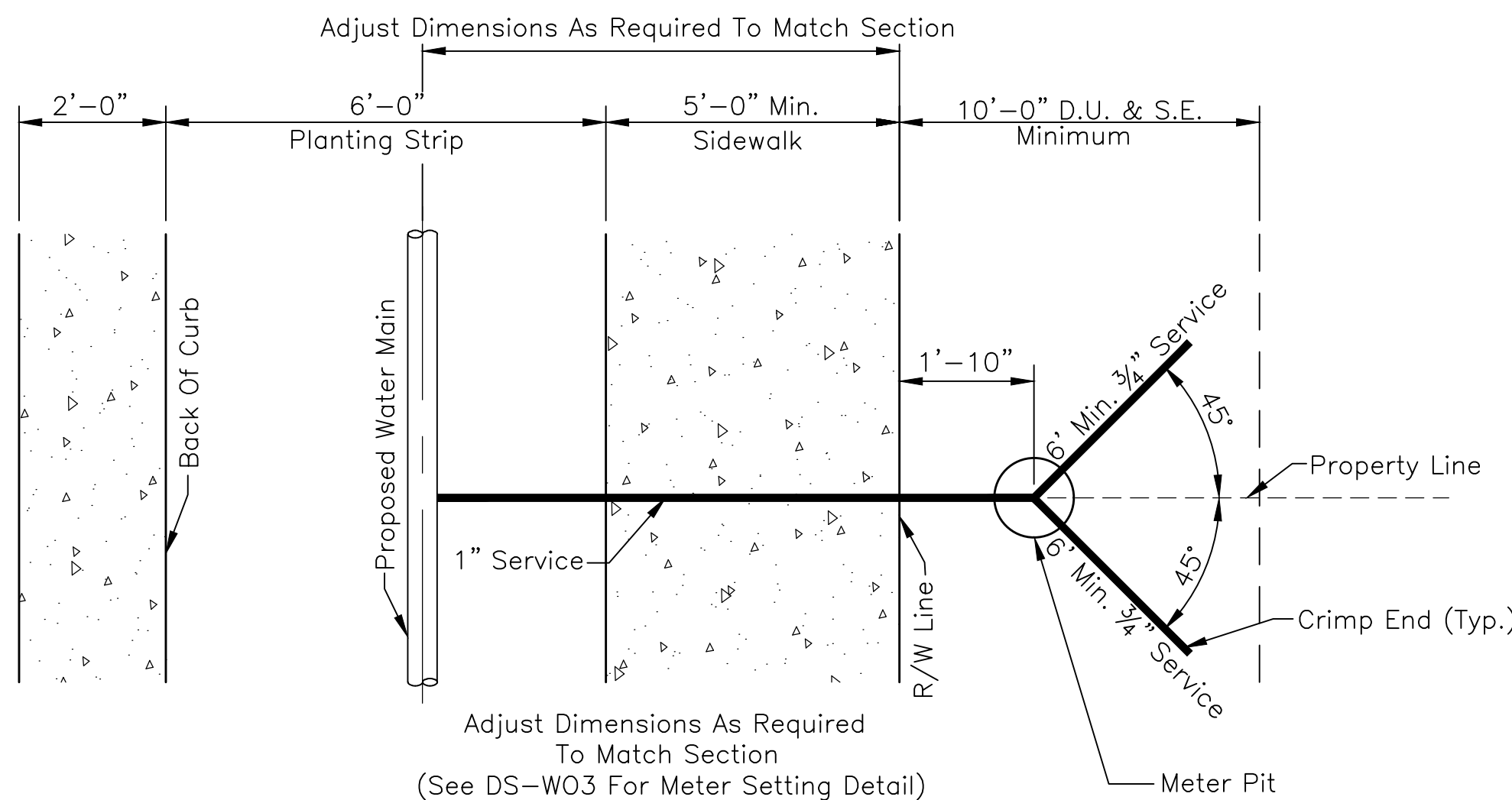
MINIMUM LENGTH OF RESTRAINED JOINT D.I. PIPE (WITHOUT POLY WRAP) EACH SIDE OF FITTING (FEET)				
PIPE SIZE	6"	8"	12"	16"
Tee Including Thrust Block (See Note 4)	15	20	28	36
Horizontal 90° OR Vertical 45° Down	15	20	28	36
Horizontal 45° OR Vertical 22½° Down	6	8	11	15
Horizontal 22½° OR Vertical 11¼° Down	3	4	6	7
Horizontal 11¼°	2	2	3	3
Dead End	27	35	50	65

**NOTES:**

- Length Of Restraint Measured From Centerline Of Fitting Requiring Restraint. Length Of Restraint For Vertical Bends Up Are Equal To That For Horizontal Bends.
- Length Of Restraint Based Upon 4'-6" Cover, 150 PSI Pressure, And ASTM D2487 Soil Types CL, ML, SC, SM, SP, SW, GC, GM, GP, & GW. For Less Cover, Higher Pressure, Or ASTM D2487 Soil Types PT, OH, CH, MH, & OL, Consult Plainfield DPW.
- Restraint To Be Accomplished With Field-Lok Gasket As Manufactured By U.S. Pipe Or Fast-Grip Gaskets As Manufactured By American For Push-On Joints, Anchoring Coupling For Valves And Adjacent Tees, Romac Grip Ring For All Mechanical Joints, Or As Approved By Plainfield DPW. Romac Grip Ring May Be Deleted On The Runs Of Hydrant Tees Unless A Mainline Valve Is Within 18 Feet Of The Hydrant Tee Or Unless Hydrant Tee Is Within Another Fitting's Restraint Length. All Restraints Shall Be Of U.S. Production.
- Tees And Dead-End Crosses Require Concrete Thrust Blocks In Addition To Branch Restraint Length, "X" Area For Concrete Thrust Blocks Per Detail Shall Be As Follows; 2, 4, 6, And 10 Square Feet For 6, 8, 12, And 16 Inch Pipe, Respectively. Other Than Restraint Of MJ Fittings Adjacent To Tee, No Run Restraint Length Is Required.

**JOINT RESTRAINT DETAIL**

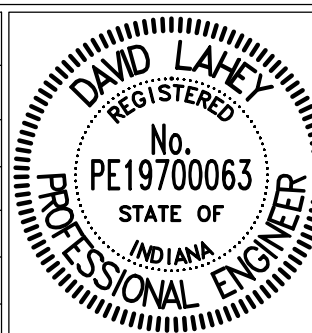
Scale: None



**TYPICAL DUAL METER SETTING DETAIL**

Scale: None

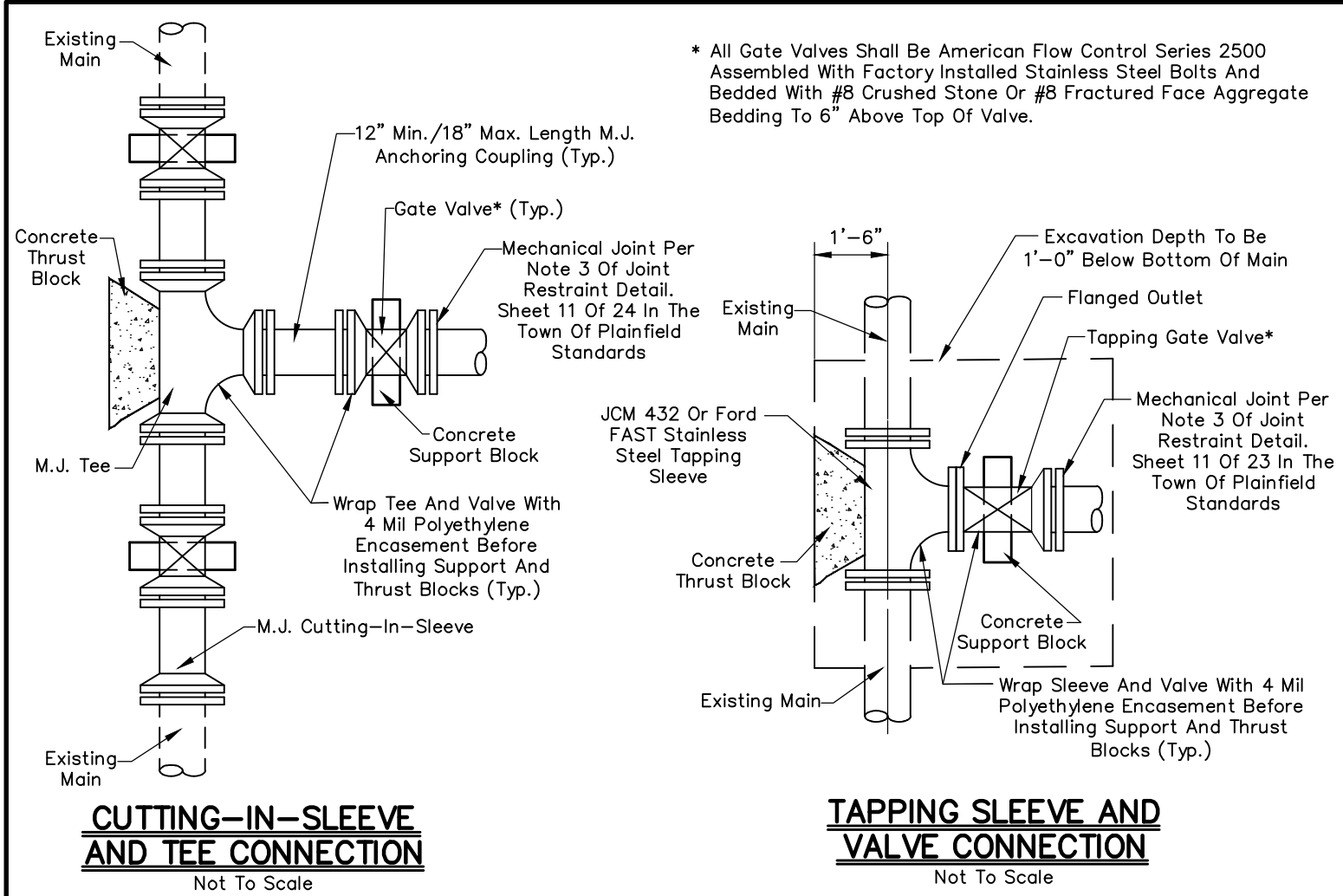
REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Lacey</i>	01/01/2015
DESIGN ENGINEER		DATE
APPROVED	<i>James J. Brou</i>	01/01/2015
EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES		DATE
APPROVED	<i>Jason Caruth</i>	01/01/2015
DIRECTOR OF PUBLIC WORKS		DATE

TOWN OF PLAINFIELD	SHEET
13	OF
29	
WATER MAIN DETAILS & NOTES	



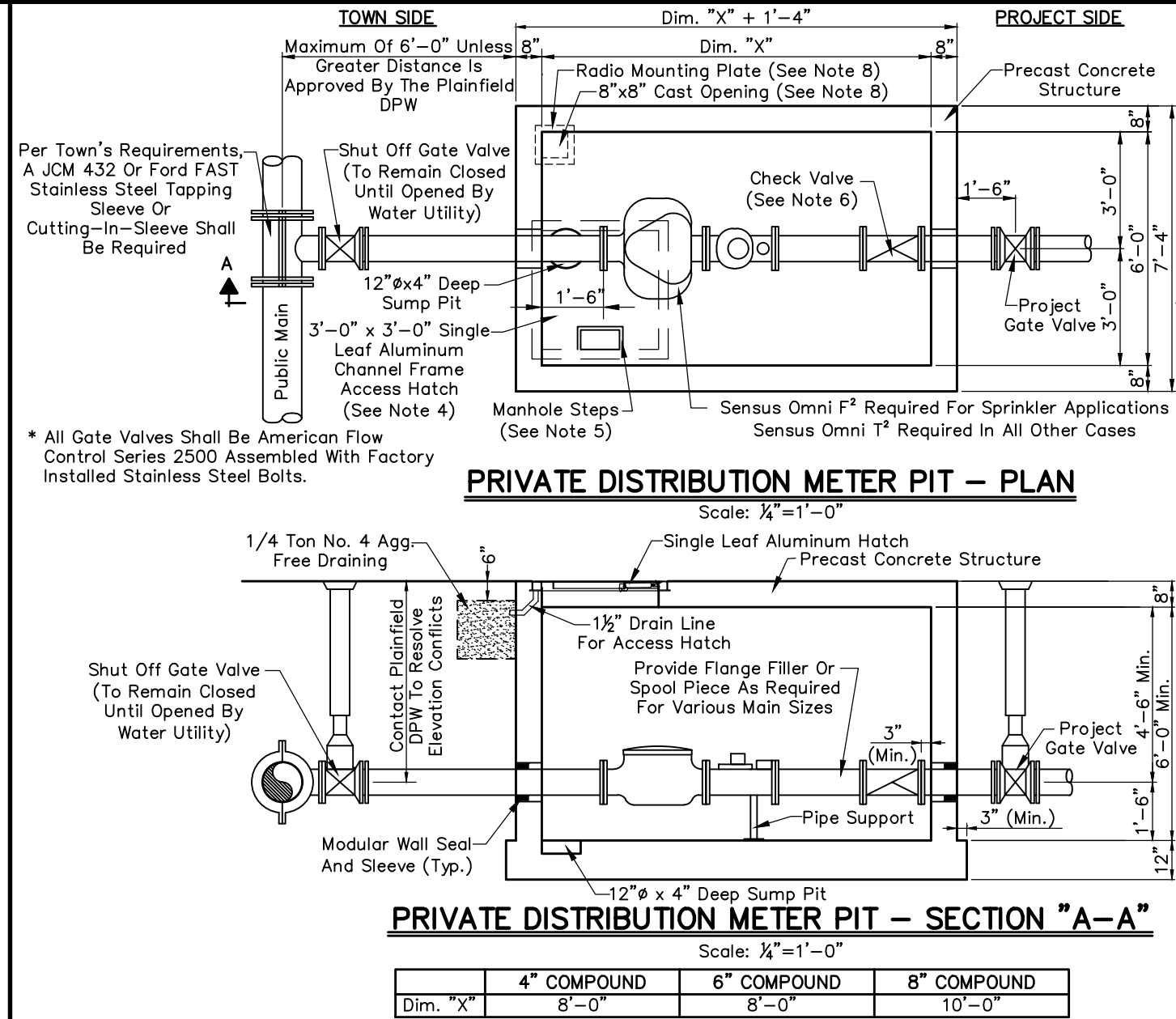


#### WATER MAIN MATERIALS

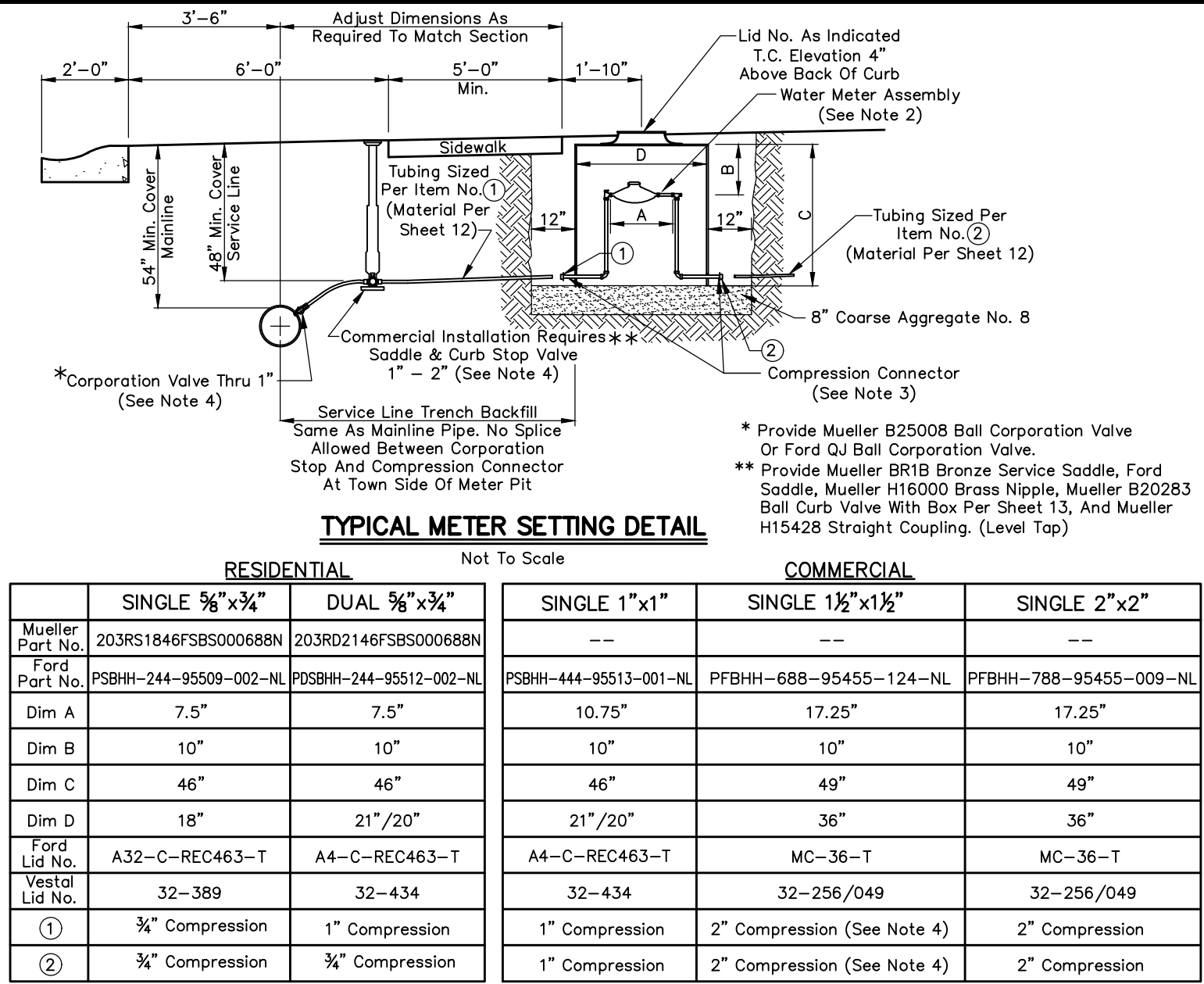
- All Pipe Provided For Use In The Town Of Plainfield Water System Shall Be Manufactured By American, Griffith, U.S. Pipe, Or As Approved By Plainfield DPW. All Fittings Provided For Use In The Town Of Plainfield Water System Shall Be Manufactured By Clow, Tyler, American, Or As Approved By Plainfield DPW.

#### WATER MAIN ABANDONMENT PROCEDURES

- In Cases Where An Existing Main Line Is To Be Abandoned, The Abandonment Shall Include The Complete Removal Of The Tee & Replacement With Sleeve And Pipe As Required.
- In Cases Where An Existing Service Line Is To Be Abandoned, The Abandonment Shall Include The Complete Removal Of The Service Line To Corporation Stop & Placement Of A Brass Cap At The Stop.

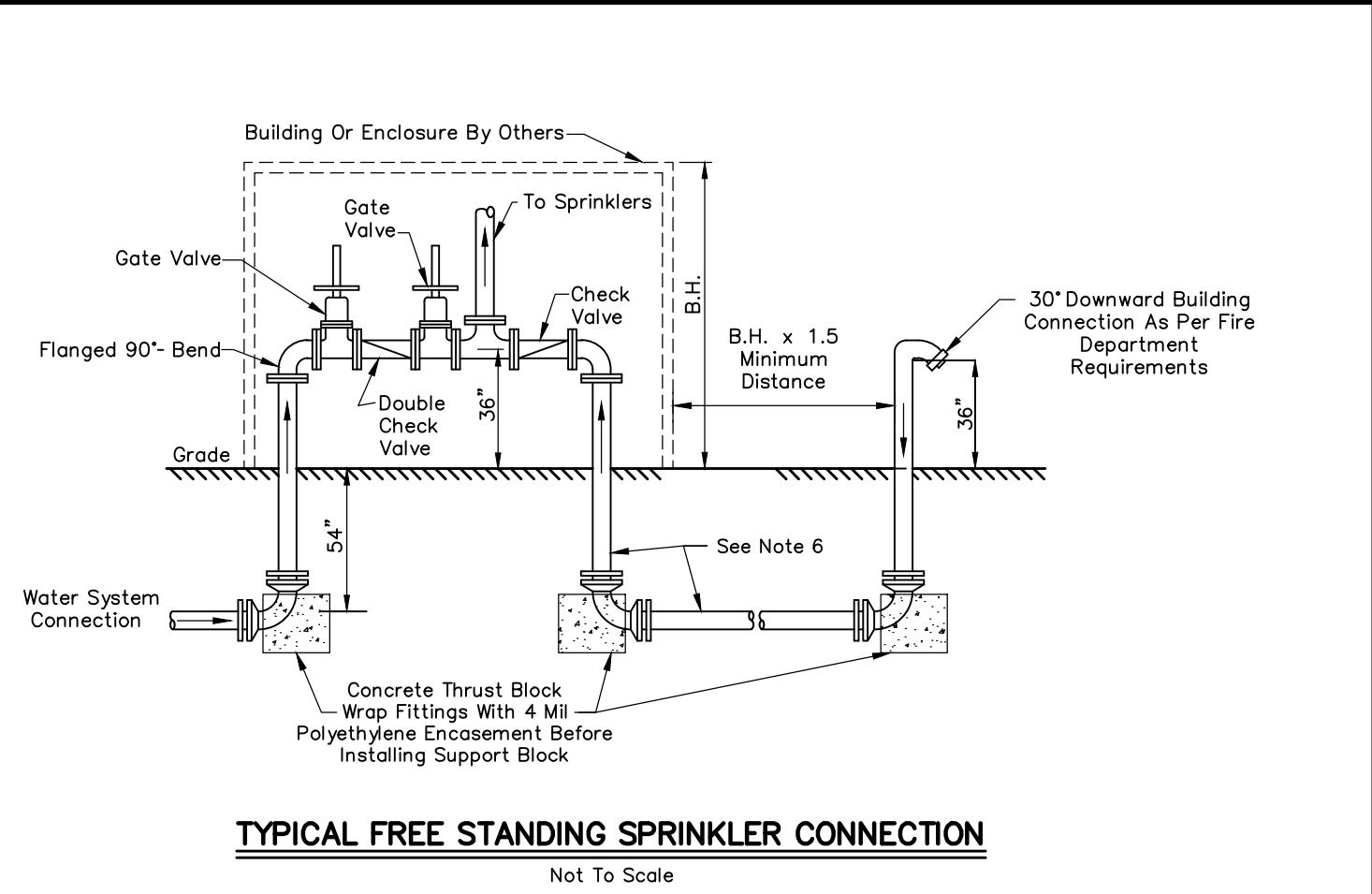


- Provide Minimum #4 Rebars On 12" Centers, Each Way In Top, Bottom, And All Sides.
- All Labor And Materials Including Meters And Radios To Be Provided By The Developer.
- Piping, Valves, Double Detector Check Valve Assembly, And Meter Shall Be Same Size Throughout. Other Than Dim. "X", Meter Pit Dimensions Shall Be Maintained For 4", 6", And 8" Mains.
- Access Hatch Shall Be On Meter Side Of Pit And Shall Be Bico Model J-44L With Drain Coupling Or As Approved By Plainfield DPW.
- Manhole Steps Shall Be Neenah R-1981-J, M.A. Industries PS-1-PF, Or As Approved By Plainfield DPW.
- Check Valve Shall Be Bronze Seated And Shall Be Provided With Bolted Covers For Easy Access To The Discs. Valve Shall Be Outside Adjustable Weight And Lever As Mueller A-2600-6-01, Kennedy/Clow 1106LW, Or As Approved By The Plainfield DPW.
- Piping In And Within 2 Feet Of Meter Pit Shall Be Class 53 Flanged Ductile Iron Pipe. Transition To Class 50 At A Mechanical Joint.
- Provide 1/4"x12"x12" Steel Plate, Point Plate With 5 Mils DFT Tnemec 74-ANSI No. 61 Gray Over 3 Mils DFT Tnemec 90-97 Primer. Center Plate Over 8"x8" Opening. Secure Plate With Four 1/4"x304 S.S. Anchor Bolts, Nuts, And Washers.



#### METER SETTING NOTES:

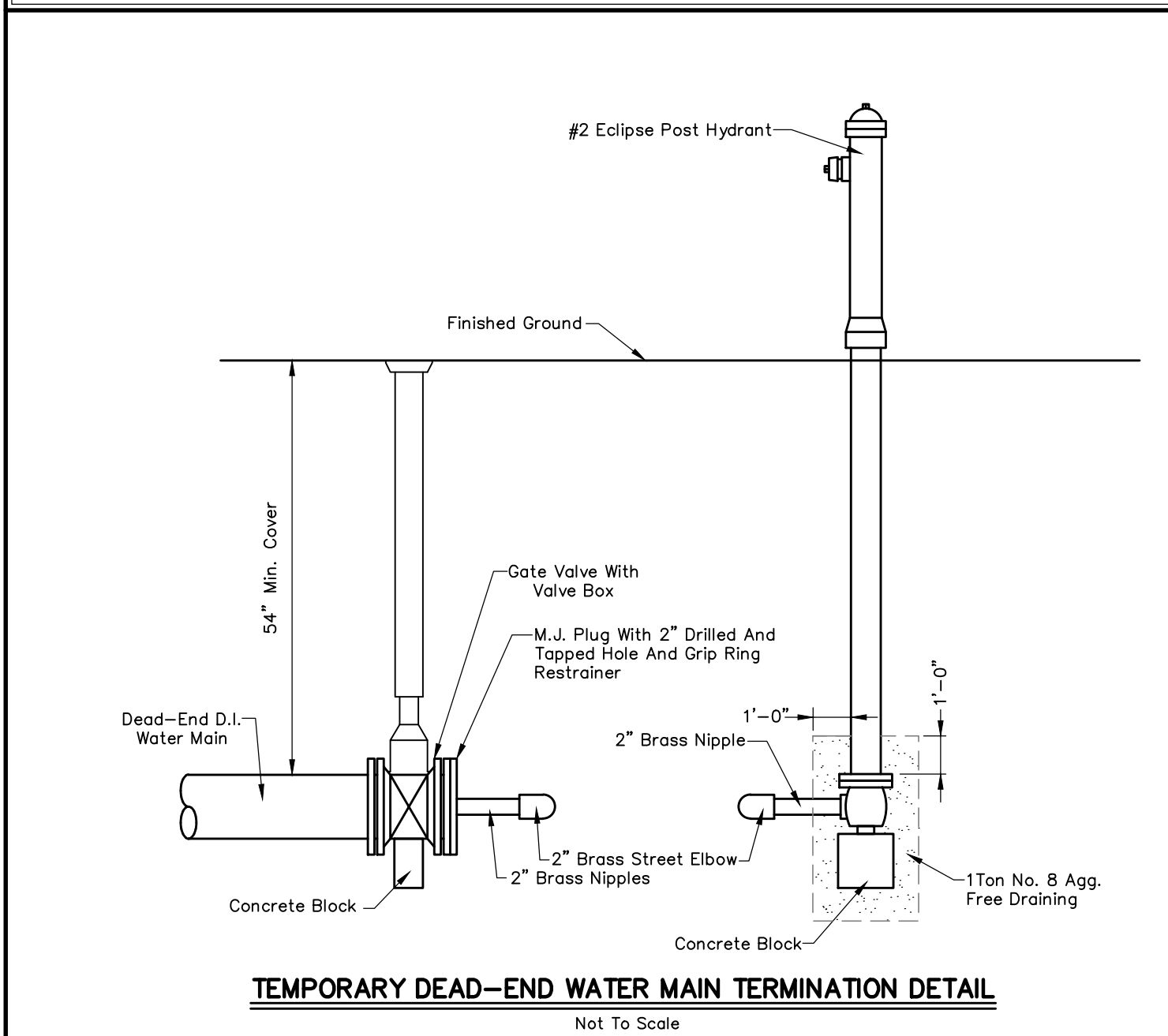
- Residential Construction Requires The Use Of Dual Meter Installations Whenever Possible. The Meter Box Shall Not Be Installed In A Driveway.
- Water Meter Assembly Shall Consist Of Sensus 5/8" x 3/4" Meter, Yoke Valve On The Supply Side Of The Meter, And Ames LF2000B Double Check Assembly On The Customer Side Of The Meter. Meters For Residential Use Provided By Town, All Other Meters, Regardless Of Use, Provided By User.
- The Contractor Shall Make All Tubing Connections Utilizing Mueller Or Ford Quick Compression Connectors.
- Tubing May Be Used 1 1/2" Or 2" For Such 1 1/2" Meter. Provide Reducing Fittings As Required.
- Residential Meters Require A Radio Read System. Commercial Meters And Radios Are Not Provided By The Town.
- See Development Standards For Private Distribution Meter Pit And Procedures For Abandonment Of Service Lines.
- Meter Lid Adjustment Shall Be Accomplished With Adjusting Rings Manufactured By Mueller Or Ford. The Maximum Adjustment Shall Be 4 Inches.



#### NOTES:

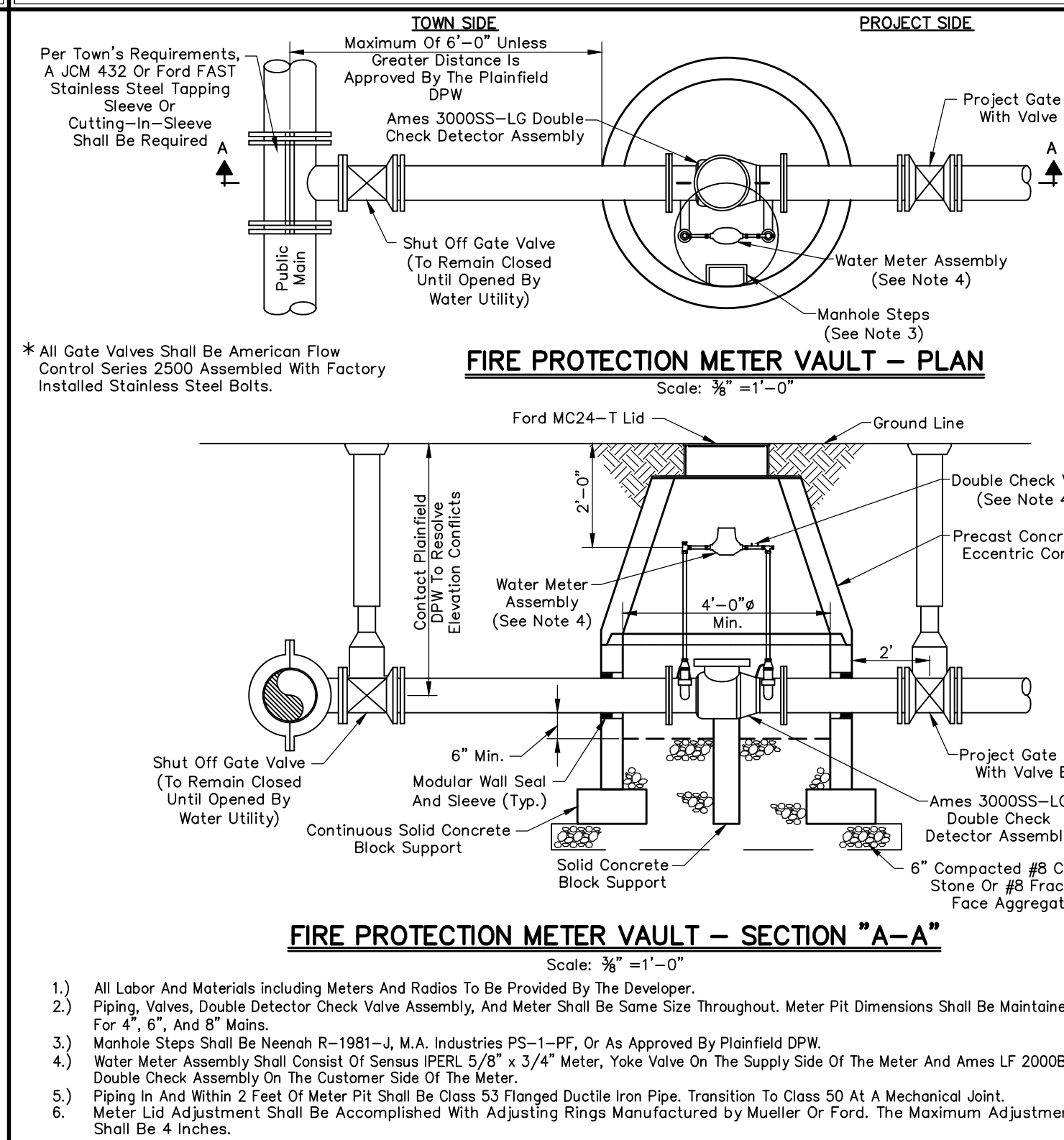
- The Connection Shall Be Within 150', As Measured On A Paved Roadway, Of A Fire Hydrant Supplied By The Public Water Main. The Connection And Hydrant Location Shall Be Such That From The Hydrant To A Fire Department Pump, And From The Pump To The Free Standing Sprinkler Connection, Access To Premises Will Not Be Blocked.
- Underground Pipe Shall Be Designed And Constructed As Required For An Underground Fire Main Using NFPA 24. Design Shall Allow For Water To Drain After Use.
- The Free Standing Sprinkler Connection Shall Be A 5" Threadless 45° Downward Connection.
- The Free Standing Sprinkler Connection Shall Be Painted Red, And Labeled "SPRINKLER CONNECTION" Or "STANDPIPE CONNECTION" With The Address Served Displayed.
- Where The Connection Is Subject To Vehicular Damage, The Connection Shall Be Protected. Protection Components Shall Not Be Closer Than 36" To The Connection And Shall Not Interfere With The Operation Of The Connection.
- Fire Department Connection (FDC) Pipe Diameter Shall Not Be Reduced At Any Point In The FDC Line From The Point Of Connection At The Base Of The Sprinkler/Standpipe Riser (BGR) To The Point Of Attachment Of The 5 Inch Storz Coupling.
- Underground Piping For Free Standing Sprinkler Connection Shall Conform To All Water Main Material Notes On Sheet 12.

### DEVELOPMENT STANDARD - DETAIL DS-W01



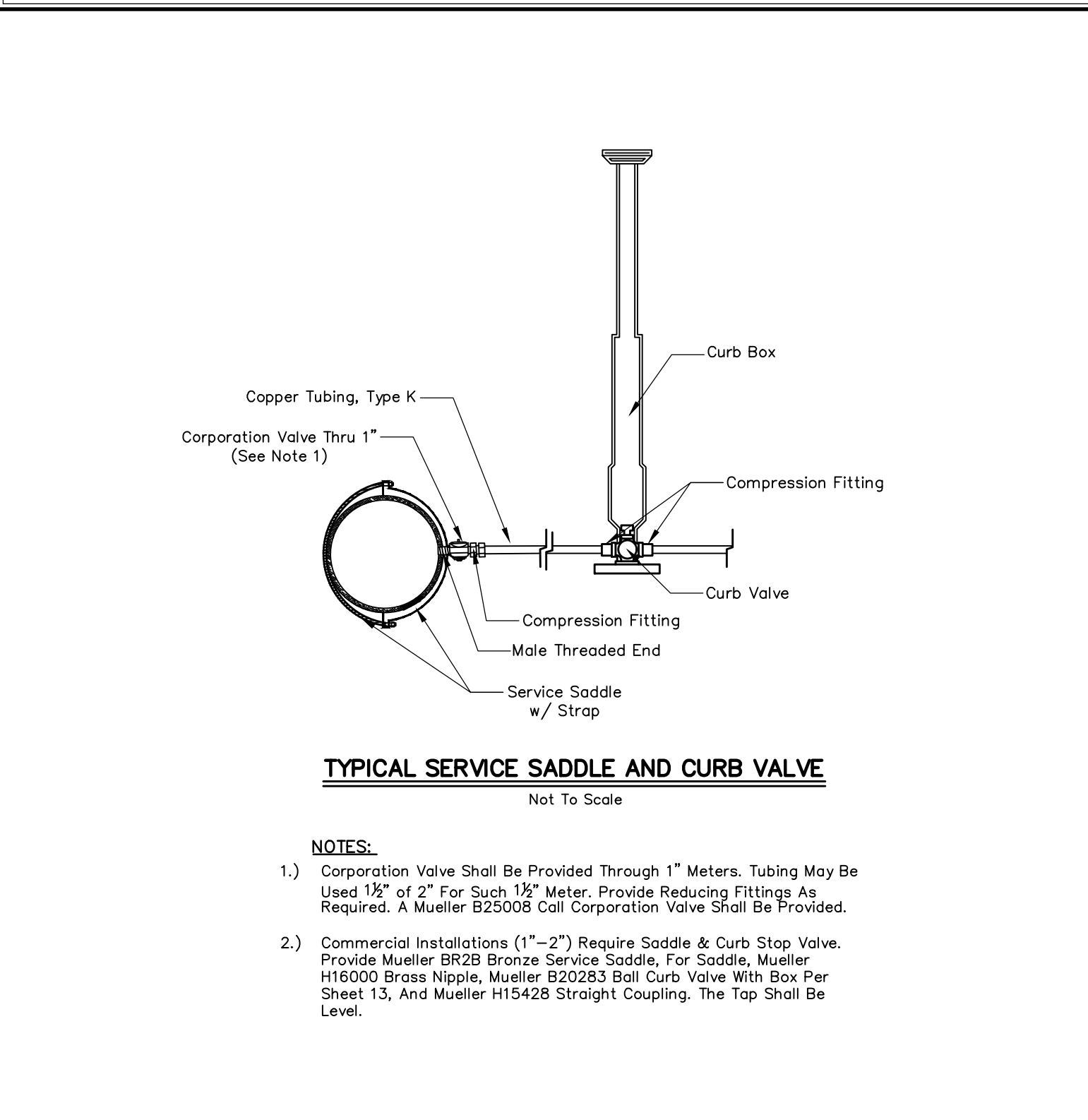
### DEVELOPMENT STANDARD - DETAIL DS-W05

### DEVELOPMENT STANDARD - DETAIL DS-W02



### DEVELOPMENT STANDARD - DETAIL DS-W06

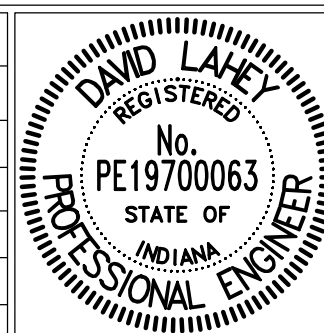
### DEVELOPMENT STANDARD - DETAIL DS-W03



### DEVELOPMENT STANDARD - DETAIL DS-W07

### DEVELOPMENT STANDARD - DETAIL DS-W04

REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Lacey</i>	01/01/2015
DESIGN ENGINEER		DATE
APPROVED	<i>James J. Brou</i>	01/01/2015
EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES		DATE
APPROVED	<i>Jason Carls</i>	01/01/2015
DIRECTOR OF PUBLIC WORKS		DATE

TOWN OF PLAINFIELD  
WATER (W)  
DEVELOPMENT STANDARDS

SHEET  
14  
OF  
29



SANITARY SEWER REINFORCED CONCRETE PIPE

- 1.) Reinforced Concrete Pipe For Use As Sanitary Sewers Shall Be Class III, IV, Or V As Specified By Design Engineer Per ASTM C76. Lift Holes Shall Not Be Permitted.
- 2.) Each Section Of Reinforced Concrete Pipe Shall Be Vacuum Tested By The Manufacturer Prior To Delivery To The Job Site. Only Pipe Sections Passing Vacuum Test Shall Be Marked As "Vacuum Tested". Vacuum Test Requirements Are As Follows:
- A. Each Section Of Pipe Shall Tested By Bringing The Internal Pressure Within The Pipe To 3.5 PSIG Below Atmospheric Pressure And The Pressure Must Not Drop To Less Than 2.5 PSIG Below Atmospheric Pressure Within The Time Limitation As Determined By The Following:
- $$T = \frac{0.022 D^2 L}{2}$$
 Where: T = Time In Seconds  
D = Diameter Of Pipe In Inches  
L = Length Of Pipe In Feet
- B. Any Pipe Section Failing To Meet This Test Shall Not Be Permitted For Use As Sanitary Sewers In The Town Of Plainfield.
- 3.) Lateral Connections Shall Be Made With KOR–N–Tee, Inserta–Tee, Or Town Approved Equal.
- 4.) Each Pipe Section Shall Be Marked With The Date Of Manufacture, Size, And Class Of Pipe, Specification Designation, Manufacturer And Plant Identification.

SANITARY SEWER POLYVINYL CHLORIDE (PVC) PIPE

- 1.) PVC Pipe Diameters Of 8 Inches Through 15 Inches Shall Meet Or Exceed All Requirements Of ASTM D3034, And Shall Have A Minimum Cell Classification Of 12454. Reference Should Be Made To ASTM D1784 For A Summarization Of Cell Class Properties. PVC Pipe Diameters Greater Than 15 Inches Shall Meet Or Exceed All Requirements Of ASTM F679, And Shall Have A Minimum Cell Classification Of 12454.
- 2.) The Minimum Wall Thickness Of PVC Pipe 8 Inches Through 15 Inches In Diameter Shall Conform To SDR–35, Type PSM, As Specified In ASTM D3034 (See Note 5 For Fittings). The Minimum Wall Thickness For P.V.C. Pipe Greater Than 15 Inches Shall Conform To PS 46 As Specified In ASTM F679. P.V.C. Pipe Shall Have A Minimum Pipe Stiffness Of 46 Pounds Per Square Inch For Each Diameter When Measured At Five Percent Deflection And Tested In Accordance With ASTM D2412.
- 3.) All PVC Sanitary Sewers With A Cover Depth Of 15 Feet Or Greater Shall Be SDR 26 Or PS 115 Rated.
- 4.) PVC Open Profile Or Closed Profile Sewer Pipe Shall Meet Or Exceed All Requirements Of ASTM F794 Or ASTM F949, And Shall Have A Minimum Cell Classification Of 12454 And A Minimum Uniform Pipe Stiffness Of 50 Pounds Per Square Inch For Each Diameter When Measured At Five Percent Deflection And Tested In Accordance With ASTM D2412 (See Note 5 For Fittings).
- 5.) Pipe Joints Shall Have A Bell Wall, Gasket Groove, And Spigot Which Is Integral With The Pipe. The Assembly Of Joints Shall Be In Accordance With Pipe Manufacturer's Recommendations And ASTM D3212. Solvent Cement Joints Shall Not Be Allowed For Mainline Pipe.
- 6.) Pipe Fittings Shall Be SDR–26 Manufactured Fittings Made Of PVC Plastic Having A Cell Classification Of 12454 As Defined In ASTM D1784. Saddle Connections Shall Not Be Allowed For New Construction. Lateral Connections Shall Occur At SDR–26 Tee–Wyes.
- 7.) Each Pipe Section Shall Be Marked With The Name Of Manufacturer, Trademark Or Tradename, Nominal Pipe Size, Production/Extrusion Code, Material And Cell Classification, And ASTM Number.
- 8.) Installation Shall Be In Accordance With Recommended Practice ASTM D2321.

SANITARY SEWER GENERAL NOTES AND AS–BUILT DRAWINGS

- 1.) See Development Standards DS–S01, DS–S02, For Sanitary Sewer Lateral Requirements.
- 2.) Sanitary Sewer Pipe Of Other Material Or Material Not Meeting These Specifications Shall Require The Prior Written Approval Of Plainfield DPW.
- 3.) The Contractor Shall Submit Information To The Town Engineer Showing Conformance With These Specifications Upon Request.
- 4.) As–Built Drawings Shall Be Submitted To Plainfield DPW. GPS Collected Coordinates Shall Depict Actual Horizontal And Vertical Locations Of Utility Assets Such As: Manholes, Laterals, Stubs, Air Release Valves, Flushing Stations, Cleanouts, Risers, And Pump Stations/Wet Wells.

SANITARY SEWER DEFLECTION TESTING AND TELEVISING

- 1.) Deflection Testing Is Required For All Mainline Flexible Pipe And Plainfield DPW Shall Be Given 24 Hour Written Notice Of Deflection Testing. An Allowable Deflection Of 5 Percent Inside Pipe Diameter Will Be Acceptable After All Backfilling Has Been In Place For 30 Days. A Nine Point "Go–No–Go" Mandrel Shall Be Used For The Deflection Test. A Proving Ring Shall Be Provided For Each Mandrel. All Pipe Exceeding The Allowable Deflection Shall Be Televised To Determine The Extent Of Replacement Or Rerounding Required. The Reworked Section Shall Be Retested 30 Days After Completion. Contractor Shall Bear All Testing Costs. The "Go–No–Go" Mandrel Shall Be Manually Pulled Without The Use Of Mechanical Devices.
- 2.) Following Air And Mandrel Testing, Televising Is Required. Plainfield DPW Shall Be Given 24 Hour Written Notice Of Televising. A Camera Equipped With Remote Control Devices To Adjust Light Intensity And 1,000 Linear Feet Of Sewer Cable Shall Be Provided. The Camera Shall Transmit A Continuous Image To The Television Monitor As It Is Being Pulled Through Pipe. The Image Shall Be Clear Enough To Enable The Town Of Plainfield Representative And Others Viewing The Monitor To Easily Evaluate The Interior Condition Of The Pipe. The Camera Shall Stamp The DVD With Manhole Number, Lateral Distance From Manhole, Linear Footage And Project Number, And An Audio Voice–Over Shall Be Made During The Inspection Identifying Problems. Contractor Shall Bear All Televising Costs.
- 3.) The Pipe Shall Be Thoroughly Cleaned Before Installing Camera And Commencing Televising.
- 4.) If Any Pipe And/Or Joint Is Found To Be Leaking, Regardless Of The Results Leakage Testing, In The Sole Judgement Of The Town, The Contractor Shall Repair That Portion Of The Work To The Satisfaction And Approval Of The Town Of Plainfield.

SANITARY SEWER LEAKAGE TESTING

- 1.) The Town Of Plainfield Shall Be Given 24 Hour Written Notice Of The Required Leakage Testing Procedure To Be Performed By The Contractor. Low Pressure Air Shall Be Slowly Introduced Into The Sealed Line Until The Internal Air Pressure Reaches 4 PSIG Plus The Groundwater Head Divided By 2.31 (Maximum Test Pressure Is 9 PSIG).
- 2.) At A Stable Internal Air Pressure Within 0.5 PSIG Of The Initial Internal Air Pressure, Timing Shall Commence With A Stopwatch Or Similar Device Of 99.8 Percent Accuracy. Timing Shall End When The Internal Air Pressure Drops 1 PSIG Below The Stable Internal Air Pressure.
- 3.) The Line Shall Be Accepted If The Time Shown In Table 1 For The Designated Pipe Size And Length Elapses Before The Air Pressure Drops 1 PSIG Below The Stable Internal Air Pressure At Which Time The Test Can Be Discontinued For The Accepted Line.

TABLE 1

SPECIFICATION TIME REQUIRED FOR A 1.0 PSIG PRESSURE DROP FOR SIZE AND LENGTH OF PIPE INDICATED FOR Q=0.0015

1 Pipe Diameter (In.)	2 Minimum Time (Min:Sec)	3 Length For Minimum Time (Ft.)	4 Time For Longer Length (Sec.)	Specification Time For Length (L) Shown (Min.:Sec.)								
				100 Ft.	150 Ft.	200 Ft.	250 Ft.	300 Ft.	350 Ft.	400 Ft.	450 Ft.	
4	3:46	597	.380L	3:46	3:46	3:46	3:46	3:46	3:46	3:46	3:46	
6	5:40	398	.854L	5:40	5:40	5:40	5:40	5:40	5:40	5:42	6:24	
8	7:34	298	1.520L	7:34	7:34	7:34	7:34	7:36	8:52	10:08	11:24	
10	9:26	239	2.374L	9:26	9:26	9:26	9:53	11:52	13:51	15:49	17:48	
12	11:20	199	3.418L	11:20	11:20	11:24	14:15	17:05	19:56	22:47	25:38	
15	14:10	159	5.342L	14:10	14:10	17:48	22:15	26:42	31:09	35:36	40:04	
18	17:00	133	7.692L	17:00	19:13	25:38	32:03	38:27	44:52	51:16	57:41	
21	19:50	114	10.470L	19:50	26:10	34:54	43:37	52:21	61:00	69:48	78:31	
24	22:40	99	13.674L	22:47	34:11	45:34	56:58	68:22	79:46	91:10	102:33	
27	25:30	88	17.306L	28:51	43:16	57:41	72:07	86:32	100:57	115:22	129:48	
30	28:20	80	21.366L	35:37	53:25	71:13	89:02	106:50	124:38	142:26	160:15	
33	31:10	72	25.852L	43:05	64:38	86:10	107:43	129:16	150:43	172:21	193:53	
36	34:00	66	30.768L	51:17	76:55	102:34	128:12	153:50	179:29	205:07	230:46	

NOTE:

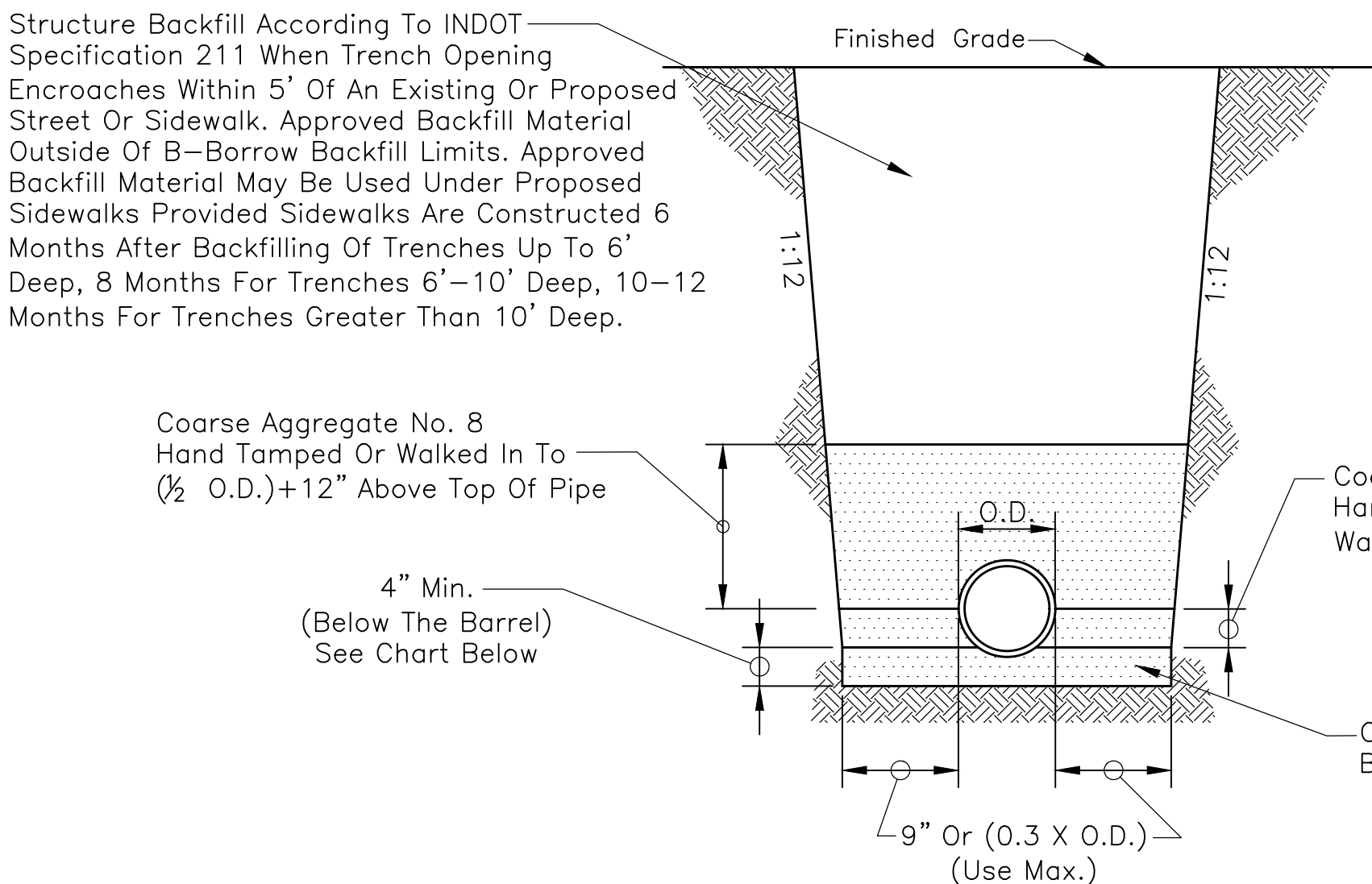
For More Efficient Testing Of Long Test Sections And/Or Sections Of Larger Diameter Pipes, A Timed Pressure Drop Of 0.5 PSIG May Be Used In Lieu Of The 1.0 PSIG Timed Pressure Drop. If A 0.5 PSIG Pressure Drop Is Used, The Required Test Time Shall Be Exactly Half As Long As Those Shown Above.

SANITARY FORCE MAIN PRESSURE AND LEAKAGE TESTING

- 1.) The Town Of Plainfield Shall Be Given 24 Hour Written Notice Of The Required Pressure And Leakage Test To Be Performed By The Contractor. The Pressure And Leakage Test Shall Be Performed In Accordance With The Basic Provisions Of AWWA C600. All Force Mains Shall Be Given A Hydrostatic Test Of At Least 1.5 Times The Shutoff Head Of The Connected Pumps Or 150 PSI, Whichever Is Lesser. Test Pressure Shall Not Exceed Pipe Restraint Design Pressures Or Rated Pressure Of The Valves. Loss Of Water Pressure During Test Shall Not Exceed 5 PSI In A 2 Hour Test Period.
- 2.) Valves Shall Not Be Operated In Either Direction At Differential Pressures Exceeding The Rated Valve Working Pressure.
- 3.) The Pressure And Leakage Test Shall Be Performed Following The General Form Of The Following:
- A. Record Time And Line Pressure Prior To Start Of Test.
- B. Pump Water Into New Force Main Until Pressure Reaches At Least 1.5 Times The Shutoff Head Of The Connected Pumps Or 150 PSIG, Stop Pumping And Record Time And Line Pressure.
- C. Contractor Shall Remain At Site For One Hour. The Test Shall Be Voided If Any Adjustments Are Made To The Force Main, Test Equipment, Or Appurtenances. Tightening Of Fittings On Test Equipment Is Allowed. Following The One Hour Period, Record Time And Line Pressure.
- D. Pump Water Into New Force Main From A Calibrated Container Of Water Until Pressure Reaches 150 PSIG, Stop Pumping When Pressure Reaches 150 PSIG, Record Time, Line Pressure, And Amount Of Water Pumped To The Nearest 1/100 Gallon. The Calibrated Container Shall Have Markings At 1/10 Gallon Increments.
- E. Repeat Steps C And D One Additional Time.
- 4.) A Test Section Of Force Main Is Considered Satisfactory If It Meets The Following:

Main Size (Inches)	Allowable Leakage (Gal./Hr./1000 Ft.)
4	0.33
6	0.50
8	0.66
10	0.83
12	0.99

- 5.) If The Leakage From A Test Section Is Greater Than Permitted Under These Specifications, The Contractor Shall Locate And Repair The Defective Joints, Mains, And Appurtenances. The Pressure And Leakage Test Shall Then Be Repeated Until Satisfactory Results Are Obtained. All Labor And Materials Required To Meet The Requirements Of The Pressure And Leakage Test Shall Be At The Expense Of The Contractor.

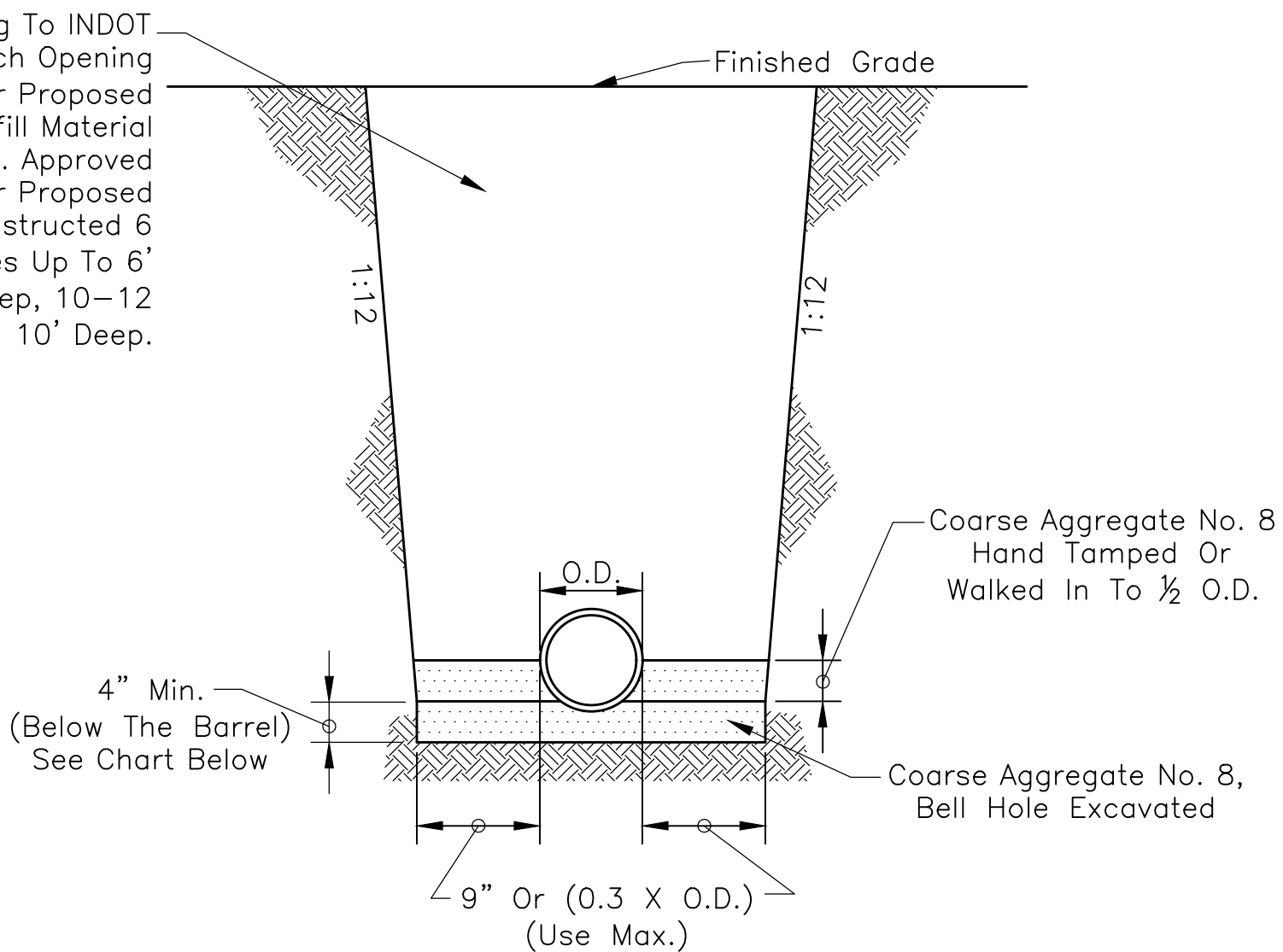


Pipe Size	8" To 15"	18" And Over
Bedding Below The Pipe Barrel	O.D./4 Min.=4"	O.D./4 Min.=8"

\*See Development Standard DS–S01 For Lateral Pipe Bedding\*

PVC PIPE BEDDING DETAIL

Scale: None



Pipe Size	8" To 15"	18" And Over
Bedding Below The Pipe Barrel	O.D./4 Min.=4"	O.D./4 Min.=8"

RCP BEDDING DETAIL

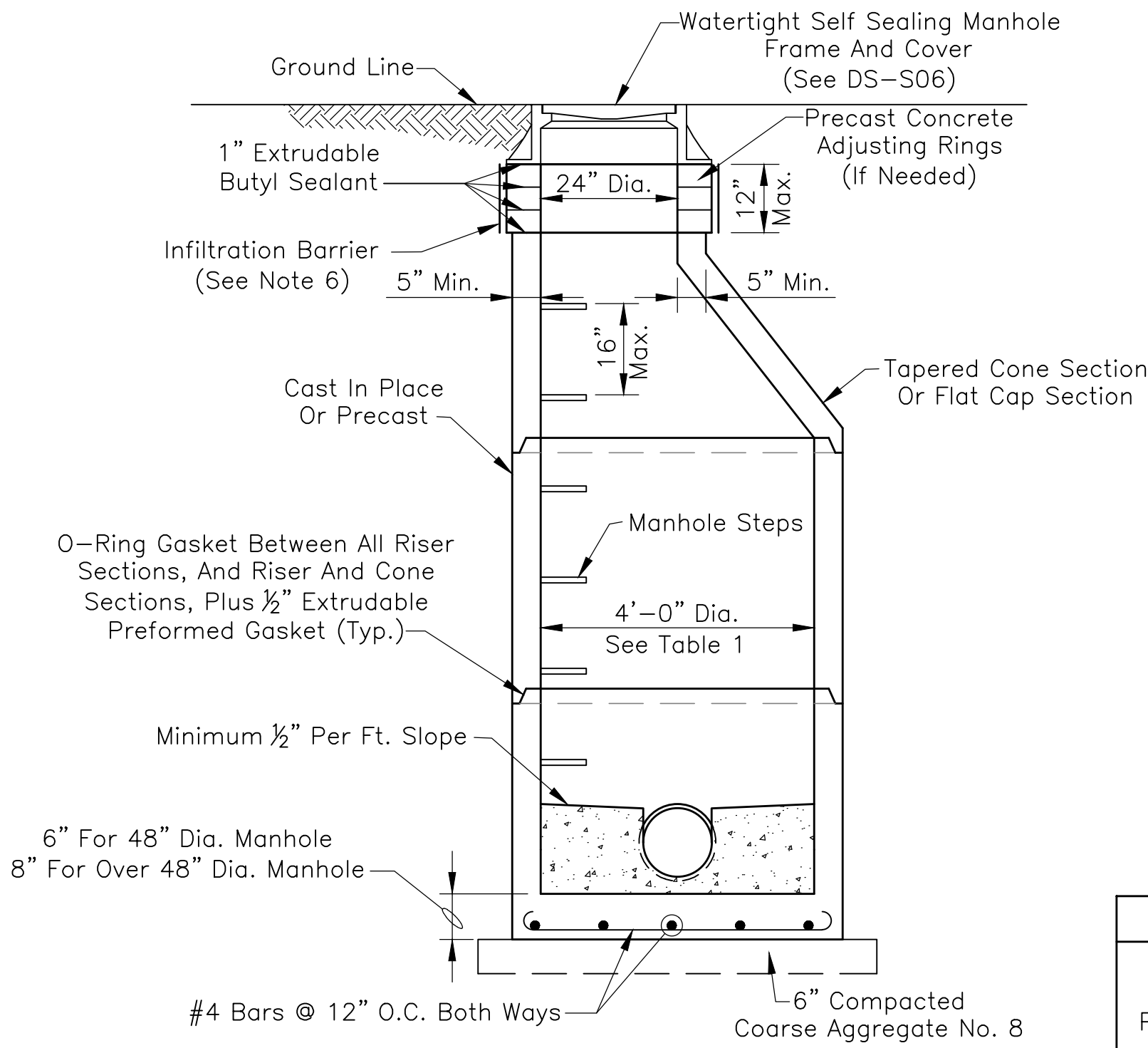
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REVISIONS				RECOMMENDED FOR APPROVAL  DESIGN ENGINEER  APPROVED  EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES  APPROVED  DIRECTOR OF PUBLIC WORKS	DATE 01/01/2015  DATE 01/01/2015  DATE 01/01/2015	TOWN OF PLAINFIELD  SANITARY SEWER BEDDING DETAILS AND NOTES	SHEET 15 OF 29
Rev. No.	Description	Date					



SANITARY MANHOLE GENERAL NOTES

- 1.) Precast Concrete Manholes Shall Conform To ASTM C478, With Rubber Type Gaskets Equal To ASTM C443. Monolithic Cast In Place Manholes Shall Only Be Used With The Prior Written Approval Of The Town. The Base And First Riser Section Of The Precast Concrete Manhole Shall Be Integrally Cast As One Unit. Precast Concrete Cones Shall Be Of The Eccentric Cone Type. No "See Through" Lift Holes Shall Be Allowed On Precast Concrete Manholes 48 Inches In Diameter Or Less. In Addition To The Rubber Type Gaskets, All Joints Shall Receive A 1/2 Inch Diameter Non-Asphaltic Mastic (Kent-Seal Or As Approved By Plainfield DPW) Conforming To ASTM C990. Sewer Connection To Manhole Shall Be KOR-N-SEAL, A-LOK, Press-Seal, Or As Approved By Plainfield DPW.
- 2.) Where One Solid Riser Or Barrel Section Cannot Be Used, Final Adjustment In Elevation Of The Frame And Cover Shall Be Accomplished By The Use Of A 4 Inch Minimum Thickness Adjusting Ring As Detailed Herein To A Maximum Combined Thickness Of 12 Inches. Brick Or Block Shall NOT Be Used In The Construction Of A Manhole Or To Adjust The Elevation Of The Frame And Cover.
- 3.) Manhole Steps Shall Be Neenah No. R-1981-J, M.A. Industries No. PS 1-PF, Or As Approved By Plainfield DPW.
- 4.) Manhole Frame And Cover Shall Be Per Development Standards DS-S06 Or Town Approved Equal.
- 5.) The Lowest Elevation To Receive Gravity Sanitary Service Must Be One Foot Above The Top Of Manhole Casting Elevation Of Either The First Upstream Or Downstream Manhole On The Public Sewer To Which Connection Is To Be Made. Those Portions Of The Building Not Meeting The Stated Gravity Sanitary Service Requirement Shall Be Provided With A Grinder Pump System Or Town Approved Equal Discharging To The Gravity Building Connection Outside Of The Public Right-Of-Way.
- 6.) Infiltration Barrier Shall Be 60 Mils Minimum EPDM Sealed With A 2 Inch Mastic Strip To Cone (Manhole) And To Top Of Casting Lip And Shall Be Infi-Shield Or Town Approved Equal.
- 7.) Plainfield DPW May Approve Alternate Drop Connection If There Are Special Circumstances.
- 8.) Lateral Connections To A Manholes Are Prohibited.
- 9.) 10% Of All Sanitary Manholes Shall Be Vacuum Tested With Castings Per ASTM C1244 Following Full Installation. All Sanitary Manhole Sections Shall Be Vacuum Tested In The Shop Prior To Shipment. Dewatering Shall Continue In Order To Prevent Hydrostatic Pressures From Affecting The Test.

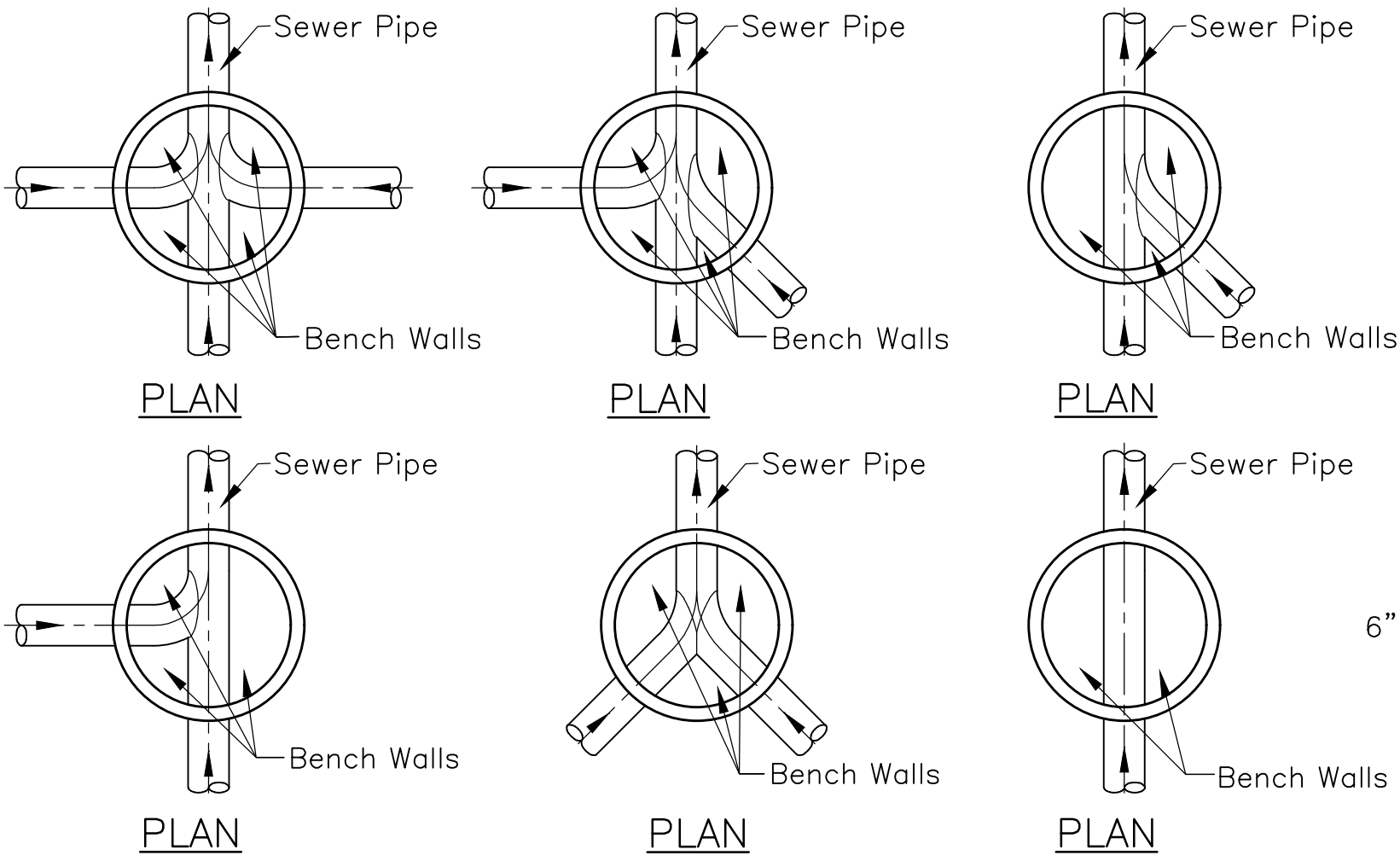


SANITARY MANHOLE, TYPE A

Scale: 1/2" = 1'-0"

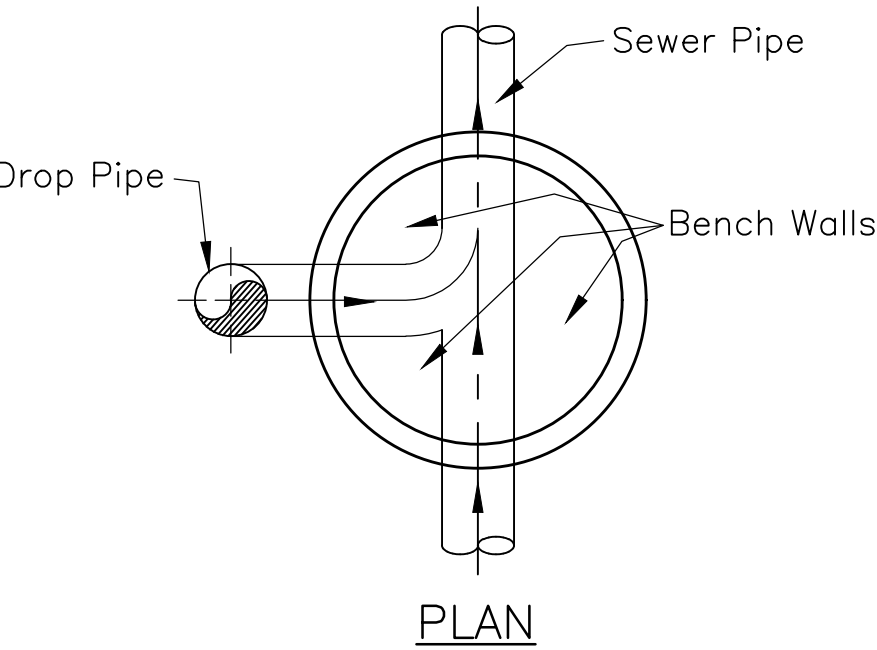
Pipe Size	Minimum Manhole Diameter	
	Pipe Entering/ Pipe Exiting At 0° To 45° Bend	Pipe Entering/ Pipe Exiting At 45° To 90° Bend
8"-21"	48"	48"
24"	48"	60"
27"-30"	60"	60"
33"-36"	60"*	72"

\* 72" With A-Lock Connector



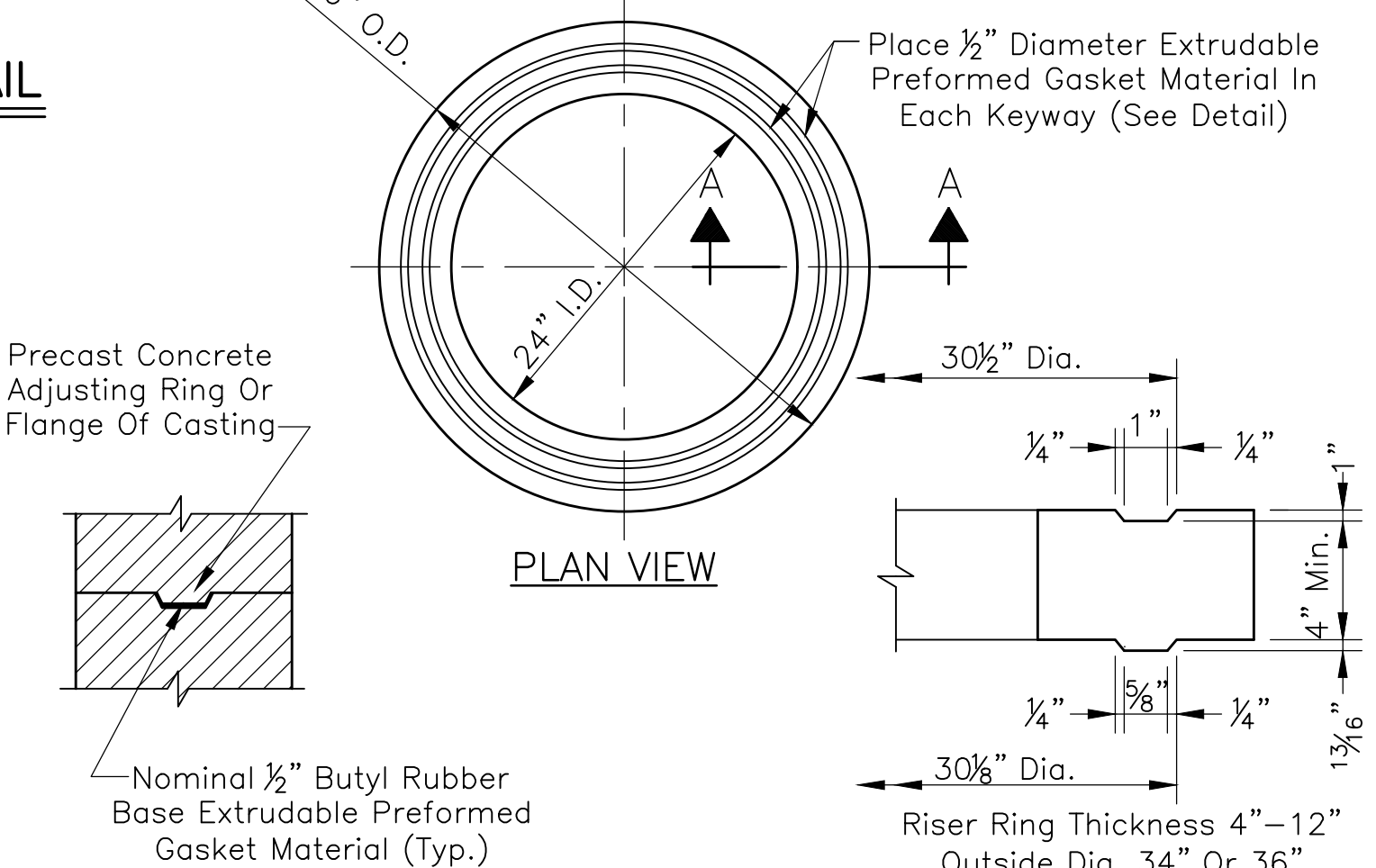
BENCH WALL DETAILS

Not To Scale



BENCH WALL DETAIL

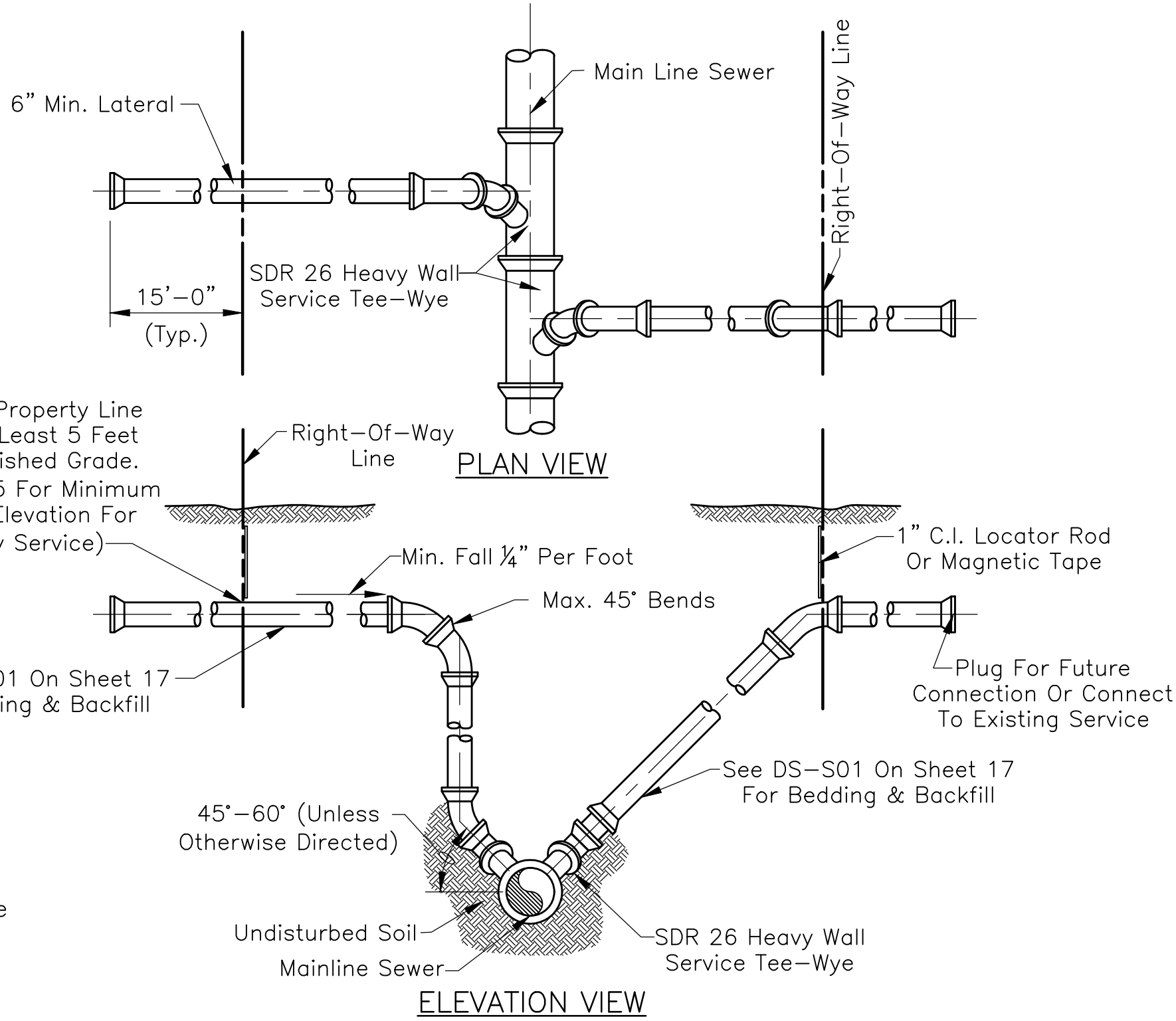
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GASKET DETAIL

PRECAST CONCRETE ADJUSTING RING

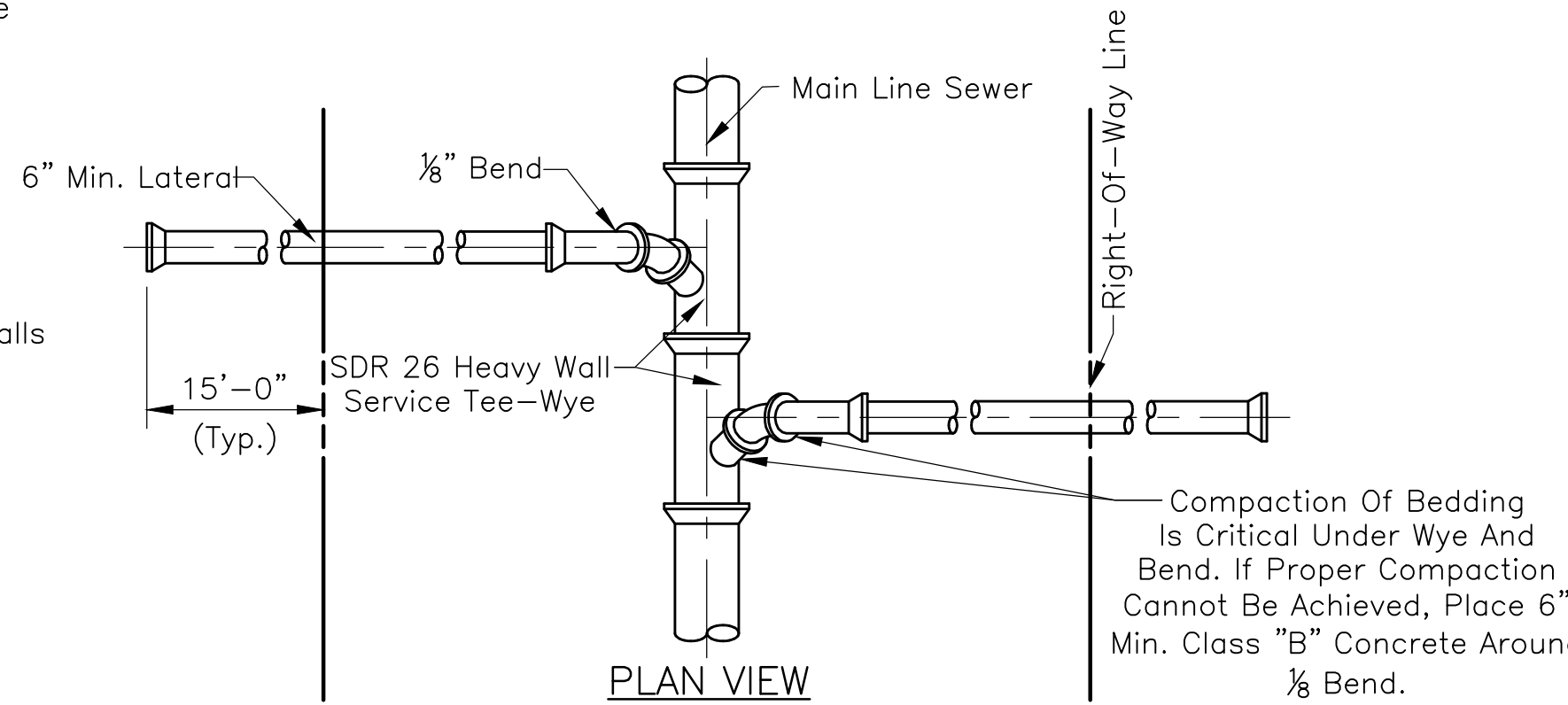
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SERVICE CONNECTION FOR DEEP SEWERS

(15' DEEP AND OVER)

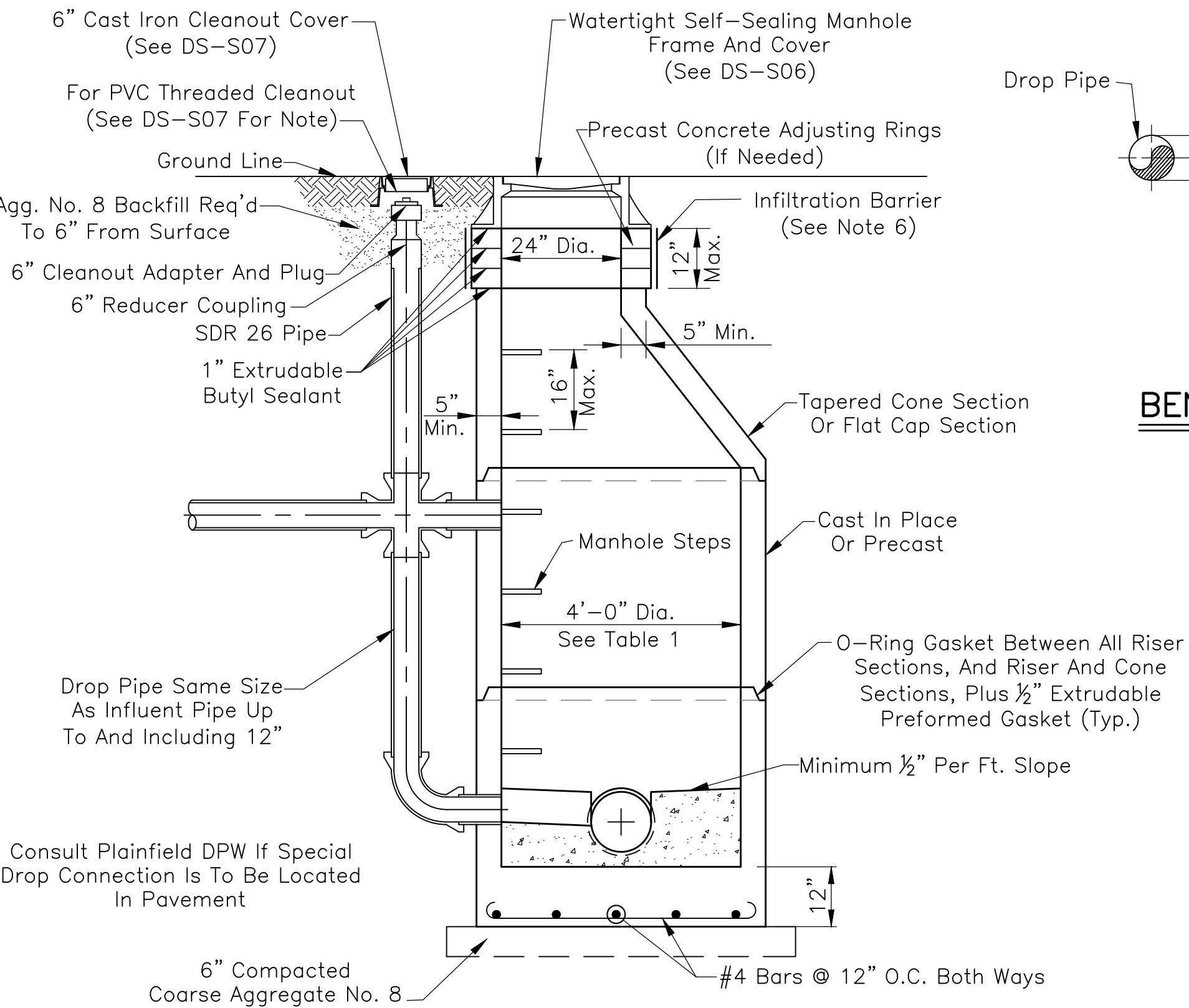
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SERVICE CONNECTION FOR SHALLOW SEWERS

(LESS THAN 15' DEPTH)

Not To Scale

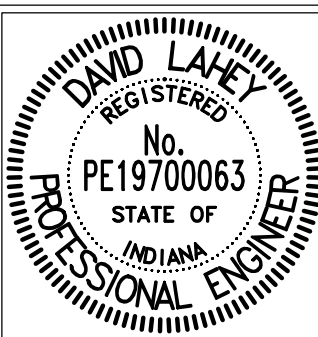


SPECIAL DROP CONNECTION

Scale: 1/2" = 1'-0"

(\*For Use Outside Of Pavement Only\*)

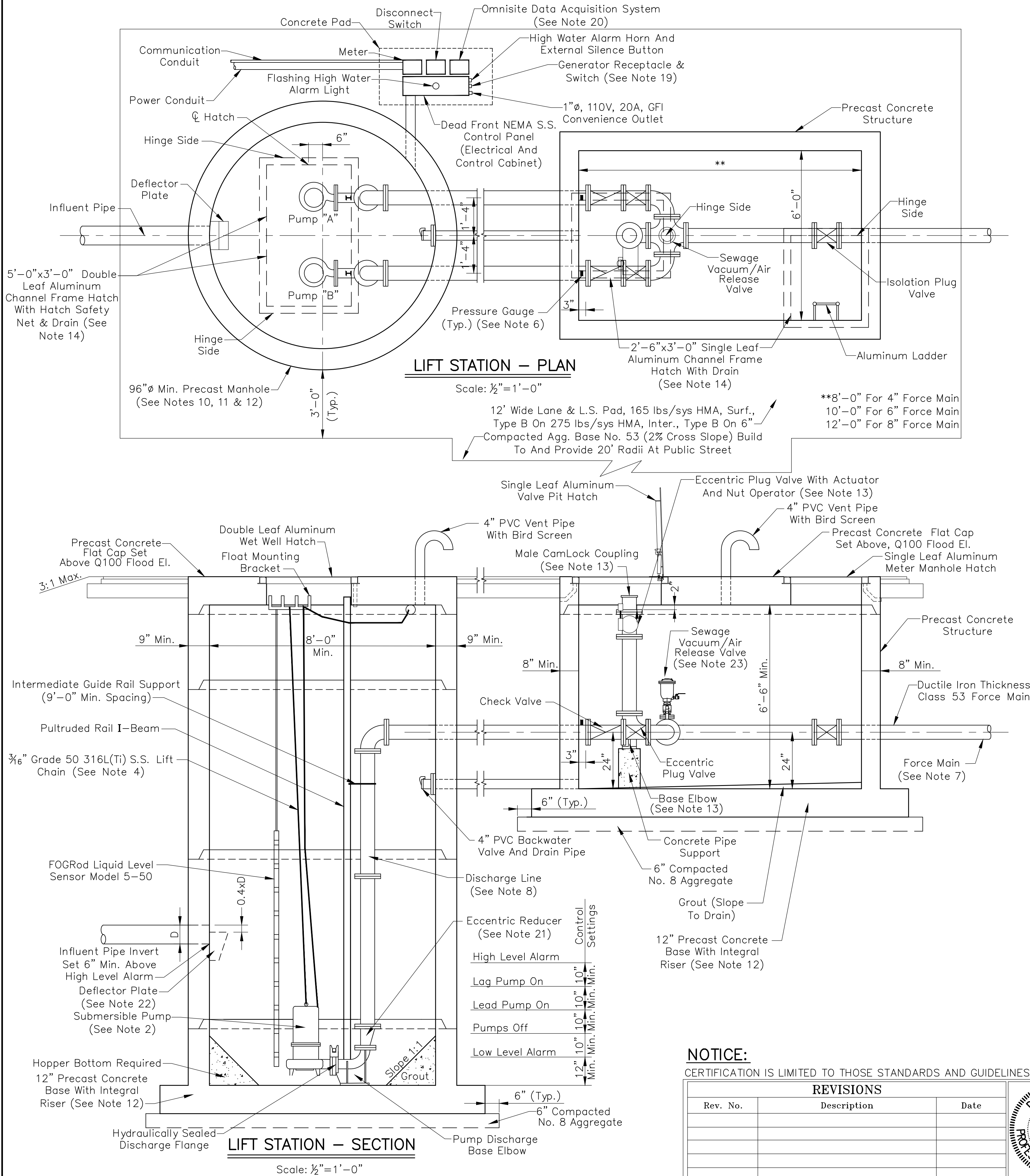
REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Lacey</i>	01/01/2015
APPROVED	<i>James J. Brou</i>	01/01/2015
APPROVED	<i>Jason Caruth</i>	01/01/2015

TOWN OF PLAINFIELD	SHEET 16 OF 29
SANITARY SEWER DETAILS AND NOTES	





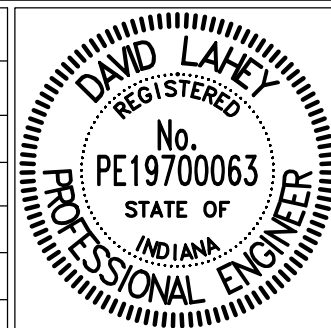
**GENERAL NOTES:**

- Actual Lift Station Dimensions, Control Settings, And Pump Selection To Be As Indicated By The Design Engineer's Certification Sheet.  
Final Orientation And Location Of Hatches And Control Panel Are To Be Field Verified.
- Pumps "A" And "B" Shall Be Identical, Centrifugal, Submersible, Solids Handling, Non-Clog Design Capable Of Handling 3 Inch Sphere Solids, Fibrous Material, Sludge, And Material Found In Typical Raw Sewage. Fit Replaceable Bronze Wear Ring To Volute. Pumps Shall Be Hydromatic, Flygt, Or Plainfield DPW Approved Equal. Manufacturer Shall Warrant The Pumps For One Year After Installation.  
All Mating Surfaces Intended To Be Watertight Shall Be Machined And Fitted With Nitrile Rubber O-Rings With Sealing Complete When Metal To Metal Contact Is Made, Resulting In Controlled Compression Of O-Rings Without Specific Torque Limit. Fasteners Shall Be 316 S.S.  
Mechanical Shaft Seal System Running In An Oil Reservoir Shall Have Separate, Constantly Lubricated Lapped Seal Faces. The Lower Seal Unit Between Media And Oil Reservoir Shall Consist Of One Stationary Seat And One Rotating Ring Held In Place By Its Own Spring. The Rotating Seat Ring And The Stationary Seat Ring Shall Be Made Of Tungsten-Carbide. The Lower Seal Shall Be Removable Without Disassembling The Seal Chamber. The Upper Seal Between Seal Chamber And Motor Shall Be Of The Same Design With Its Own Spring. Seals Shall Be Maintenance Free, But Shall Be Easily Inspectable.  
Lower Seal Failure Alarm Shall Be Engaged By Seal Failure Sensor Provided In The Seal Chamber Which Senses Water Intrusion Through Lower Seal.  
Over Temperature Alarm, And Pump Shut Down, Shall Be Engaged By Heat Sensor Attached To The Motor Windings. Motor Winding And Stator Lead Insulation Shall Be Class F With Maximum Temperature Capability Of 155° C Or Better. Housing Shall Be Filled With High Dielectric Oil. Air Filled Housing May Be Acceptable When Approved By Plainfield DPW. Pump And Motor Shall Be Designed To Operate Partially Or Fully Submerged In Pumped Media Without The Use Of Cooling Jackets.  
Rail System Shall Enable The Easy Removal Of The Pump Without The Need For A Person To Enter The Wet Well. A Non-Corrosive FRP I-Beam Shall Be Provided For Each Pump. The Guide Rail Shall Be Supported At The Bottom By The Discharge Elbow, Aligned Perfectly Plumb And Securely Affixed To Access Frame. One Intermediate Guide Rail Support Is Required For Each 9' Of Guide Rail Length. Schedule 40 S.S. Guide Rails May Be Acceptable If Pump Is Approved By Plainfield DPW.
- Check Valve Shall Be Bronze Seated And Shall Be Provided With Bolted Covers For Easy Access To The Discs. Valve Shall Be Outside Adjustable Weight And Lever As Mueller A-2600-6-01, Kennedy/Clow 1106LW, Or As Approved By Plainfield DPW. The Valve Shall Be Furnished With Fusion Bonded Epoxy Coating Inside And Out In Accordance With AWWA C550.
- Provide Sufficient Lift Chain, Float Mounting Cable, And Pump Power And Sensor Cable To Enable Non-Spliced Field Adjustment. Lift Chain Shall Have A Minimum Work Load Limit Of 1100 Pounds. Float Mounting Cable Shall Be Held In Place By Weight, Floats Shall Be Fastened To Cable With S.S. Clamps Near Each Float Location. Pump Power And Sensor Cable Shall Be Suitable For Submersible Pump Applications And This Shall Be So Indicated By A Code/Legend Permanently Embossed On The Cable.
- Pump Valve Shall Be An Eccentric Buna-N Rubber Faced Plug With Hand Lever Operation In-Line And Gear Operation On Bypass. Valve Shall Be Valmatic F-5800-R, Kennedy/Clow F-5412, Or As Approved By Plainfield DPW. The Valve Shall Be Furnished With Fusion Bonded Epoxy Coating Inside And Out In Accordance With AWWA C550.
- Pressure Gauge Shall Be Trerice Model 450 LFB Or Plainfield DPW Approved Equal. Drill And Tap Run Of Pipe To Install Pressure Gauge.
- Piping Not Within 2 Feet Of Wet Well And Valve Pit Shall Be DI AWWA C151, HDPE AWWA C906, PVC ASTM D2241, PVC AWWA C900, Or Plainfield DPW Approved Equal. See Design Engineer's Certification Sheet For Pipe Class.
- Piping In And Within 2 Feet Of Wet Well And Valve Pit Shall Be Class 53 Flanged Ductile Iron Pipe And Shall Be Manufactured By Tyler, American, Or U.S. Pipe, Or As Approved By Plainfield DPW. All Fasteners Within Wet Well And Valve Vault Shall Be 316 S.S.
- Piping And Fittings In Wet Well And Valve Pit Shall Be Factory Primed Tnemec Series Purple Prime To A Dry Film Thickness Of 5.0 To 11.0 Mils And Shall Be Field Painted With Tnemec Series 69-Color To A Dry Film Thickness Of 5.0 To 6.0 Mils. Fittings Shall Be Manufactured By U.S. Pipe, Tyler, Mueller, Or As Approved By Plainfield DPW.
- Damp Proof All Exterior Vertical Surfaces Which Are Backfilled Against With Bituminous Coating, Mastersseal 614.
- Lift Station Manhole And Valve Pit Structures Shall Be Precast Concrete In Accordance With ASTM C478, With Rubber Gaskets Equal To Gasket Material Or Plainfield DPW Approved Equal. See Sanitary Sewer Details And Notes Sheet For Manhole Steps.
- Horizontal Projections From Precast Integral Base And Riser May Be Required To Enable The Weight Of The Vertical Soil Ring Above The Projection To Resist Buoyancy Forces. See Design Engineer's Certification Sheet.
- CamLock Coupling And Eccentric Plug Valve On Bypass Line Shall Be 4 Inch Diameter With Transition To Force Main Size Occurring With Concentric Reducer Placed On Top Of Base Elbow. Fix Operating Nut For Eccentric Plug In Vertical Position To Enable Wrench Operation From Surface. Layout Of All Valve Vault Fittings And Equipment To Be Based Upon Bypass Line Being Close To Hatch Opening, As Shown.
- Aluminum Hatches Shall Be Channel Frame Type Flygt Safe-Hatch. Leaf Shall Be ½ Inch Aluminum Diamond Plate Live Load Rated To 300 PSF. Channel Frame Shall Be ½ Inch Extruded Aluminum With A Mill Finish And Bituminous Coating On Exterior Surfaces. Hatch Shall Be Provided With Type 316 S.S. Hardware Throughout, Automatic Hold-Open Arm With Release Handle, Siam Lock With Removable Handle, 1-½ Inch Drain Coupling, Padlock Hasp, And USF Fabrication Fall-Through Protection Hatch Safety Net.
- Sewer Connection To Wet Well Shall Be KOR-N-SEAL, A-LOK, Press-Seal, Or Plainfield DPW Approved Equal.
- Force Main Penetrations Of Wet Well And Valve Pit Shall Be Made Watertight Through The Use Of Portland Cement Grout.
- Automatic Pump Control Panel Shall Include All Necessary Items And Appurtenances Which Might Normally Be Considered A Part Of A Complete System, Including But Not Limited To: Condensate Heater; Push To Test Button (External); Push To Silence Button (External); Alternator Selector Switch For Manual Designation Of Lead Pump; Time Delay Relay For Lag Pump Start; And Pump Run Time Hour Meters. System Shall Be Supplied By One Manufacturer, Shall Be Factory Assembled, Wired, Tested, And Shall Be Per Complete Electrical Drawings And Instructions. Major Components And Sub-Assemblies Shall Be Identified By Their Function With Laminated, Engraved, Bakelite Nameplates. System Shall Be Built In A Minimum 60"x36"x12" NEMA 4X S.S. Enclosure Suited For The Specified Horsepower And Voltage Of The Pumps. The Outer Door Of The Panel Shall Be A Hinged Dead Front With Provisions For Padlocking. Inside Shall Be A Separate Hinged Panel To Protect All Electrical Components, H-O-A Switches, Run Lights, Circuit Breakers, Etc., Mounted Such That Only The Faces Protrude Through Said Panel With No Wiring Fixed To Said Panel. The Manufacturer Shall Warrant The Control Center For One Year After Installation Covering 100% Parts And Labor.  
Provide The Services Of A Factory Trained, Qualified Representative To Inspect, Adjust, Place The System In Trouble Free Operation, And Instruct Operating Personnel In The Proper Operation And Care Of The System.  
All Major Components Of Control Center Shall Be American-Made And Available From Local Sources. Pump Manufacturer Shall Accept The Control Center In Writing To Ensure Unit Responsibility And Warranty.  
Provide A Manual Transfer Type Disconnect Switch Housed In A Separate NEMA 4X S.S. Enclosure With External Operation Handle Capable Of Being Locked In The "ON" Normal Position Or The "ON" Secondary Position With A Middle "OFF" Position.  
A Lightning Arrestor Shall Be Provided At The Phase Relay Block And Connected To Each Line Of The Incoming Side Of The Power Input Terminals. A Single Main Fusible/Breaker Disconnect Switch Of Adequate Size To Provide Power For Control, Operation, And Appurtenant Components Shall Be Provided. Provide A Circuit Breaker And Magnetic Starter With Each Leg Manual Reset Overload Protected For Each Pump. Starters Shall Have Auxiliary Contacts On 3Ø Applications To Operate Both Pumps Simultaneously. Provide A Phase Monitor With Phase Fail Relay. Provide A Circuit Breaker And Transformer To Power The Control Panel With 1Ø, 115 Volt Service For All Control Functions Including OMNISITE Data Acquisition System, Radio And Flowmeter. Provide A Green "Run" Light, And H-O-A Switch To Enable Field Connections.  
Materials And Installation Of The Required Equipment Grounding Shall Be In Accordance With NEC Section 250-83(c). All Wiring Shall Have Not Less Than 600 Volt Insulation. Wiring And Bus Shall Be In Accordance With NEC, State, Local, And NEMA Standards. All Wiring Shall Be Color Coded. Minimum 4 Inch Diameter, Schedule 40 Conduit Shall Be Provided From Wet Well To Control Panel Enabling Pump Power And Sensor Cables, And Float Switch Cables To Be Easily Pulled. Seal Conduit At Control Panel To Prevent Sewer Gases From Entering. All Conduits, Fittings, Or Connections Shall Enter From The Bottom Of Enclosures.  
Sump Level Rise To Lead Pump Run Float Causes Lead Pump To Operate. Lead Pump Operating And Sump Level Falling To Pumps Off Float Causes Lead Pump To Shut Off. Lead Pump Operating And Sump Level Rising To Lag Pump Run Float Causes Lag Pump To Operate. Lag Pump Operating And Sump Level Falling To Pumps Off Float Causes Both Pumps To Shut Off. Sump Level Rise To High Level Alarm Causes High Level Alarm To Operate. Sump Level Fall To Low Level Alarm Causes Low Level Alarm To Operate. An Alternating Relay Shall Be Provided To Cause Pumps To Alternate Whenever Pumps Off Float Is De-Energized. If One Pump Fails For Any Reason, The Remaining Pump Shall Operate Upon Sump Level Rise To Lag Pump Run Float. An Hour Meter Shall Be Provided For Each Pump To Record The Elapsed Operating Time Of Each Pump.  
Four Manuals Shall Be Presented To The Owner Which Shall Include The Following Minimum Information: 1) Operation Instructions; 2) Maintenance Instructions; 3) Recommended Spare Parts List; 4) Lubrication Schedule; 5) Structural Diagrams; 6) As-Built Wiring Diagrams; And 7) Bill Of Materials.
- Generator Receptacles To Be Crouse-Hinds Arktite AR1042 100amp Receptacle Or Crouse-Hinds Arktite AR2041 200amp Receptacle With Factory Sealed Switch For Receipt Of The Town Of Plainfield's Portable Generator Set.
- Provide OMNISITE XR 50 Data Acquisition System For Duplex Pump Stations And OMNISITE Crystal Ball Data Acquisition System For Triplex Pump Stations That Incorporates: 1 Spare Input/Output, 1 Input For Flowmeter, 5 Outputs To Control Being Lead Remote On, Lead Remote Off, Lag Remote On, Lag Remote Off, Remote Alarm Acknowledge, 10 Inputs From Control Being Hatch(es) Open Alarm, Panel(s) Open Alarm, Pump "A" On, Pump "B" On, Pump "A" Fail, Pump "B" Fail, Phase Fail Alarm, Power Fail Alarm, High Water Alarm, And Pump(s) Seal Failure. Remote Lead Pump Override And Remote Lag Pump Override.
- Eccentric Reducer To Be Installed As Required For Force Main Size. Consult Plainfield DPW If Force Main Piping Is Greater Than 6 Inch Diameter.
- ¾" Stainless Steel Deflector Plate Required On All Influent Pipes. As Supplied by Moorsville Welding or DPW Approved Equal.
- Air/Vacuum Release Valve Shall Be An ARI D-025P Combination Air Valve For Wastewater And Shall Be Sized By The Design Engineer According To The Volume Of Main And Maximum Force Main Operating Pressure. The Pipe Nipples And Gate Valve For The Air/Vacuum Release Valve Shall Be Stainless Steel.

**NOTICE:**

CERTIFICATION IS LIMITED TO THOSE STANDARDS AND GUIDELINES PER THIS SHEET. CONSTRUCTION IS SUBJECT TO CONSTRUCTION DRAWINGS, SHOP DRAWINGS, AND DESIGN ENGINEER'S CERTIFICATION SHEET.

REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Lacey</i>	DATE	01/01/2015
APPROVED	<i>James J. Brou</i>	DATE	01/01/2015
APPROVED	<i>James L. Carls</i>	DATE	01/01/2015

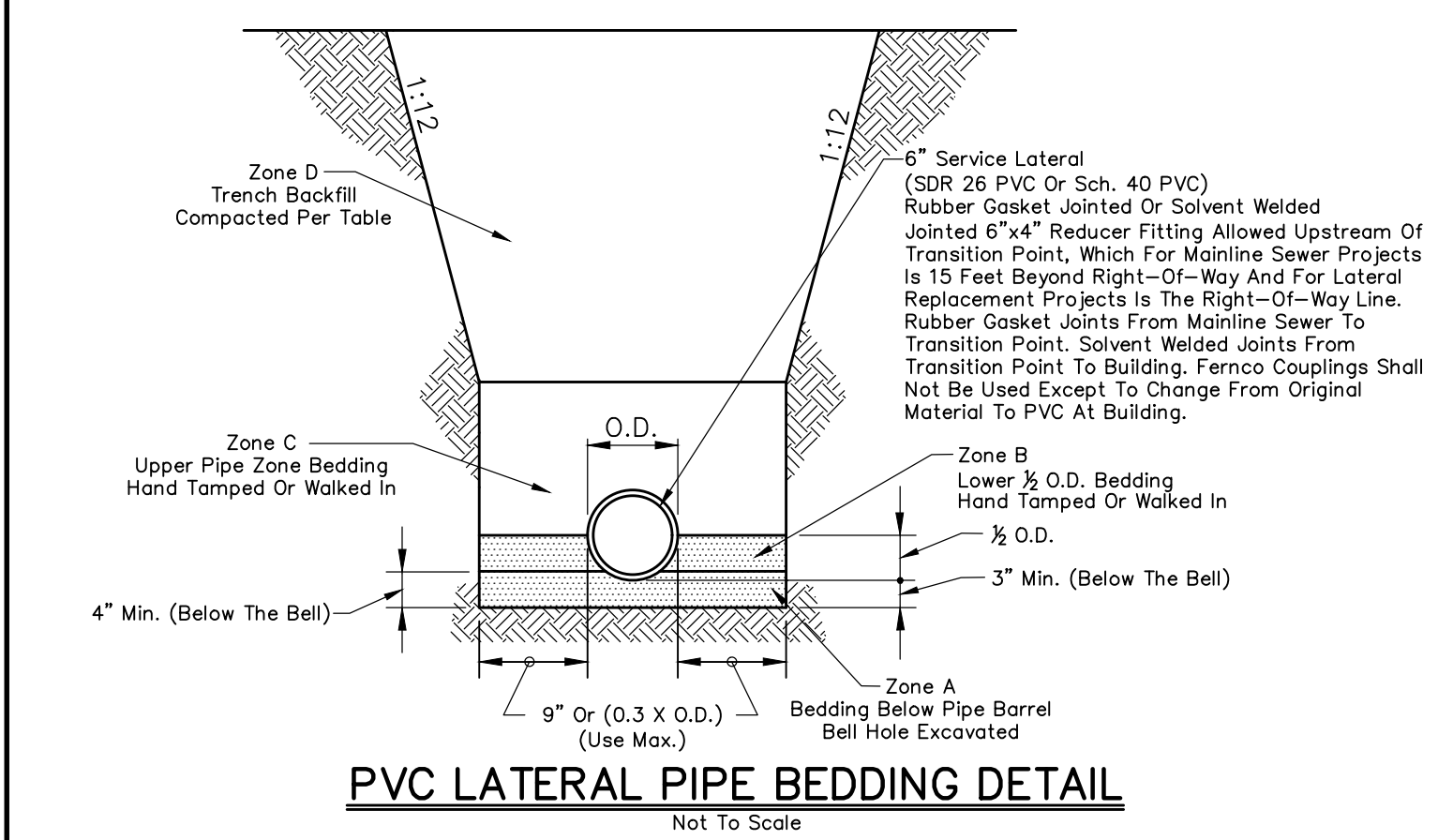
TOWN OF PLAINFIELD	SHEET 17 OF 29
SANITARY SEWER LIFT STATION STANDARDS & GUIDELINES	



PVC LATERAL PIPE BEDDING & BACKFILL TABLE**					
Bedding/Backfill Zone As Indicated On Detail	Back Of Curb To Back Of Curb	Planting Strip Or Existing Sidewalk	Private Property For Repair/Replace	Future Sidewalk Under 6 Month Rule*	Private Property Under 6 Month Rule*
Zone D Trench Backfill Compacted Per Table	Flowable Fill Or Same As Zone 'B'	Flowable Fill Or Same As Zone 'B'	Approved Excavated Material @ 85% Standard Proctor	Approved Excavated Material @ 85% Standard Proctor	Approved Excavated Material @ 85% Standard Proctor
Zone C Upper Pipe Zone Bedding Hand Tamped Or Walked In	Flowable Fill Or Same As Zone 'B'	Flowable Fill Or Same As Zone 'B'	"B"-Borrow Or Well-Graded Sand	Coarse Aggregate No. 8	Coarse Aggregate No. 8
Zone B Lower 1/2 O.D. Bedding Hand Tamped Or Walked In	Coarse Aggregate No. 8	Coarse Aggregate No. 8	"B"-Borrow Or Well-Graded Sand	Coarse Aggregate No. 8	Coarse Aggregate No. 8
Zone A Bedding Below Pipe Barrel Bell Hole Excavated	Coarse Aggregate No. 8	Coarse Aggregate No. 8	"B"-Borrow Or Well-Graded Sand	Coarse Aggregate No. 8	Coarse Aggregate No. 8

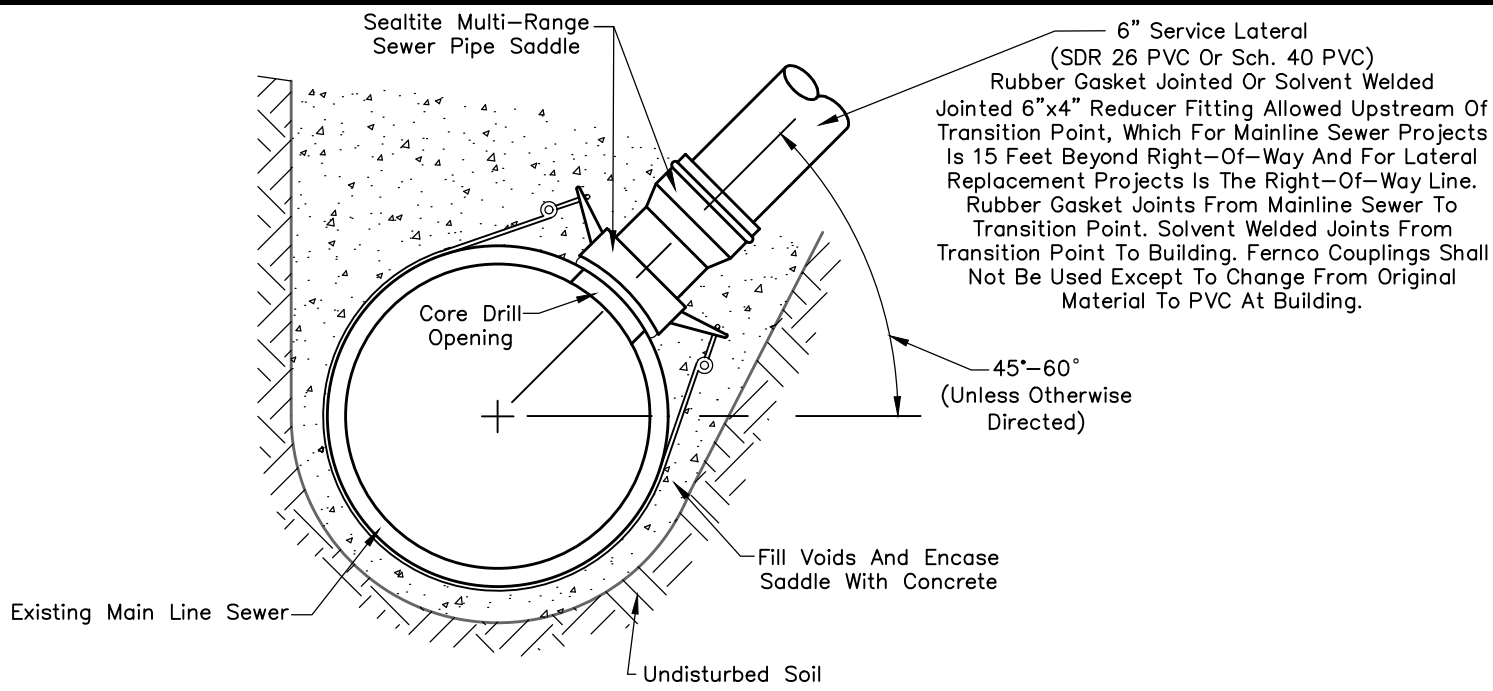
\*Approved Excavated Material May Be Used Under Proposed Sidewalks Provided Sidewalks Are Constructed 6 Months After Backfilling Of Trench And As Such Any Additional Lateral Pipe Built On Private Property Under Initial Sewer Construction Shall Be In Accordance With "Private Property Under 6 Month Rule" Column.

\*\*The PVC Lateral Pipe Bedding And Backfill Table Is Intended To Show Minimum Material Requirements. Flowable Fill May Be Used For Any Zone C, Or Zone D Work. "B"-Borrow May Be Used Whenever Excavated Material Is Required By Table. #8 Crushed Stone Or #8 Fractured Face Aggregate May Be Used Whenever "B"-Borrow Is Required By Table.



PVC LATERAL PIPE BEDDING DETAIL

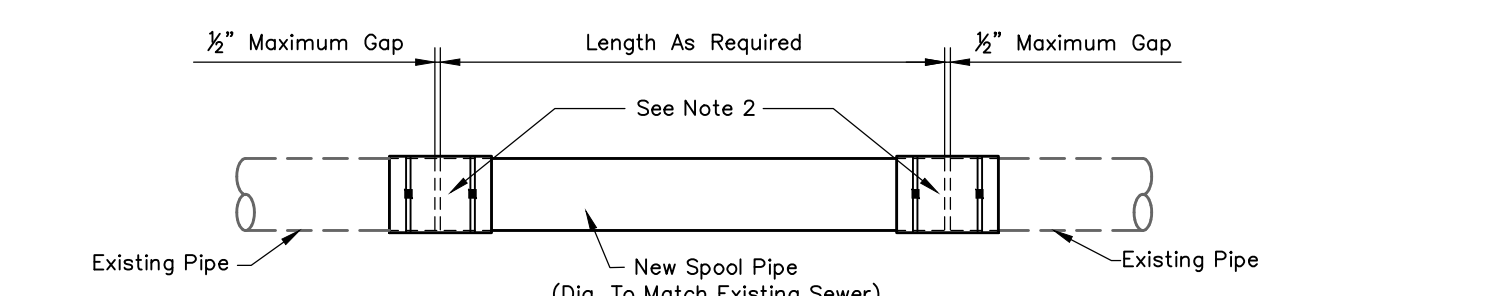
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- NOTE:
- Sewer Pipe Saddle Shall Be General Engineering Company Sealrite Type "U" For Laterals Connecting To Existing Mainline Sanitary Sewer With A 6.275" OD To 30.00" OD.
  - Sewer Pipe Saddle Shall Be General Engineering Company Sealrite Type "C" For Laterals Connecting To Existing Mainline Sanitary Sewer Over 30.00" OD.

SANITARY LATERAL SADDLE TAP

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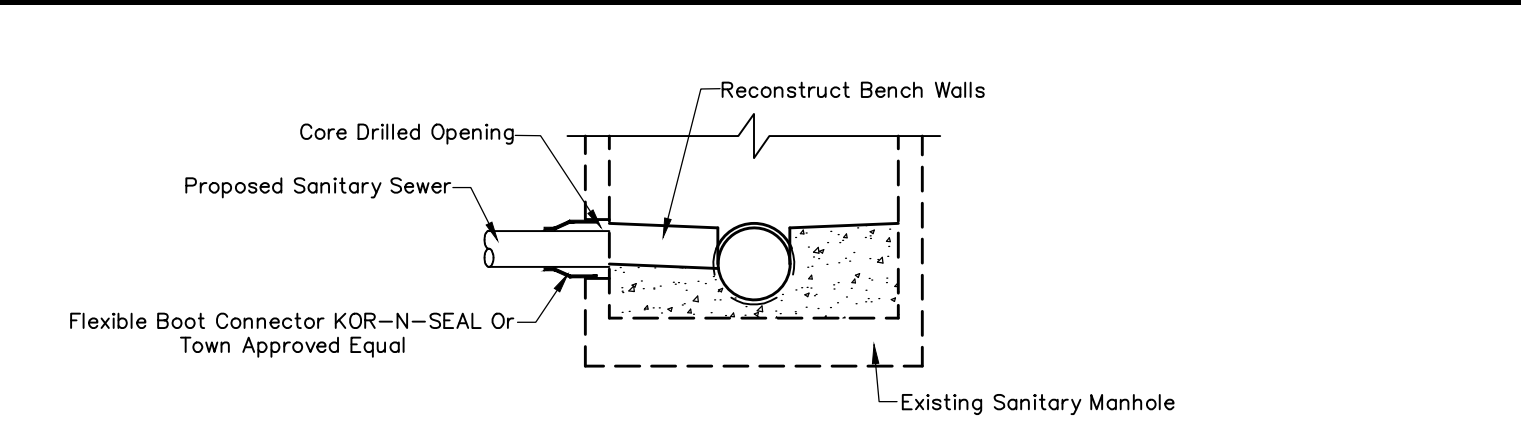


- NOTE:
- Cut Existing Pipe(s) On Both Sides Of The Existing Lateral Service. Remove Existing Wye/Fitting(s) And Pipe(s) Section And Install New Spool Pipe As Detailed Above.
  - Fernco Type 1000 RC Strong Back Couplings Are Required Where Plastic (PVC) Is Connected To Clay Or Cast Iron Pipe.

Where New PVC Pipe Is Being Connected To Existing PVC Pipe A Ductile Iron Repair Sleeve With Romac Grip Rings And IPS Transition Gaskets Shall Be Used.

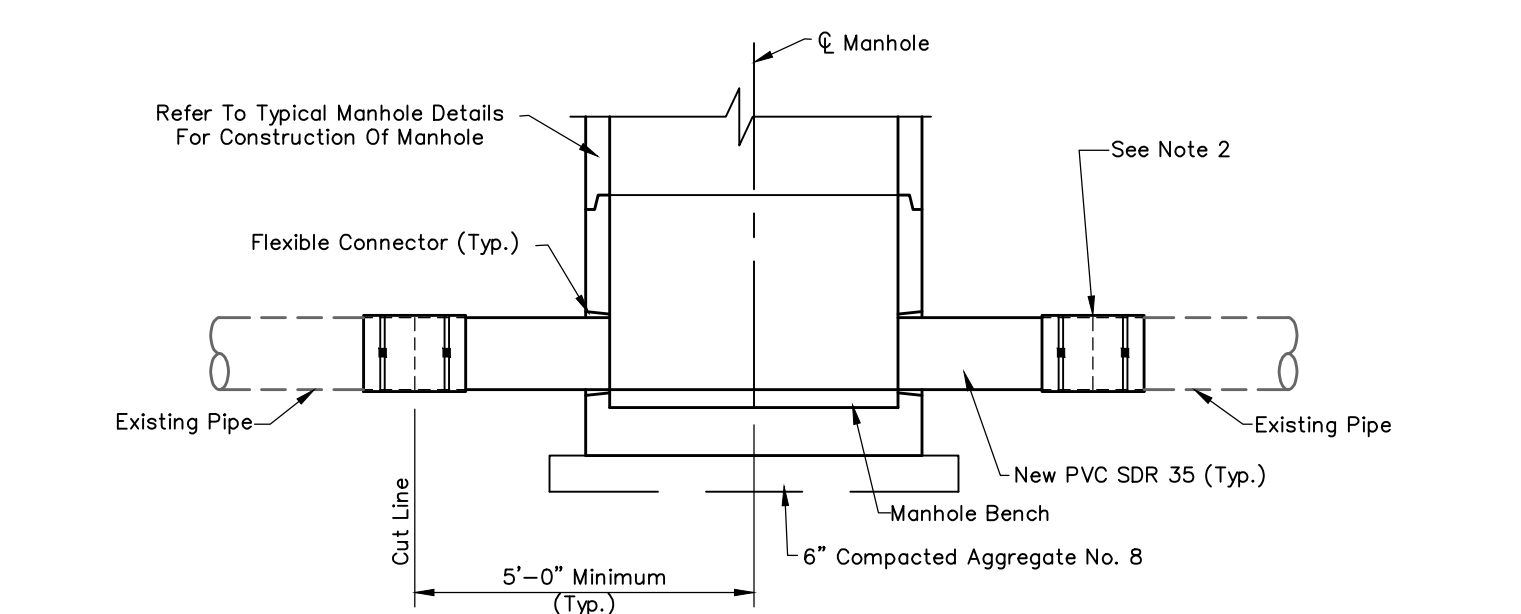
EXISTING SANITARY LATERAL ABANDONMENT

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EXISTING MANHOLE CONNECTION DETAIL

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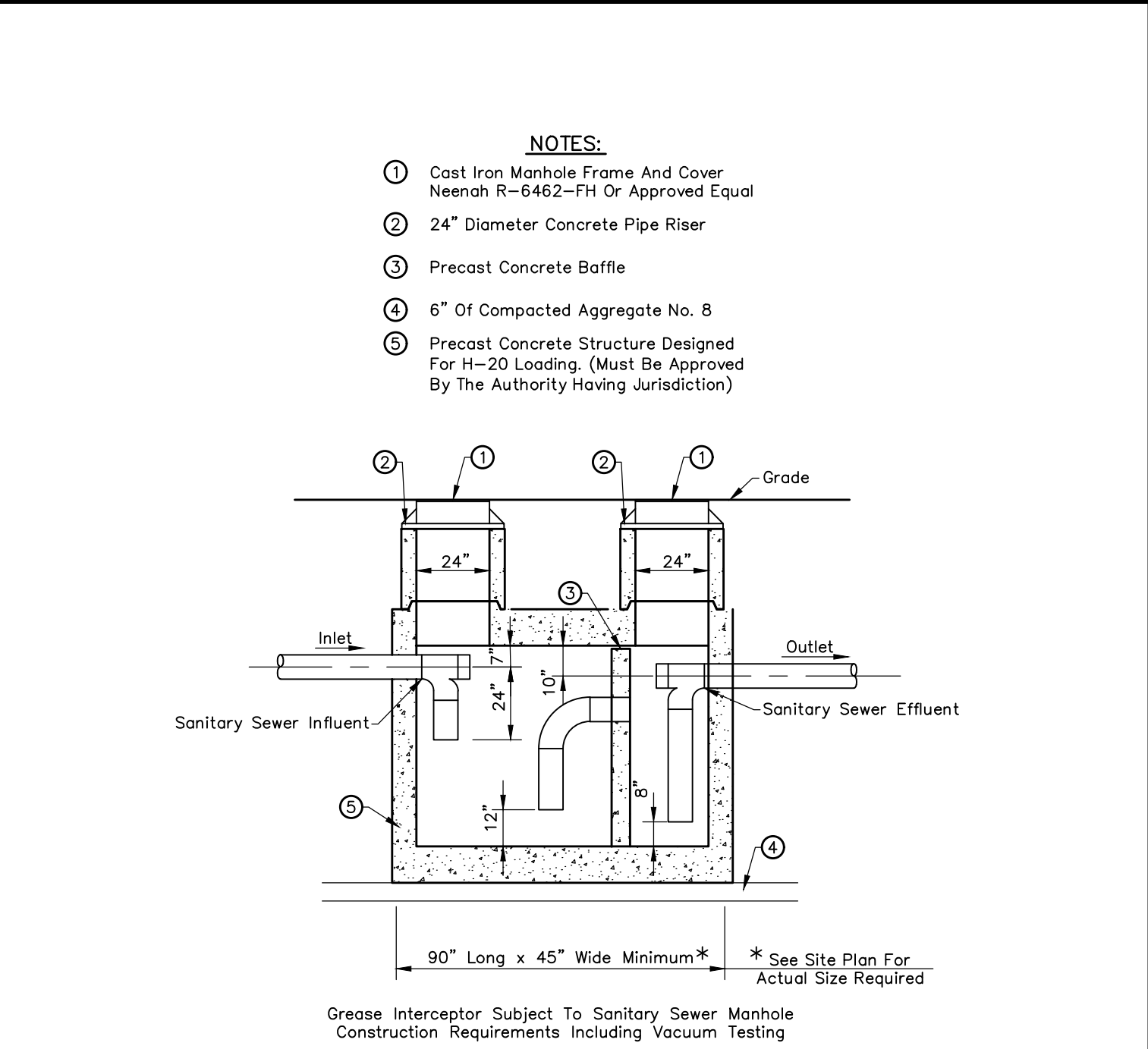


- NOTE:
- Cut Existing Pipe(s) On The Side Of The Proposed Manhole. Remove Existing Pipe(s) Section And Install Manhole Base. Proceed With Typical Connections And Manhole Construction.
  - Fernco Type 1000 RC Strong Back Couplings Are Required Where Plastic (PVC) Is Connected To Clay Or Cast Iron Pipe.

Where New PVC Pipe Is Being Connected To Existing PVC Pipe A Ductile Iron Repair Sleeve With Romac Grip Rings And IPS Transition Gaskets Shall Be Used.

SPECIAL MANHOLE CONNECTION DETAIL

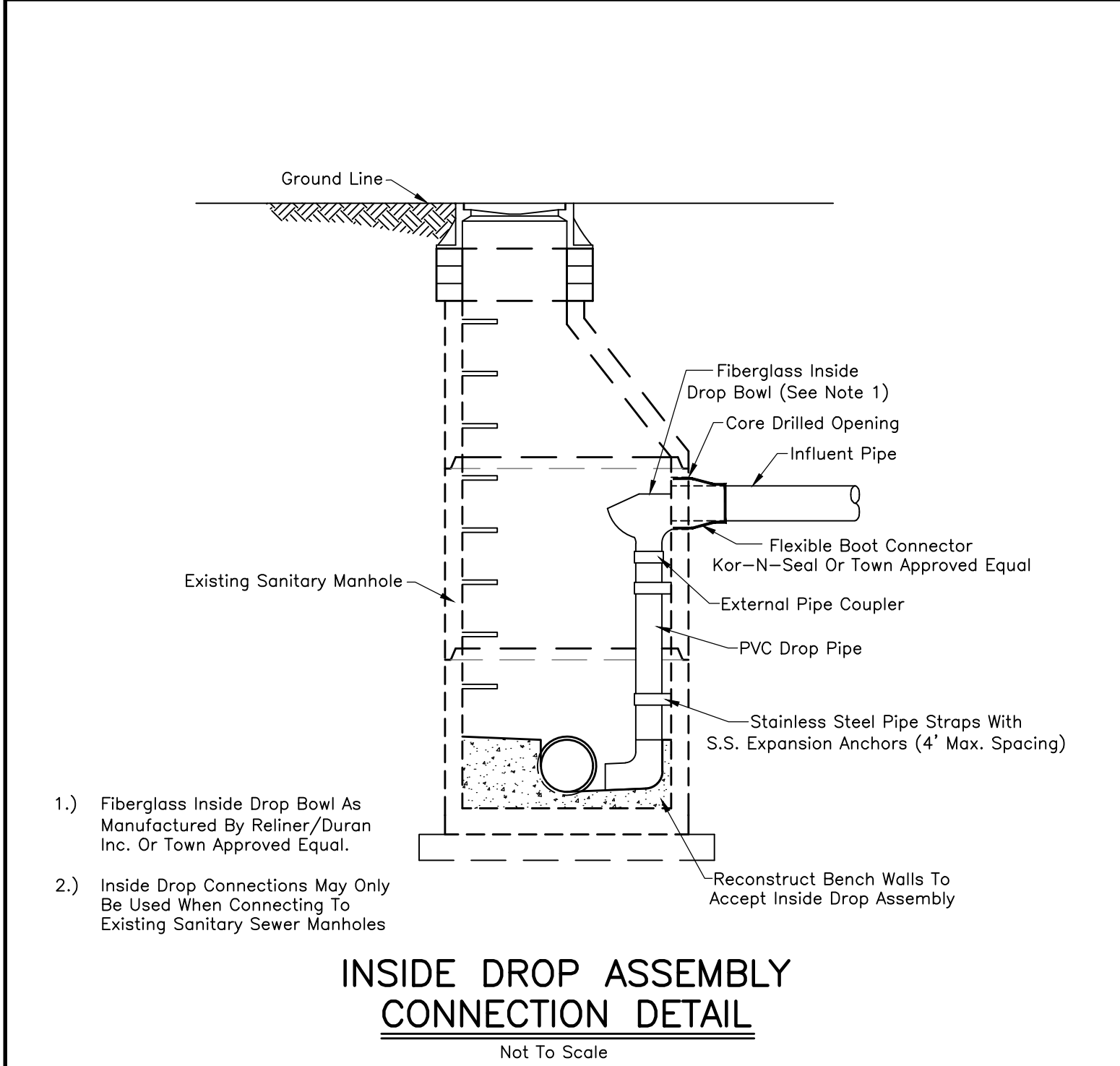
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GREASE INTERCEPTOR DETAIL

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DEVELOPMENT STANDARD – DETAIL DS-S01

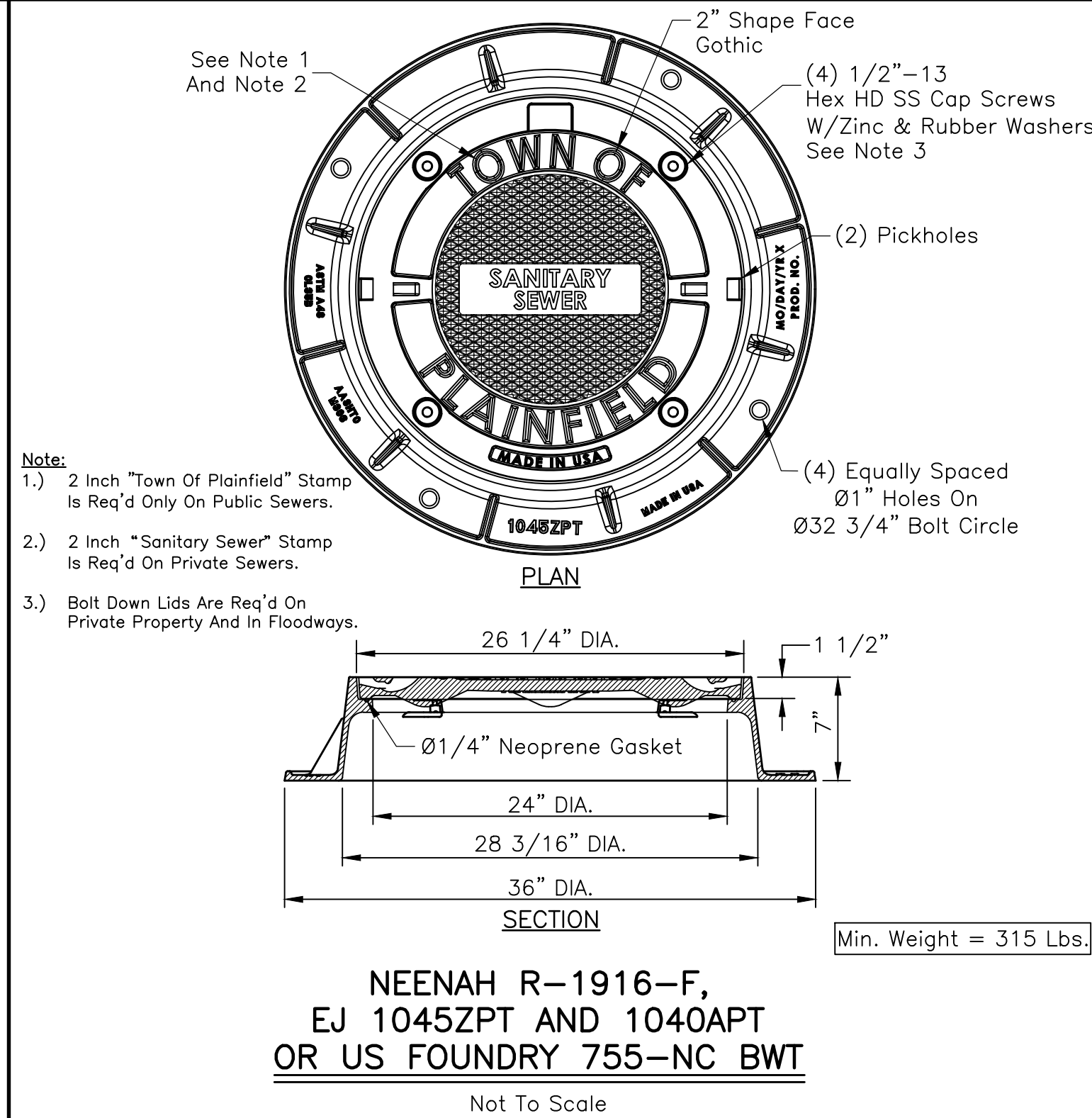


INSIDE DROP ASSEMBLY CONNECTION DETAIL

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DEVELOPMENT STANDARD – DETAIL DS-S05

DEVELOPMENT STANDARD – DETAIL DS-S02

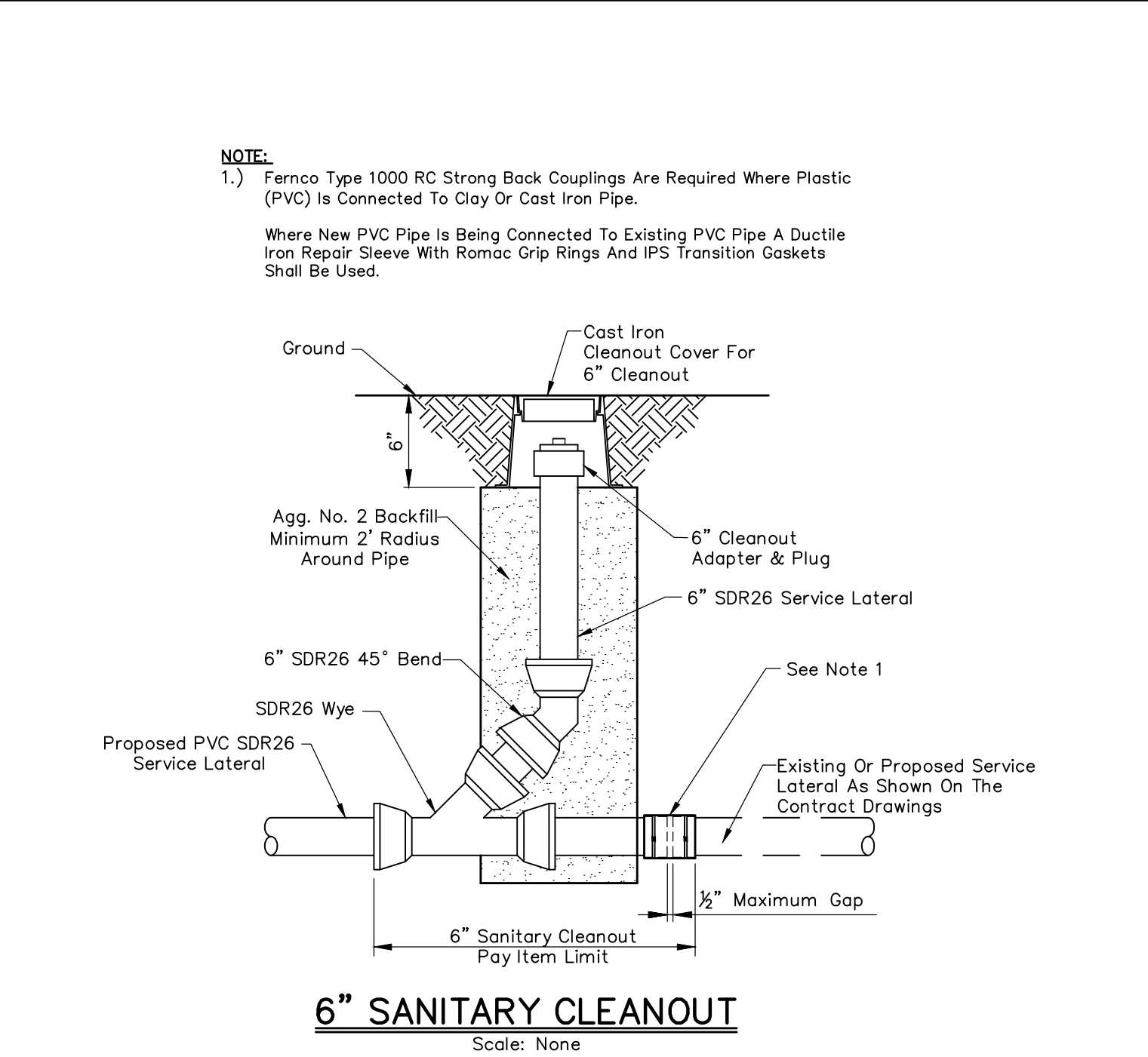


NEENAH R-1916-F, EJ 1045ZPT AND 1040APT OR US FOUNDRY 755-NC BWT

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DEVELOPMENT STANDARD – DETAIL DS-S06

DEVELOPMENT STANDARD – DETAIL DS-S03

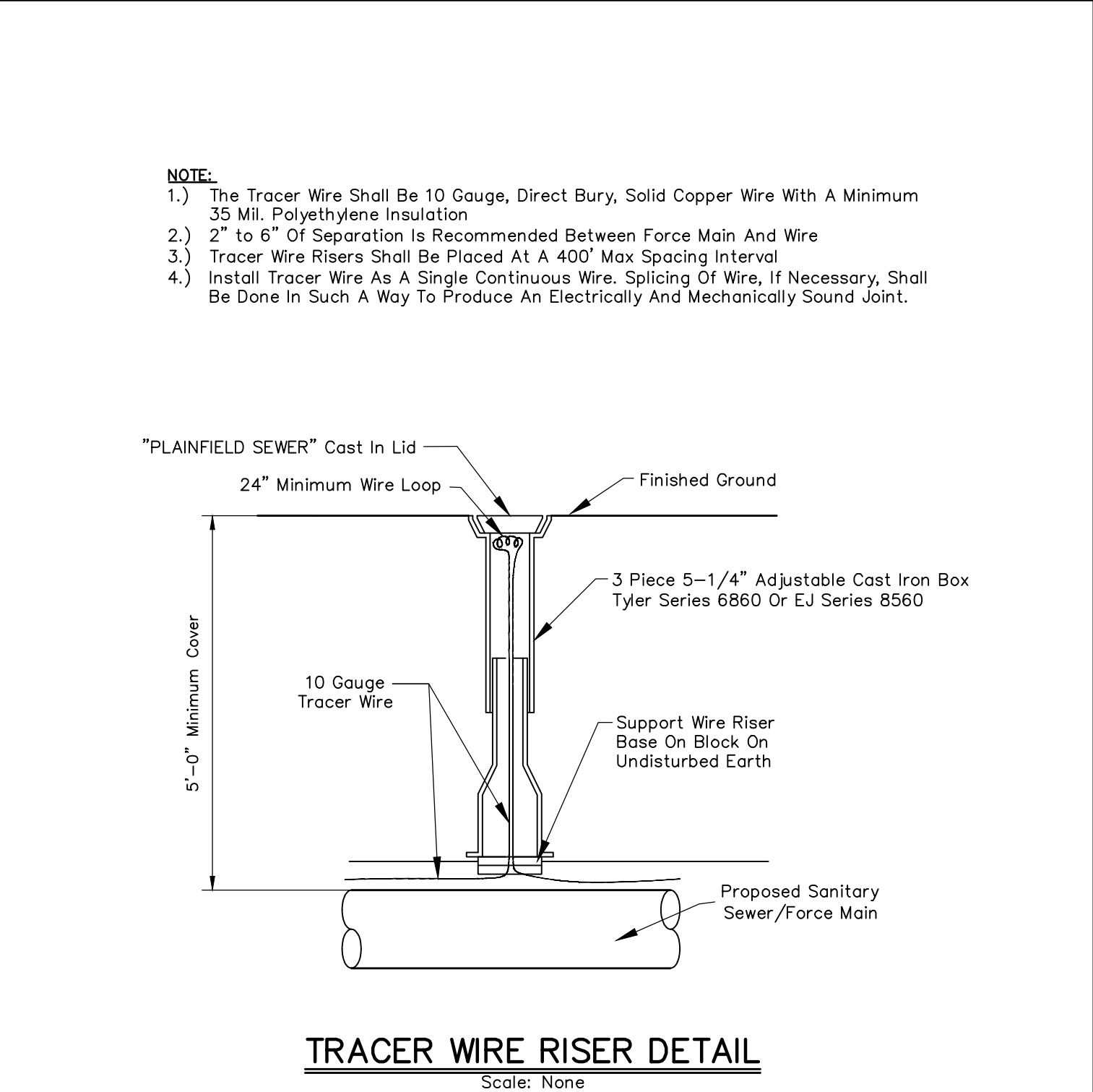


6" SANITARY CLEANOUT

Scale: None

DEVELOPMENT STANDARD – DETAIL DS-S07

DEVELOPMENT STANDARD – DETAIL DS-S04

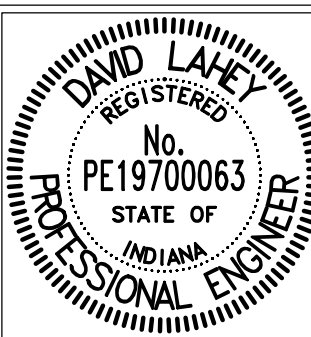


TRACER WIRE RISER DETAIL

Scale: None

DEVELOPMENT STANDARD – DETAIL DS-S08

REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Lutz</i>	01/01/2015
DESIGN ENGINEER		DATE
APPROVED	<i>James J. Brou</i>	01/01/2015
EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES		DATE
APPROVED	<i>Jason Caruth</i>	01/01/2015
DIRECTOR OF PUBLIC WORKS		DATE

TOWN OF PLAINFIELD

SANITARY SEWER (S) DEVELOPMENT STANDARDS

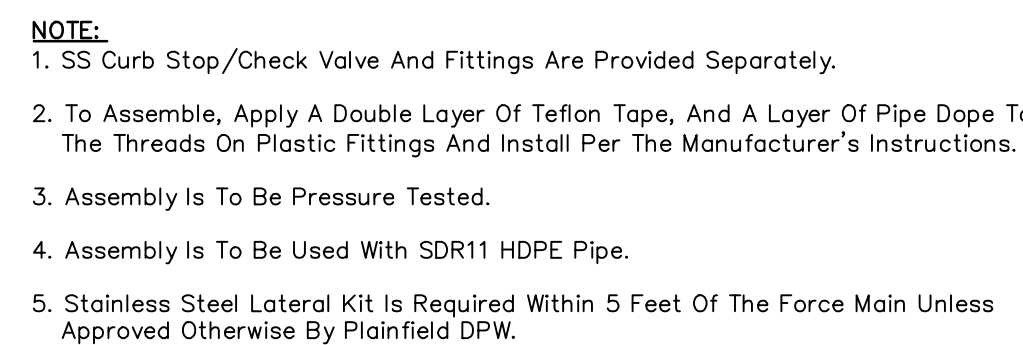
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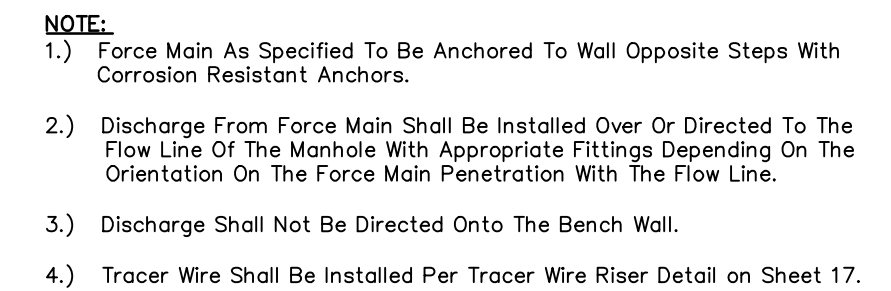
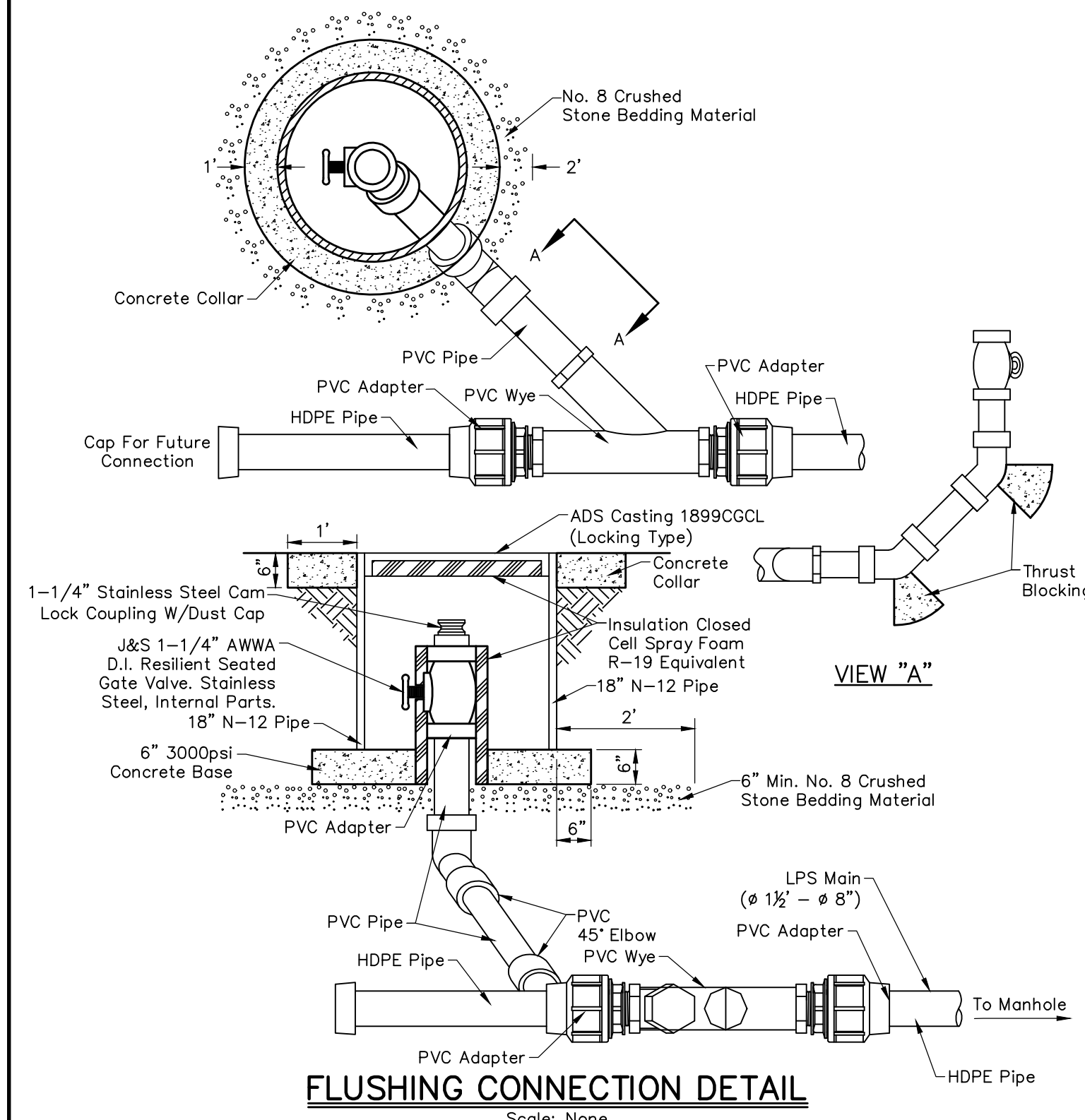


1. Calculations Shall Be Developed For LPS System Design Including The Following:
  - a. Topographical Map
  - b. Soil Conditions
  - c. Frost Depth
  - d. Water Table
  - e. Applicable Codes
  - f. Discharge Location
  - g. Lot Layout
  - h. Total Number Of Lots
  - i. Dwelling Types
  - j. Use And Flow Factors
  - k. Area Development Sequence And Timetable
2. Grinder Pumps Shall Be Sized Based Upon Recommended Flow In GPD And Must Consider The Following:
  - a. Wet Well And Discharge Piping Must Be Protected From Freezing
  - b. Model And Basin Size Must Be Appropriate For Incoming Peak Flows
  - c. Appropriate Alarm Devices Must Be Used
3. Grinder Pumps Shall Be Owned By The Property Owner, Not The Town Of Plainfield.
4. Power For Grinder Pumps Shall Be Provide By Property Owner.
5. Pipe Shall Be Either PVC SDR 21 Or HDPE DR 11.
6. Air/Vacuum Valves Shall Be Installed At All System High Points And Significant Changes In Grade.
7. Air Release Valves Shall Be Installed At Intervals Of 2,000 Feet On All Horizontal Runs That Lack A Clearly Defined High Point.
8. Air Release Valves Shall Be Installed At The Beginning Of Each Downward Leg In The System That Exhibits A 30-Foot Or More Drop.
9. Cleanout And Flushing Stations Shall Be Incorporated Into The Pipe Layout. Cleanouts Shall Be Installed At The Terminal End Of Each Main, At Every 1,000 Feet On Straight Runs Of Pipe, And Whenever Two Or More Mains Come Together And Feed Into Another Main.
10. A Pipe Schedule And Zone Analysis Shall Be Developed To Ensure The Design Conforms With A Criteria Of Flow Velocity Greater Than Or Equal To 2.0 Feet Per Second And Total Design Head Of Less Than Or Equal To 185 Feet.

## Scale: None



Scale: None



Scale: None

Technical drawing of a 70 Gallon Capacity Semi-Positive Displacement Type Pump. The drawing shows a vertical cylindrical HDPE tank with a corrugated dual wall. Key components labeled include:

- Gasketed Lid, HDPE
- Strain Relief Cord Connector
- Protective Cable Shroud (HDPE)
- Power/Alarm Cable 12-6 W/GND.
- E/One Equalizer
- Electrical Quick Disconnect NEMA 6P (EOD)
- Quick Disconnect ASSY. (304 S.S.)
- S.S. Cast Ball Valve
- Discharge 1-1/4" FPT
- 1-1/4" Discharge Line (304 S.S.)
- Check Valve (NORYL)
- Anti-Siphon Valve (NORYL)
- HDPE Tank Dual Wall, Corrugated 70 Gallon Capacity
- 29.5" DIA.
- Internal Well Vent 2.0" DIA.
- Inlet, Grommet To Accept 4.50" O.D.PVC Pipe (Standard). Dust Cover Supplied For Shipment (Not Suitable For Burial)
- ALARM ON/OFF
- Dimensions:
  - 41.6" To Discharge
  - 14" 24 Gal.
  - 18" 32 Gal.
  - 26" 47 Gal.
  - 36" To Inlet

Semi-Positive Displacement Type Pump  
Each Directly Driven By A 1 HP Motor

Scale: None



## Scale: None

REVISIONS	
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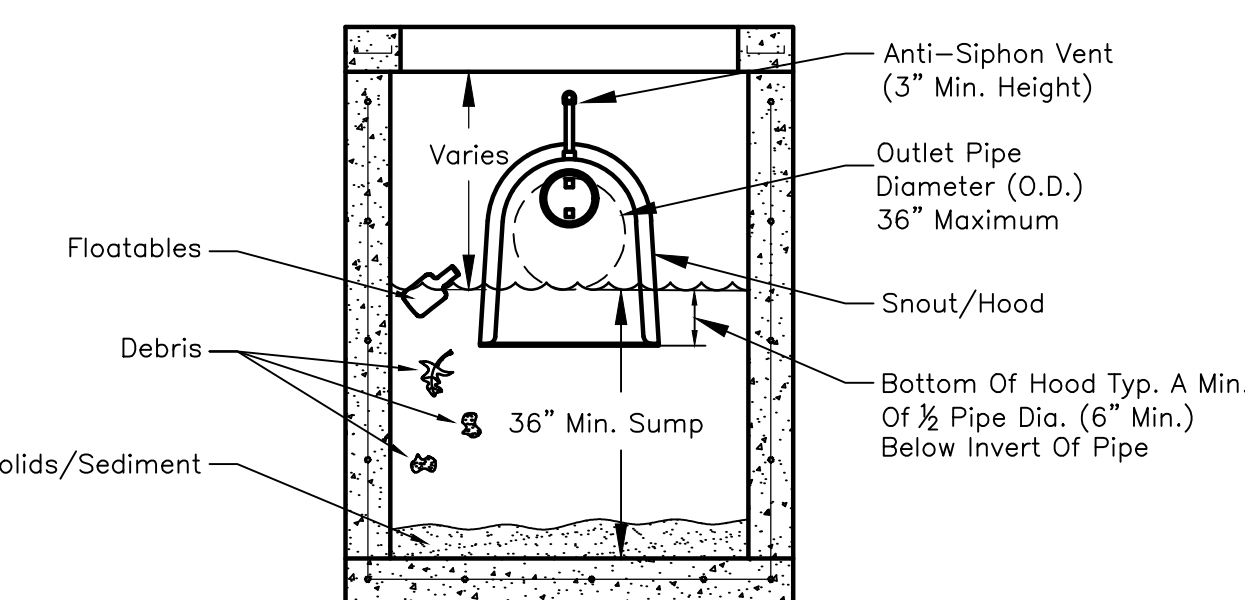
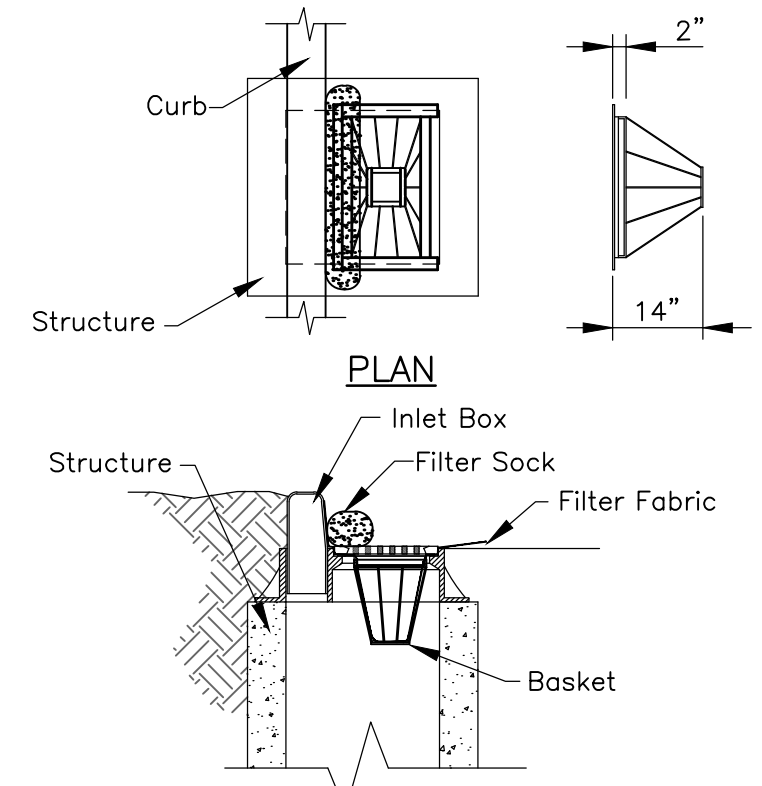
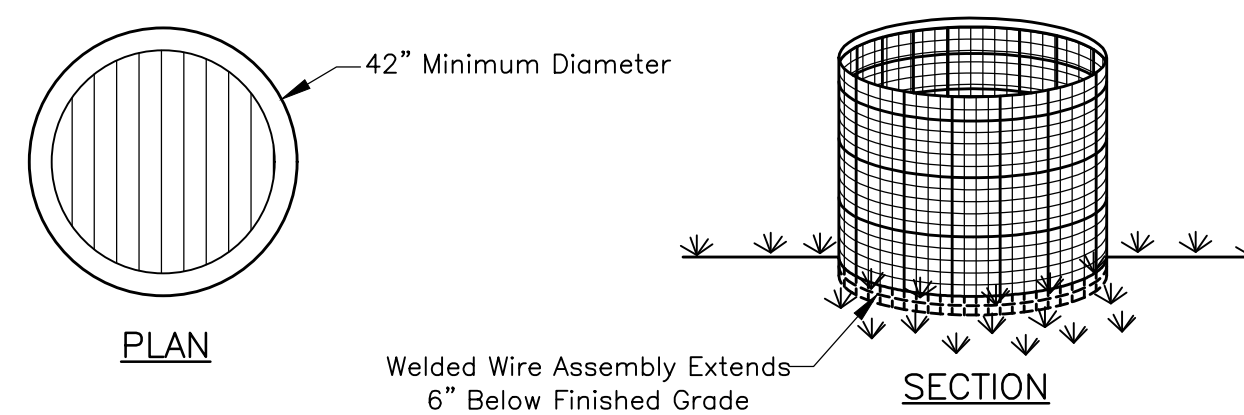
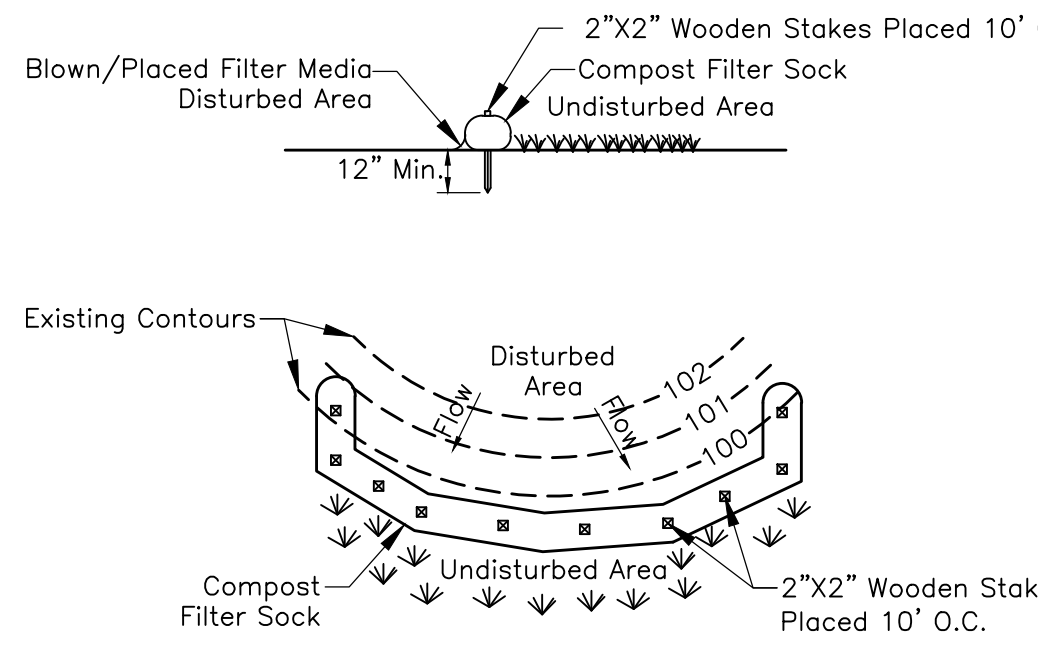
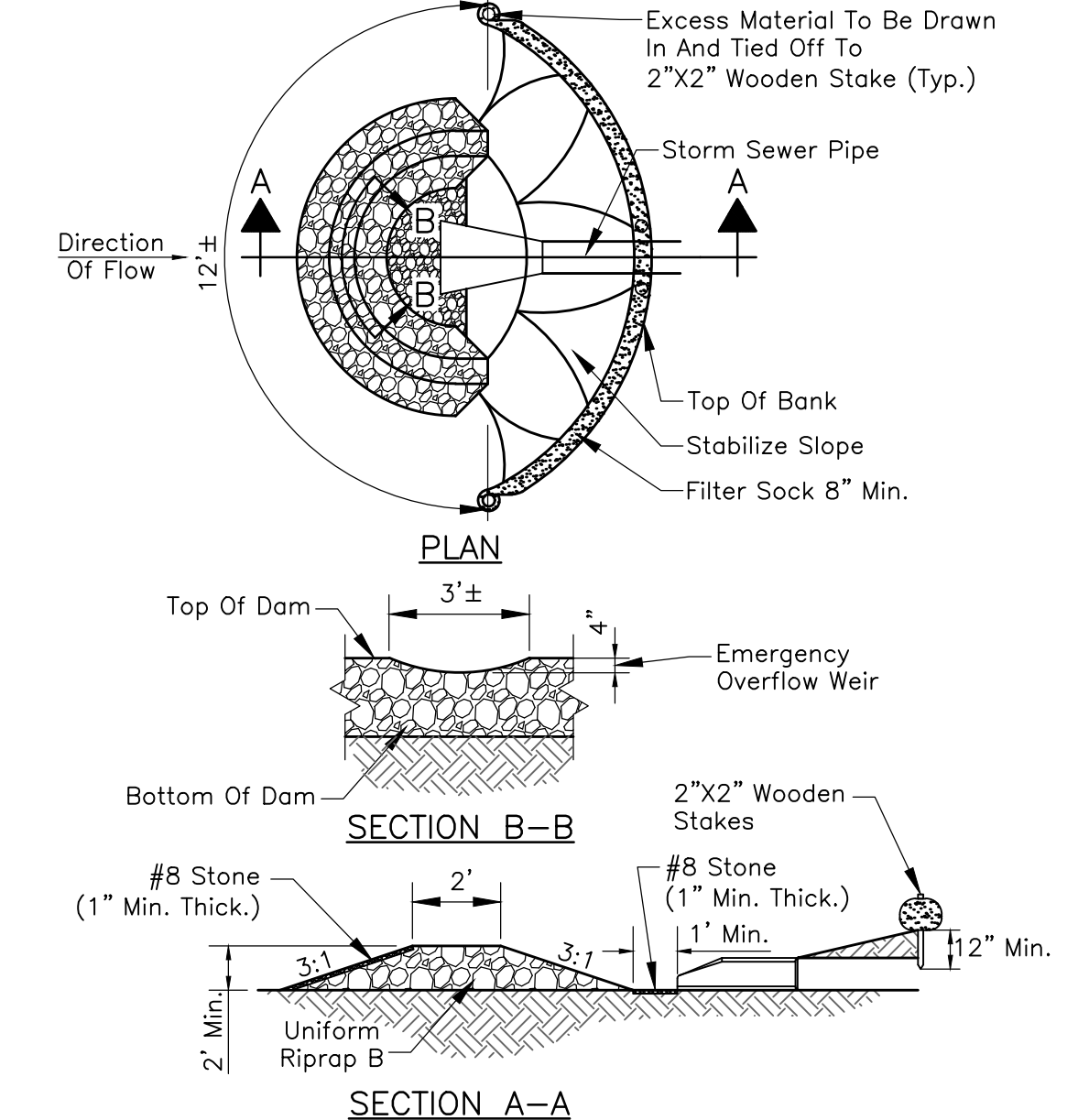
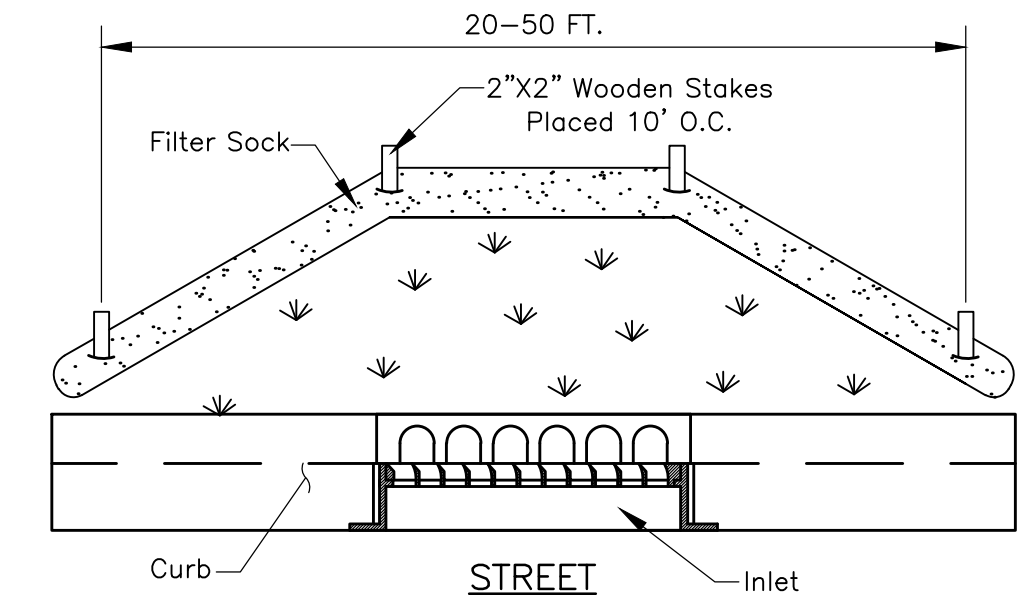
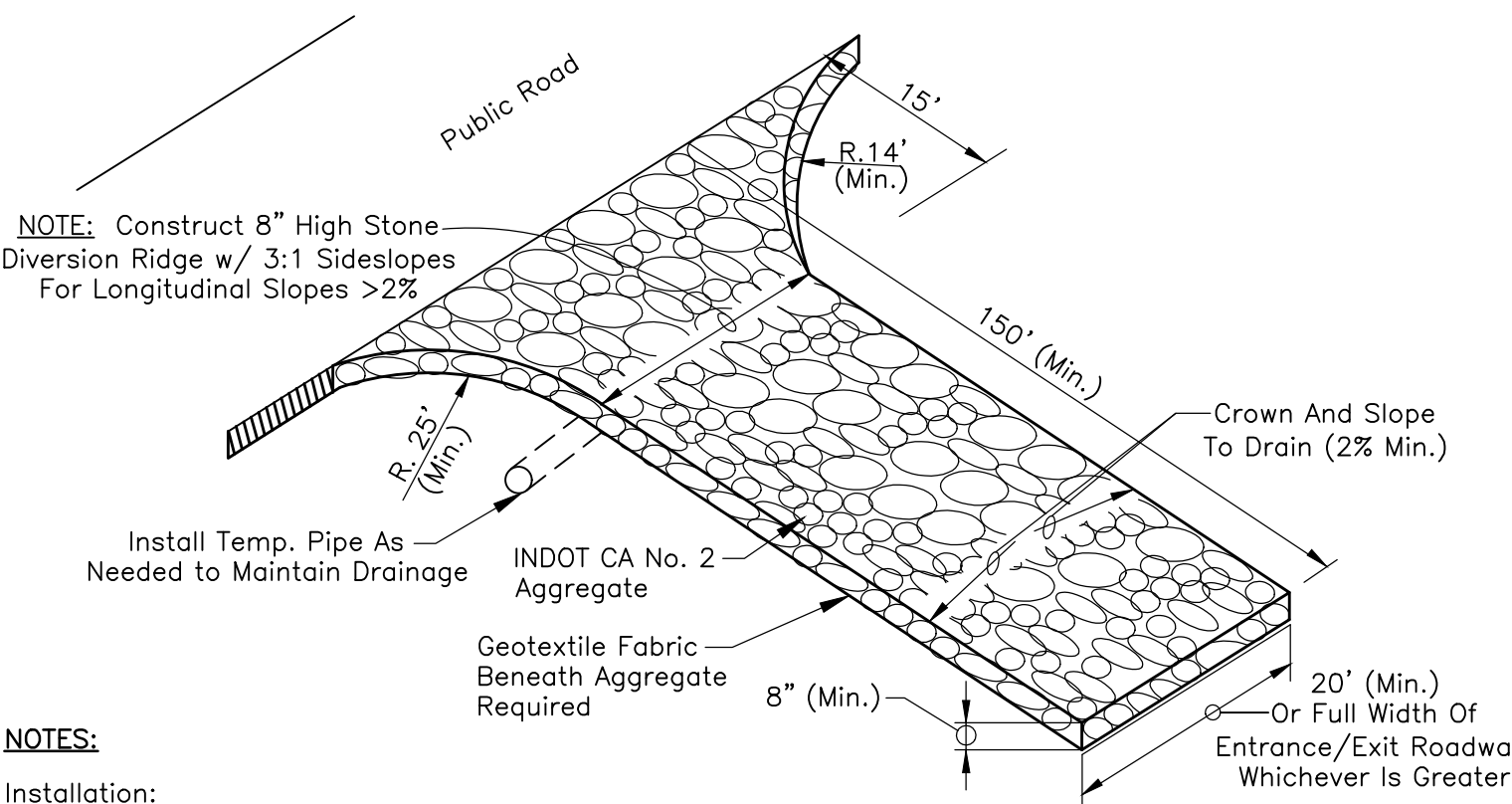
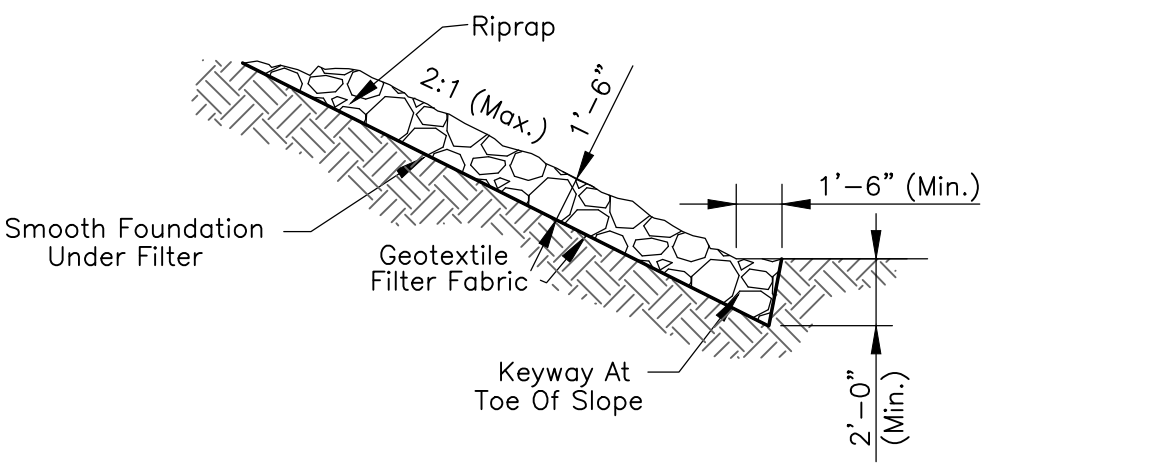
RECOMMENDED FOR APPROVAL	<i>David Lutz</i> DESIGN ENGINEER	01/01/2015 DATE
APPROVED	<i>S. J. [Signature]</i> EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES	01/01/2015 DATE
APPROVED	<i>Jason Carlett</i> DIRECTOR OF PUBLIC WORKS	01/01/2015 DATE

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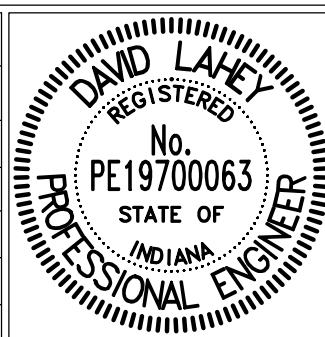






<div><p><b>NOTES:</b></p><p><u>Installation:</u></p><p>Snout/Hood Is Installed Over The Outlet Pipe Of A Catch Basin Or Stormwater Structure To Reduce Floatable Trash And Debris, Free Oils, And Other Solids From Stormwater Discharges.</p><p>Snout/Hood Shall Be Centered And Anchored Over The Outlet Pipe And Must Cover The Pipe O.D. To Ensure Proper Installation.</p><p>Structure Shall Be Sumped To Manufacturer's Recommended Depth. Minimum Sump Depth Is Typically 2.5 To 3 Times The I.D. Of The Outlet Pipe Size (Minimum Of 36").</p><p>Snout/Hood Shall Be Equipped With An Anti-Siphon Vent.</p><p>Maximum Flow And Velocity Shall NOT Exceed Manufacture's Recommendation.</p><p><u>Maintenance:</u></p><p>Sediment Depth And Surface Pollutants In The Sump Shall Be Measured Monthly And After Each Rain Event Of 8" or More.</p><p>The Sump Shall Be Emptied At Least Yearly And When The Sump Is Half Full, Or Six Inches Of Floatable Pollutants Accumulate On The Surface.</p><p>The Snout/Hood Shall Be Inspected Yearly And The Anti-Siphon Vent Shall Be Flushed To Ensure It Is Clear.</p><p><b>SNOUT/HOOD OIL WATER DEBRIS SEPARATOR</b></p><p>Not To Scale</p></div>	<div><p><b>NOTES:</b></p><p><u>Installation:</u></p><p>Install Basket Curb Inlet Protection As Soon As Inlet Boxes Are Installed (New Development) Or Prior To Land Disturbing Activities (Existing Development).</p><p>If Necessary, Adapt Basket Dimensions To Fit Inlet Box Dimensions.</p><p>Remove The Grate And Install The Frame Into The Grate Opening. Cut And Install Geotextile Fabric According To The Manufacturer's Recommendations. Replace The Grate. Install Filter Sock Across Inlet Box Opening.</p><p><u>Maintenance:</u></p><p>Inspect Daily And After Each Storm And Remove Sediment. Replace Or Clean Geotextile Fabric And Filter Sock As Needed. Remove Tracked On Sediment From The Street (But Not By Flushing With Water) To Reduce The Sediment Load On This Curb Inlet Practice.</p><p><b>BASKET CURB INLET PROTECTION</b></p><p>Not To Scale</p></div>	<div><p><b>NOTES:</b></p><p><u>Installation:</u></p><p>6" x 6" Welded Wire Mesh Shall Be Formed Of 10Ga. Steel Conforming To ASTM A-185.</p><p>Geotextile Shall Be Wrapped Three Inches Over The Top Member Of The 6" x 6" Welded Wire Mesh And Shall Be Secured With Fastening Rings Through Both Geotextile Layers And Close Around A Steel Member At Six Inches On Center. Fastening Rings Shall Be Constructed Of Wire Conforming To ASTM A-641, A-809, A-370, And A-938.</p><p>Geotextile Shall Be Secured To The Sides Of Welded Wire Mesh With Fastening Rings At A Spacing Of One Per Square Foot Except For The Bottom 2-Inches Which Shall Extend Past The Welded Wire And Be Left Unsecured For Entrenchment.</p><p>Welded Wire Assembly Shall Be Formed Into A Minimum 42" Diameter Circle With a 3" Minimum Overlap On The Ends Secured By Wire Or Zip Ties.</p><p>Welded Wire Assembly Shall Then Be Placed In A 6" Deep Trench And Backfilled And Compacted Over The Geotextile Flap.</p><p><u>Maintenance:</u></p><p>Inspect The Welded Wire Inlet Protector Weekly And After Each Rainfall Event.</p><p>If Geotextile Tears, Start To Decompose, Or In Any Way Becomes Ineffective, Replace The Affected Portion Immediately.</p><p>Remove The Deposited Sediment When It Reaches Half The Height Of The Structure At Its Lowest Point Or Is Causing The Structure To Shift. Take Care To Avoid Undermining The Structure During Clean Out.</p><p>After The Contributing Drainage Area Has Been Stabilized, Remove The Structure And Sediment Deposits, Bring The Disturbed Area To Grade, And Stabilize.</p><p><b>WELDED WIRE INLET PROTECTION</b></p><p>Not To Scale</p></div>	<div><p><b>NOTES:</b></p><p><u>Installation:</u></p><p>Filter Sock Should Maintain Solid Contact With The Soil And Be Installed In A Manner That Minimizes Gaps Between The Bottom Of The Sock And The Underlying Substrate.</p><p>Filter Socks Should Be Installed Parallel To The Contour With Both Ends Of The Sock Extended Upslope At A 45 Degree Angle To The Rest Of The Sock.</p><p>Socks Placed On Earthen Slopes Should Be Staked In The Center Of The Sock Or Immediately Downslope Of The Sock At The Interval Recommended By The Manufacturer. Socks Installed On Paved Surfaces Shall Have Concrete Blocks Placed Immediately Downslope Of The Sock At An Interval Recommended By The Manufacturer.</p><p><u>Maintenance:</u></p><p>Traffic Shall Not Be Permitted To Cross Filter Socks.</p><p>Inspect The Structure Weekly And After Each Rainfall Event. Damaged Socks Shall Be Repaired According To The Manufacturer's Specifications Or Replaced Within 24 Hours Of Inspection.</p><p>Remove Deposited Sediment When It Reaches Half The Height Of The Filter Sock At Its Lowest Point.</p><p>Take Care To Avoid Undermining The Filter Sock During Clean Out.</p><p>After The Contributing Drainage Area Has Been Stabilized, Remove And Properly Dispose Of Any Unstable Sediment And Construction Material, And Stabilize.</p><p><b>FILTER SOCK</b></p><p>Not To Scale</p></div>
<div>DEVELOPMENT STANDARD – DETAIL DS-E01</div>	<div>DEVELOPMENT STANDARD – DETAIL DS-E02</div>	<div>DEVELOPMENT STANDARD – DETAIL DS-E03</div>	<div>DEVELOPMENT STANDARD – DETAIL DS-E04</div>
<div><p><b>NOTES:</b></p><p><u>Installation:</u></p><p>Around the Outer Perimeter Of The Excavated Area, Lay A Ring of INDOT Uniform B Riprap To A Height Of 12 to 24 Inches Above The Top Of The Storm Drain. Foundation Shall Be Laid On Geotextile Fabric.</p><p><u>Maintenance:</u></p><p>Inspect The Structure Weekly And After Each Rainfall Event. After The Contributing Drainage Area Has Been Stabilized, Remove And Properly Dispose Of Any Unstable Sediment And Construction Material, And Stabilize.</p><p><b>ROCK DONUT</b></p><p>Not To Scale</p></div>	<div><p><b>NOTES:</b></p><p><u>Installation:</u></p><p>Filter Sock Is Not Recommended For Use As A Diversion And Should Not Be Used Across A Stream, Channel, Ditch, Swale, Or Anywhere That Concentrated Flow Is Anticipated.</p><p>Filter Sock Should Maintain Solid Contact With The Soil And Be Installed In A Manner That Minimizes Gaps Between The Bottom Of The Sock And The Underlying Substrate.</p><p><u>Maintenance:</u></p><p>Inspect The Silt Fence Weekly And After Each ½" Rainfall Event.</p><p>If Fence Fabric Tears, Starts To Decompose, Or In Any Way Becomes Ineffective, Replace The Affected Portion Immediately.</p><p>Inspect The Structure Weekly And After Each Rainfall Event. Damaged Socks Shall Be Repaired According To The Manufacturer's Specifications Or Replaced Within 24 Hours Of Inspection.</p><p>Remove Deposited Sediment When It Reaches Half The Height Of The Filter Sock At Its Lowest Point. Take Care To Avoid Undermining The Filter Sock During Clean Out.</p><p>After The Contributing Drainage Area Has Been Stabilized, Remove And Properly Dispose Of Any Unstable Sediment And Construction Material, And Stabilize.</p><p><b>FILTER SOCK BEHIND CURB</b></p><p>Not To Scale</p></div>	<div><p><b>NOTES:</b></p><p><u>Installation:</u></p><p>A Stable Construction Entrance Must Be Provided At All Points Of Construction Traffic Ingress And Egress To The Project Site. Avoid Locating On Steep Slopes Or At Curves In Public Roads.</p><p>Remove All Vegetation And Other Objectionable Material From The Foundation Area, And Grade The Foundation And Crown For Positive Drainage.</p><p>Install Pipe Under The Pad (If Needed) To Maintain Proper Public Road Drainage.</p><p>Place Geotextile Fabric On The Graded Foundation To Improve Stability.</p><p>Place Aggregate To Dimensions And Grade Shown On The Erosion Control Plan, Leaving The Surface Smooth And Sloped For Drainage.</p><p>Divert All Surface Runoff And Drainage From The Stone Pad To A Sediment Trap Or Basin.</p><p><u>Maintenance:</u></p><p>Reshape Pad And Topdress As Needed For Drainage And Runoff Control. Immediately Remove Mud And Sediment Tracked Or Washed Onto Public Roads By Brushing Or Sweeping. Flushing Should Only Be Used If The Water Is Conveyed Into A Sediment Trap Or Basin.</p><p><b>TEMPORARY GRAVEL CONSTRUCTION ENTRANCE</b></p><p>Not To Scale</p></div>	<div><p><b>NOTES:</b></p><p><u>Installation:</u></p><p>Excavate Only Deep Enough For Both Filter And Riprap. Compact Any Fill Material To The Density Of The Surrounding Undisturbed Soil.</p><p>Cut A Keyway In Stable Material At The Base Of The Slope To Reinforce The Toe. Keyway Depth Should Be 1 ½ Times The Design Thickness Of The Riprap, And Should Extend A Horizontal Distance Equal To The Design Thickness.</p><p>Place Geotextile Fabric On The Smoothed Foundation, Overlapping The Edges 12 Inches Minimum. Secure With Anchor Pins Spaced Every 3 Feet Along The Overlap.</p><p>Immediately After Installing The Filter, Add The Riprap To Full Thickness In One Operation. <b>Do Not Dump</b> Through Chutes Or Use Any Method That Causes Segregation Of Rock Sizes, Or That Will Dislodge Or Damage The Underlying Filter Material.</p><p>If Fabric Is Damaged, Remove The Riprap And Repair By Adding Another Layer Of Fabric, Overlapping The Damaged Area By 12 Inches.</p><p>Place Smaller Aggregate In Voids To Form A Dense, Uniform, Well Graded Mass. Blend The Riprap Surface Smoothly With The Surrounding Area To Eliminate Protrusions Or Over Falls.</p><p><u>Maintenance:</u></p><p>Inspect Periodically For Displaced Aggregate Material, Slumping And Erosion At Edges, Especially Downstream Or Downslope.</p><p><b>RIPRAP</b></p><p>Not To Scale</p></div>
<div>DEVELOPMENT STANDARD – DETAIL DS-E05</div>	<div>DEVELOPMENT STANDARD – DETAIL DS-E06</div>	<div>DEVELOPMENT STANDARD – DETAIL DS-E07</div>	<div>DEVELOPMENT STANDARD – DETAIL DS-E08</div>

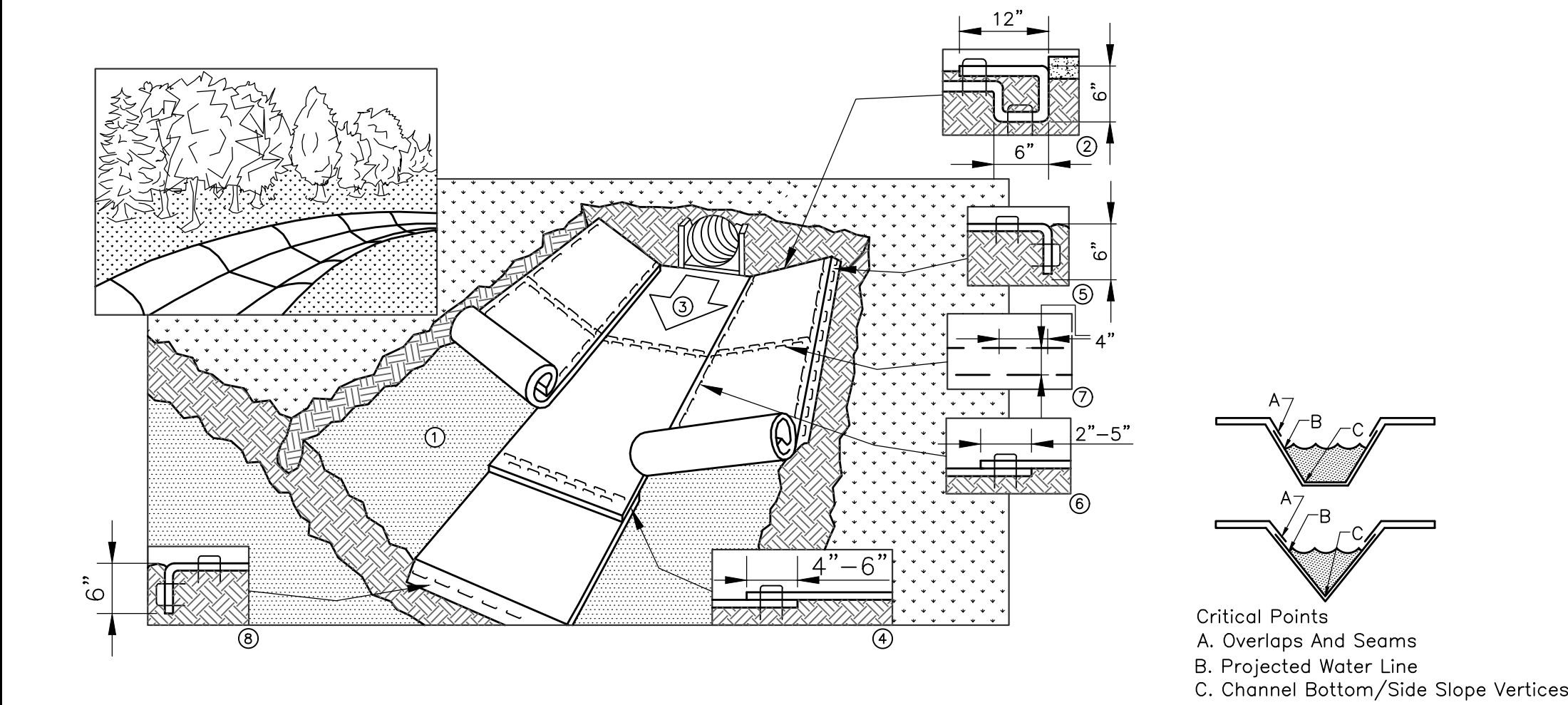
REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Lutz</i>	01/01/2015
	DESIGN ENGINEER	DATE
APPROVED	<i>Sharon Cham</i>	01/01/2015
	EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES	DATE
APPROVED	<i>Sharon Cham</i>	01/01/2015
	MS4 OPERATOR	DATE

TOWN OF PLAINFIELD	SHEET 21 OF 29
EROSION CONTROL (E) DEVELOPMENT STANDARDS	

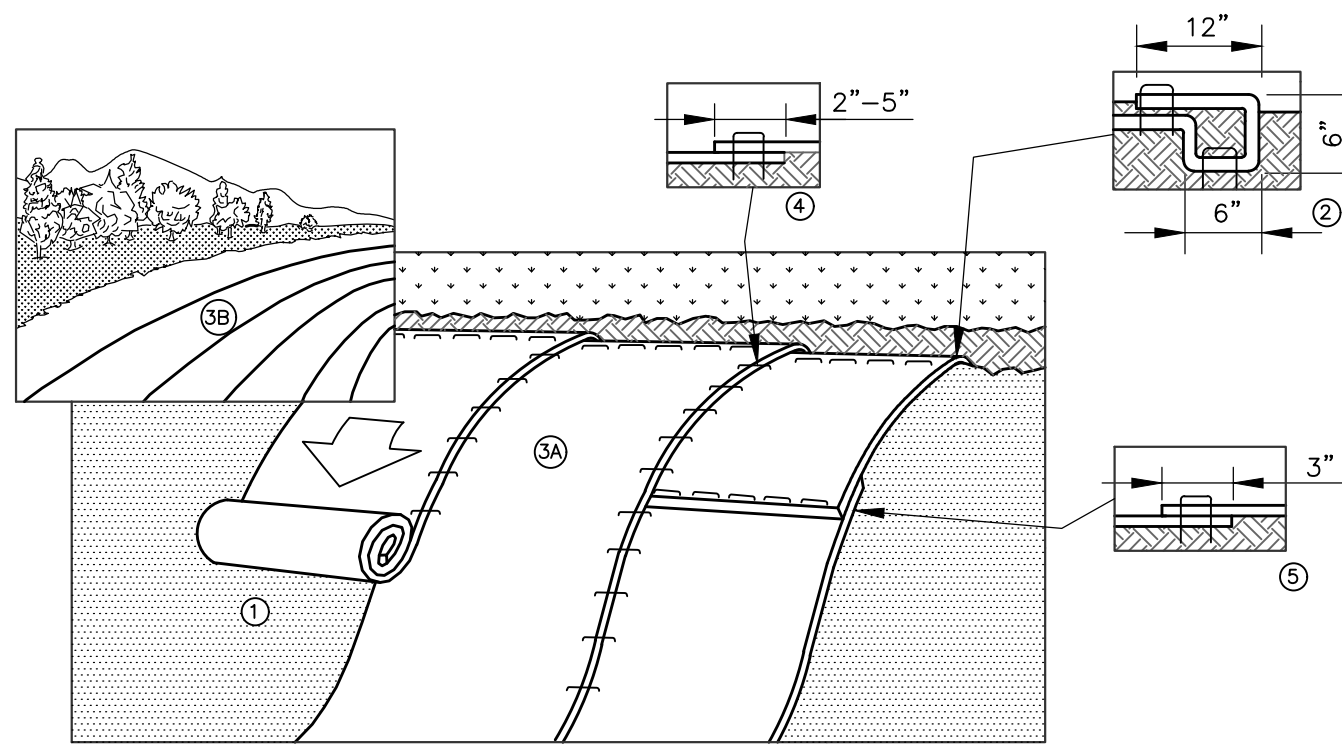




- ① Prepare Soil Before Installing Blankets, Including Any Necessary Application Of Lime, Fertilizer, Or Seed.
  - ② Begin At The Top Of The Channel By Anchoring The Blanket In A 6 Inch Deep By 6 Inch Wide Trench With Approximately 12 Inches Of Blanket Extended Beyond The Upslope Portion Of The Trench. Anchor The Blanket With A Row Of Staples/Stakes Approximately 12 Inches Apart In The Bottom Of The Trench. Backfill And Compact The Trench After Stapling. Apply Seed To Compacted Soil And Fold Remaining 12 Inch Portion Of Blanket Back Over Seed And Compacted Soil. Secure Blanket Over Compacted Soil With A Row Of Staples/Stakes Spaced Approximately 12 Inches Apart Across The Width Of The Blanket.
  - ③ Roll Center Blanket In Direction Of Water Flow In Bottom Of Channel. Blankets Will Unroll With Appropriate Side Against The Soil Surface. All Blankets Must Be Securely Fastened To Soil Surface By Placing Staples/Stakes In Appropriate Locations As Shown In The Staple Pattern Guide. When Using Optional Dot System, Staples/Stakes Should Be Placed Through Each Of The Colored Dots Corresponding To The Appropriate Staple Pattern.
  - ④ Place Consecutive Blankets End Over End (Shingle Style) With A 4-6 Inch Overlap. Use A Double Row Of Staples Staggered 4 Inches Apart And 4 Inches On Center To Secure Blankets.
  - ⑤ Full Length Edge Of Blankets At Top Of Side Slopes Must Be Anchored With A Row Of Staples/Stakes Approximately 12 Inches Apart In A 6 Inch Deep By 6 Inch Wide Trench. Backfill And Compact The Trench After Stapling.
  - ⑥ Adjacent Blankets Must Be Overlapped Approximately 2-5 Inches, (Depending On Blanket Type) And Stapled. To Ensure Proper Seam Alignment, Place The Edge Of The Overlapping Blanket (Blanket Being Installed On Top) Even With The Colored Seam Stitch On The Blanket Being Overlapped.
  - ⑦ In High Flow Channel Applications, A Staple Check Slot Is Recommended At 30-40 Foot Intervals. Use A Double Row Of Staples Staggered 4 Inches Apart And 4 Inches On Center Over Entire Width Of The Channel.
  - ⑧ The Terminal End Of The Blankets Must Be Anchored With A Row Of Staples/Stakes Approximately 12 Inches Apart In A 6 Inch Deep By 6 Inch Wide Trench. Backfill And Compact The Trench After Stapling.
- NOTE:  
\* Horizontal Staple Spacing Should Be Altered If Necessary To Allow Staples To Secure The Critical Points Along The Channel Surface.  
\*\* In Loose Soil Conditions, The Use Of Staple Or Stake Lengths Greater Than 6 Inches May Be Necessary To Properly Anchor The Blankets.

#### EROSION CONTROL BLANKET – FLOWLINE APPLICATION

Not To Scale



- ① Prepare Soil Before Installing Blankets, Including Any Necessary Application Of Lime, Fertilizer, And Seed.
- ② Begin At The Top Of The Slope By Anchoring The Blanket In A 6 Inch Deep By 6 Inch Wide Trench With Approximately 12 Inches Of Blanket Extended Beyond The Upslope Portion Of The Trench. Anchor The Blanket With A Row Of Staples/Stakes Approximately 12 Inches Apart In The Bottom Of The Trench. Backfill And Compact The Trench After Stapling. Apply Seed To Compacted Soil And Fold Remaining 12 Inch Portion Of Blanket Back Over Seed And Compacted Soil. Secure Blanket Over Compacted Soil With A Row Of Staples/Stakes Spaced Approximately 12 Inches Apart Across The Width Of The Blanket.
- ③ Roll The Blankets (A.) Down Or (B.) Horizontally Across The Slope. Blankets Will Unroll With Appropriate Side Against The Soil Surface. All Blankets Must Be Securely Fastened To Soil Surface By Placing Staples/Stakes In Appropriate Locations As Shown In The Staple Pattern Guide. When Using Optional Dot System, Staples/Stakes Should Be Placed Through Each Of The Colored Dots Corresponding To The Appropriate Staple Pattern.
- ④ The Edges Of Parallel Blankets Must Be Stapled With Approximately 2-5 Inches Overlap Depending On Blanket Type. To Ensure Proper Seam Alignment, Place The Edge Of The Overlapping Blanket (Blanket Being Installed On Top) Even With The Colored Seam Stitch On The Previously Installed Blanket.
- ⑤ Consecutive Blankets Spliced Down The Slope Must Be Placed End Over End (Shingle Style) With An Approximate 3 Inch Overlap. Staple Through Overlapped Area, Approximately 12 Inches Apart Across Entire Blanket Width.

Overlap The Blankets With The Direction Of The Flow Of The Water

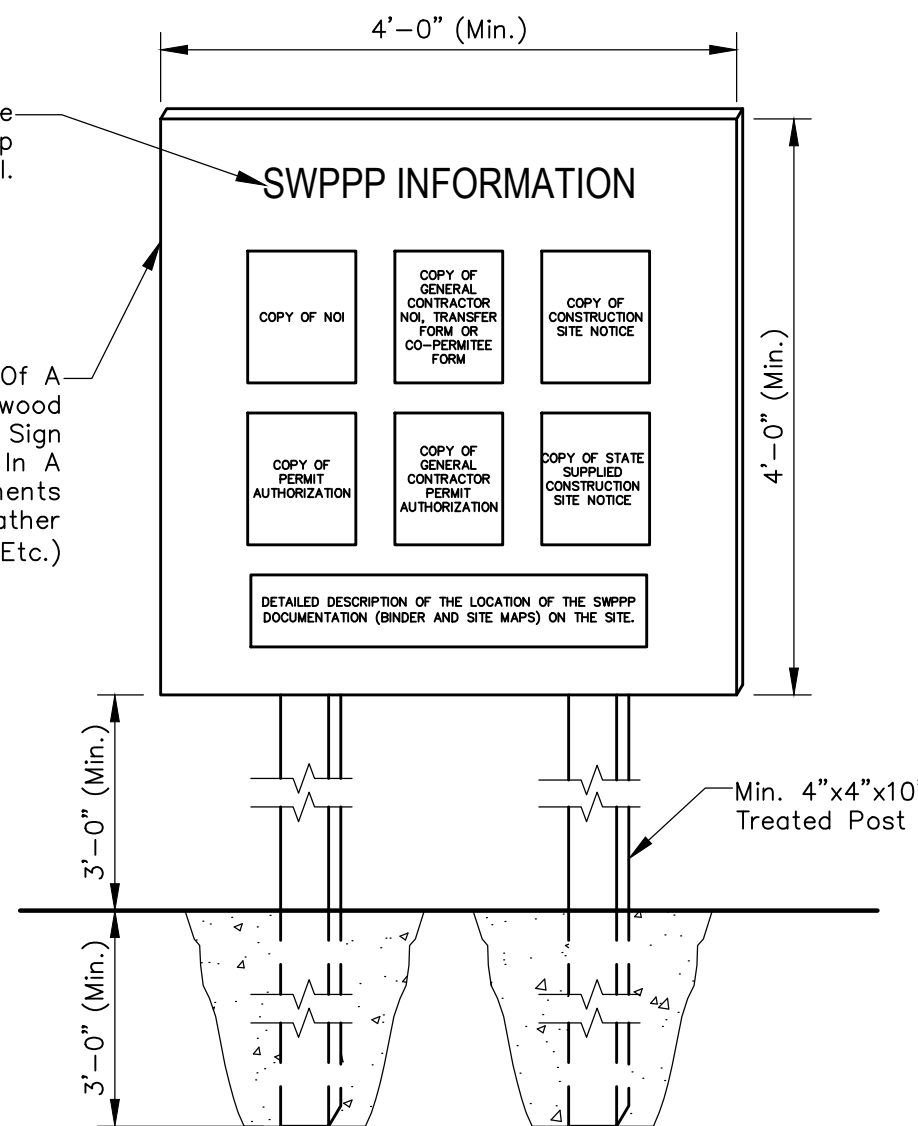
NOTE:  
\* In Loose Soil Conditions, The Use Of Staple Or Stake Lengths Greater Than 6 Inches May Be Necessary To Properly Secure The Blankets.

#### EROSION CONTROL BLANKET – SLOPE APPLICATION

Not To Scale

"SWPPP INFORMATION" Must Be Displayed Prominently Across The Top Of The Sign, As Shown In The Detail.

Sign To Be Constructed Of A Rigid Material, Such As Plywood Or Outdoor Sign Board. Sign Must Be Constructed In A Manner To Protect Documents From Damage Due To Weather (Wind, Sun, Moisture, Etc.)



#### SWPPP INFORMATION SIGN

Not To Scale

#### NOTES:

- 1.) The SWPPP Information Sign Must Be Located Near The Construction Entrance Of This Site, Such That It Is Accessible And Viewable By The General Public, But Not Obstructing Views As To Cause A Safety Hazard.
- 2.) All Posted Documents Must Be Maintained In A Clearly Readable Condition At All Times Throughout Construction And Until The Notice-Of-Termination (NOT) Is Filed For The Permit.
- 3.) Contractor Shall Post Other Storm Water And/Or Erosion And Sediment Control Related Permits On The Sign As Required.
- 4.) Sign Shall Be Located Outside Of Public Right-Of-Way And Easements Unless Approved By The Plainfield MS4 Operator.

#### SEEDING:

The Following Table Is For General Seeding Information Only. Consult The [Indiana Storm Water Quality Manual](#) For Recommendations Relating To Steep Banks And Cuts, High Maintenance Areas, And Channels And Areas Of Concentrated Flow.

#### SEEDS:

40 Percent Kentucky Bluegrass  
40 Percent Creeping Red Fescue  
20 Percent Annual Rye Grass

#### FERTILIZER:

Commercial Fertilizer (12-12-12)

#### STRAW:

Clean And Free Of Weed Seeds

Spread Fertilizer Uniformly Over Finish Graded Surfaces At A Rate Of 20 Pounds Per 1,000 Square Feet. Thoroughly Disk, Harrow, Or Rake Fertilizer Into Soil To Depth Not Less Than 2 Inches.

Distribute Seed Mix Same Day As Fertilizer Is Applied. Spread Evenly At A Rate Of 3 Pounds Per 1,000 Square Feet. Rake Lightly And Compact Areas With 100 Pound Roller.

Cover Areas With Straw Evenly Spread At A Rate Of 2 Tons Per Acre Immediately After Seeding. Water Areas With Fine Spray. Do Not Flood Or Create Washes. Protect Seeded Areas From Erosion.

Continue Watering Of These Areas On A Daily Basis For The Remainder Of The Construction Period.

Hold Sloped Areas Steeper Than 2 (Horizontal) To 1 (Vertical) With Wire Mesh Or Stakes And Wire.

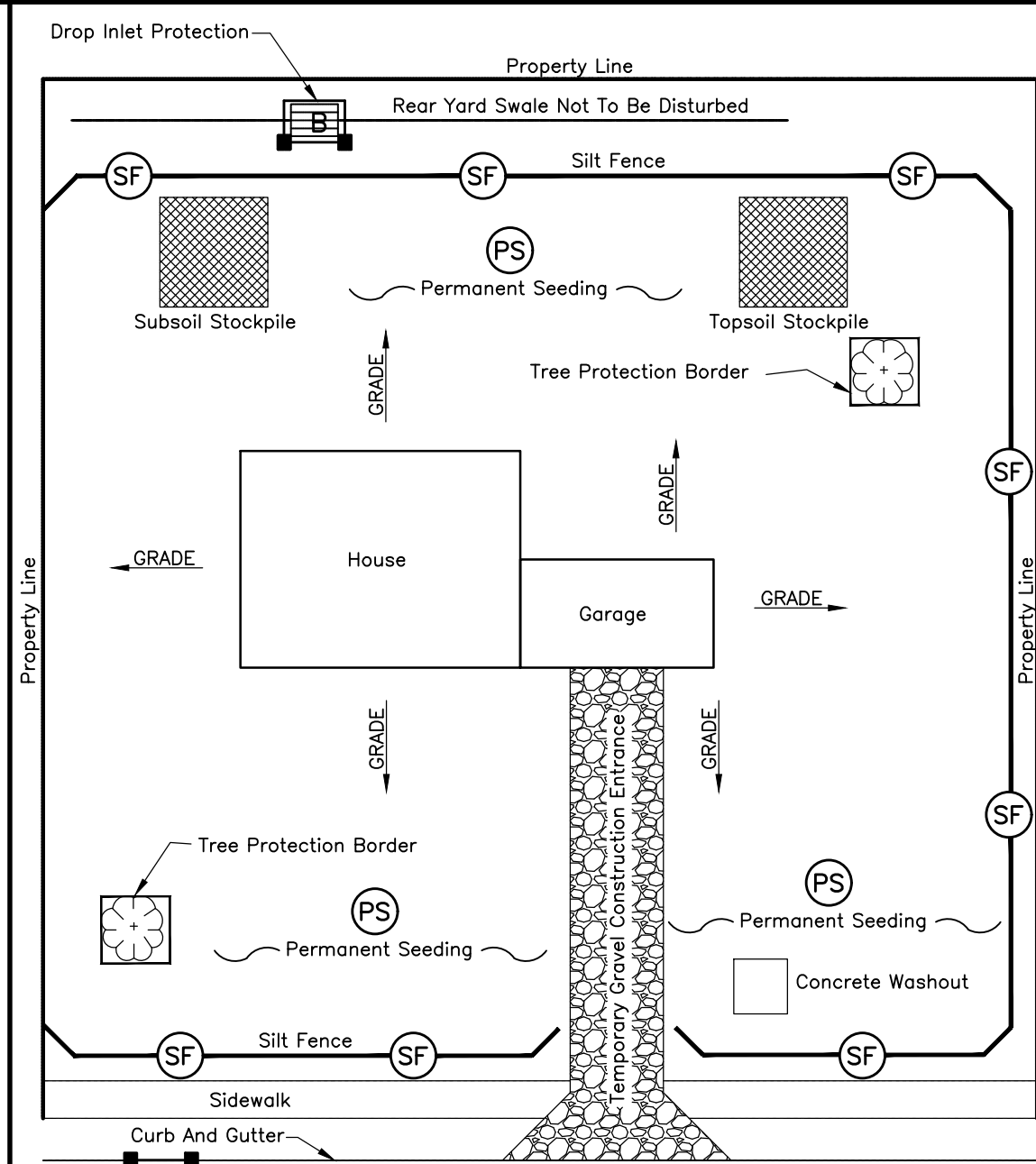
	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC
Wheat Or Rye												
Oats												
Annual Rye Grass												
Non-Irrigated*												
Irrigated												
Dormant Seeding**												

- Legend:
- Irrigation Required
  - \* Seeding Dates May Be Extended 5 Days If Mulch Applied And Planted Late Summer
  - \*\* Increase Seeding Rate By 50%

#### NOTES:

If Construction Activities Take Place During The Months Of November Through February, Use Dormant Seeding Practices In Place Of Temporary And Permanent Seeding Practices.

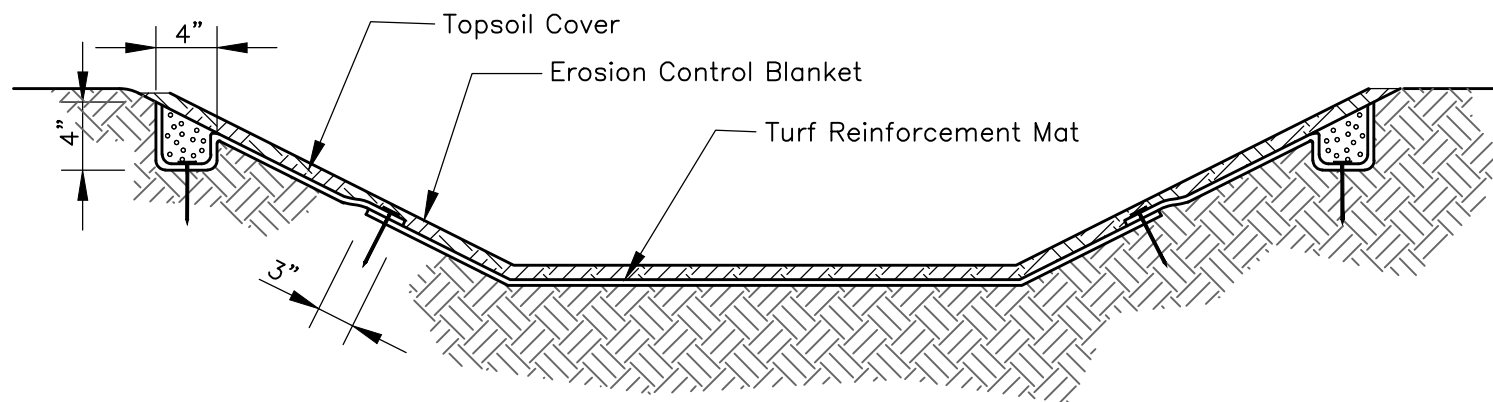
See Chapter 7 Of The [Indiana Storm Water Quality Manual](#). For Additional Seeding Recommendations.



#### SAMPLE EROSION CONTROL SITE PLAN

Not To Scale  
(For Construction Of Typical Single Family Dwellings.)

#### DEVELOPMENT STANDARD – DETAIL DS-E09



Extend Turf Reinforcement Mat To Accommodate Maximum Designed Flow Depth

#### NOTES:

##### Installation:

Select The Type Of Mat Recommended For The Site Conditions (Slope, Channel, Flow Velocity) And Problem To Be Addressed.

Install Any Practices Needed To Control Erosion And Runoff, Such As Temporary Or Permanent Diversions, Slope Drains, Sediment Basins/Traps, Silt Fence Or Straw Bale Dams.

Grade The Site As Specified.

Install The Mat According To Manufacturer's Specifications.

Backfill Topsoil To A Depth Equal To The Thickness Of The Mat.

Seed The Area After The Mat Has Been Installed And Backfilled With Soil.

Mulch The Area, Or Use Erosion Control Blankets To Stabilize The Surface.

##### Maintenance:

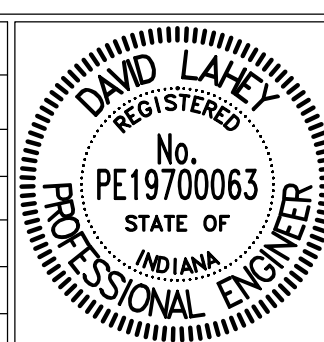
Until The Surface Is Stabilized, Inspect Weekly And After Each Storm Event For Erosion Exposing The Mat.

If A Specific Area Shows Erosion, Add Soil And Restabilize.

#### TURF REINFORCEMENT MAT

Not To Scale

REVISIONS		
Rev. No.	Description	Date



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APPROVED	<i>Shannon Egan</i>	01/01/2015
	EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES	DATE
APPROVED	<i>Shannon Egan</i>	01/01/2015
	MS4 OPERATOR	DATE

#### TOWN OF PLAINFIELD

#### EROSION CONTROL MEASURES

#### SHEET

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OF

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EROSION CONTROL NOTES

GENERAL:

Take Measures To Control Erosion And Sedimentation By Storm/Wind Events To Assure That Sediment Is Not Transported From The Site By Storm Events. Practices Such As Silt Traps Or Filters Shall Be Installed Prior To Land Disturbing Activities. New Drainage Swales Shall Be Seeded And/Or Sodded, Or Other Protective Practices Applied, Immediately Following Construction. All Practices Shall Be Maintained To Remove Sediment From Runoff Leaving The Site As Long As Unstabilized Soil Conditions Exist.

After Land Disturbing Activities Cease And The Soil Is Stabilized, Temporary Erosion Control Measures May Be Eliminated If Their Purpose Has Been Fulfilled. Any Disturbed Soil Resulting From Removal Of Such Practices Shall Be Stabilized By Approved Methods.

Dispose Properly All Waste And Unused Building Materials Including, But Not Limited To, Garbage, Debris, Cleaning Wastes, Water, Toxic Materials, And Hazardous Substances. Do Not Allow Substances To Be Carried By Runoff Into A Receiving Channel Or Storm Sewer System.

Clean Public Or Private Roadways Daily And After Major Storms Using Acceptable Methods Such as Sweeping To Remove Any Accumulated Sediment. The Developer's Contractors Are Responsible For Supervision Of The Construction Activity Within The Development And Shall Take All Necessary Actions To Remove Sediment From The Streets.

For Construction Sequence, Maintenance, And Other Soil Erosion Requirements, See Specifications For Site Clearing, Slope Protection, Erosion Control, Landscaping, And Seeding.

Erosion And Sediment Control Practices Must Adhere To, Or Exceed Those Shown On The Erosion Control Plan, And Shall Be In Accordance With The Construction Stormwater General Permit, And Indiana Storm Water Quality Manual, Indiana Department Of Environmental Management.

SURFACE STABILIZATION:

Cut Slopes Which Are To Be Topsoiled Should Be Scarified To A Minimum Depth Of 4 Inches Prior To Placement Of Topsoil. Install Erosion Control Blankets On All Slopes Of 3 (Horizontal) To 1 (Vertical).

Stabilize All Disturbed Ground Within Fifteen Days Of Being Left Inactive By Seeding, Sodding, Mulching, Or By Other Equivalent Erosion Control Practices. Immediate Stabilization Shall Be Planned To Aid In Surface Runoff And Stabilization Shall Follow A Linear Progression As The Site Is Developed.

Un-Vegetated Areas That Are Left Idle Or Scheduled To Be Left Inactive Must Be Temporarily Or Permanently Stabilized With Measures Appropriate For The Season To Minimize Erosion Potential. To Meet This Requirement, The Following Apply:

1. Stabilization Must Be Initiated By The End Of The Seventh Day The Area Is Left Idle. The Stabilization Activity Must Be Completed With Fourteen Days After Initiation. Initiation Of Stabilization Includes, But Is Not Limited To, The Seeding And/Or Planting Of The Exposed Area And Applying Mulch Or Other Temporary Surface Stabilization Methods Where Appropriate. Areas That Are Not Accessible Due To An Unexpected And Disruptive Event That Prevents Construction Activities Are Not Considered Idle.
2. Areas That Have Been Compacted May Be Excluded From The Stabilization Requirement When The Areas Are Intended To Be Impervious Surfaces Associated With The Final Land Use. Provided Run-off From The Area Is Directed To Appropriate Sediment Control Measures.

See The Landscape Plan For Permanent Ground Cover Requirements Adjacent To The Building And Parking Areas.

TEMPORARY GRAVEL CONSTRUCTION ENTRANCE/EXIT PAD:

Construct The Temporary Gravel Drive Using 6 Inches INDOT No. 2 Stone Over A Stable Foundation. Geotextile Fabric Shall Be Used Under All Drives Including Individual Lots. Grade For Positive Drainage.

Inspect The Entrance Pad Area Weekly And After Storm Events Or Heavy Use. Reshape The Pad As Needed For Drainage And Runoff Control. Top Dress Pad With Clean Stone.

SODDING:

Do Not Install Sod On Hot, Dry Soil, Frozen Soil, Compacted Clay, Loose Sand Or Gravel, Or Pesticide Treated Soil. Ideal Sodding Time Is May 1–June 1, Or September 1–October 20, Although It Can Be Installed As Early As March 15, If Available And Temperatures Are Above 32° F, Or June 1–September 1 If Irrigated.

Install Sod After Other Erosion Control Practices Have Been Completed. Break Up Compacted Soils Sufficiently To Create A Favorable Rooting Depth Of 6–8 Inches, Using A Chisel Plow, Disk, Harrow, Or Rake.

Soil Compaction Is To Be Minimized, Especially In Areas Where Permanent Vegetation Will Be Established. Topsoil Must Be Preserved, Unless Infeasible.

Apply Topsoil If The Site Is Otherwise Unsuitable For Establishing Vegetation. Shape, Smooth, And Firm The Soil Surface.

Have The Soil In The Sod Bed Tested To Determine Its pH And Nutrient Level. If The pH Is Too Acidic For The Grass Sod To Be Installed, Apply Lime According To Test Results Or At The Rate Recommended By The Sod Supplier.

Fertilize As Recommended By The Soil Test. If Testing Was Not Done, Consider Applying 400–600 Lbs./Acre Of 12–12–12 Analysis Fertilizer, Or Equivalent Fertilizer, As Recommended By The Soil Test. Work The Fertilizer Into The Soil To 2–4 Inches Deep.

Apply Fertilizer At An Appropriate Time Of Year For The Project Location, Taking Into Consideration Proximity To A Waterbody, And Preferably Timed To Coincide With The Period Of Maximum Vegetative Uptake And Growth.

Avoid Applying Fertilizer Immediately Prior To Precipitation Events That Are Anticipated To Result In Stormwater Run–Off From The Application Area.

TREE CONSERVATION/PROTECTION:

Protect Trees From Construction Equipment By Fencing Off An Area Equivalent To The Tree's Crown With Temporary Construction Safety Fence. If A Fence Cannot Be Erected, Cushion The Rooting Area With 6 Inches Of Wood Chips, Or Wood Or Brick Paths.

Create Traffic Patterns Such As To Keep Soil Compaction To A Minimum. Store Supplies And Equipment Away From Protected Tree Areas. Aerate Soil Where Compaction Has Been Excessive.

When Clearing Areas Adjacent To Protected Trees, Use Equipment Such As A Brush Cutter Or Rotary Ax, Or Cut By Hand. Where Root Areas Must Be Graded, Cut Large Roots Instead Of Tearing Them With Equipment.

Minimize Changes In The Drainage Pattern. Avoid Putting Fill Over The Root System.

Prune Low Hanging Limbs That Could Otherwise Be Broken Off By Equipment.

EROSION CONTROL NOTES CONT'D

EROSION CONTROL BLANKETS:

Erosion Control Blankets Shall Be Selected Based Upon Application And Shear Strength.

Use Machine Produced Mat Of Straw Fiber Matrix Or Curled Wood Excelsior Of 80 Percent, 6 Inch Or Longer Fiber Length.

Evenly Distribute Fibers Over Entire Area Of Blanket To Provide Consistent Thickness.

Provide Blanket With Top Side Covered With Biodegradable Extruded Plastic Mesh.

Treat Blankets To Impart Smolder Resistance Without Use Of Chemical Additives.

Provide "Curlex Blankets" By American Excelsior Company, Or "S150" By North American Green, Or Accepted Substitute.

EROSION CONTROL BLANKET STAPLES:

Use Minimum 0.091 Inch Diameter Steel Wire "U" Shape With Legs 6 Inches In Length With 1 Inch Crown.

CONCRETE AND CEMENTITIOUS WASHWATER:

Cementitious Washwater Results From The Cleaning Of Tools And Equipment Used In The Delivery, Mixing, Handling, And Working Of Cementitious Materials Often Associated With Concrete, Mortar, Plaster, Stucco, Grout And Flowable Fill.

Concrete Washouts Shall Be Of Sufficient Volume And Quantity To Contain All Liquid And Concrete Waste Generated By Washout Operations. The System Shall Be Designed To Eliminate Run–off And Minimize Precipitation From Entering The Washwater Containment System. Covering Of Containment When Not In Use Is Recommended.

Locate Washwater Containments At Least 50 Feet From Any Creeks, Wetlands, Ditches, Karst Features, Or Storm Drains/manmade Conveyance Systems. Locate When Practical In Relatively Flat Areas With Established Vegetative Cover In Areas That Provide Easy Access For Equipment That Will Require The Use Of Washwater Containment Facilities.

Prefabricated Washout Containers Or Roll–off Dumpsters Are Preferred. Structure Must Be Watertight And Have The Strength To Resist Failure Or Collapse For The Duration Of Use. Below Grade Systems Are To Be Used Only When There Is No Other Feasible Way To Implement Containment. Waterproof Lining Is Required To Have A Minimum Thickness Of 10 Mil. Be A Single Continuous Sheet Sufficient To Adequately Line The Entire Containment And Be Free Of Defects, Holes, Rips, Or Tears. Signage Is Required To Identify Washout Areas.

Washouts Shall Not Be Used For Trash Or Construction Debris. Containers Should Not Be Filled Beyond 75 Percent Of Containment Capacity. When A Containment Is At Capacity And Can No Longer Accept Washwater, Identify With "closed" Sign. No Spillage Of Washwater Shall Occur From The Transport Of The Unit. Closure Of Washwater Shall Be Accomplished When All Fluids Are Removed Or Evaporated. The Remaining Solid Cementitious Material May Be Used As Clean Fill.

FLOATING OUTLET "SKIMMER":

Sediment Basins Where Feasible, Must Withdraw Water From The Surface Of The Water Column Unless Equivalent Sediment Reduction Can Be Achieved By Use Of Alternative Measures. Alternative Measures Include But Are Not Limited To Increasing The Basin Length To Width Ratio To 4:1 Or Greater, Implementation Of Porous Baffles, Use Of Flocculants/polymers, And Or Phasing Of Project Land Disturbance And Rapid Stabilization.

Floating Outlets Can Be Implemented With The Permanent Basin Outlet Structure. The Discharge Capacity:

Dewatering Zone Volume/Dewatering Time = Required Flow Rate Of Skimmer. Locate Floating Devices Where They Can Be Easily Accessed To Facilitate Maintenance Activities And To Be Appropriately Tethered Or Restrained To Prevent Flexible Boom Damage. If Ice Formation Is A Concern, Install Boom And Inlet At An Incline To Maintain Positive Drainage Through The Device. Install Following The Manufacturer's Recommendation.

The Floating Inlet Is Designed To Drain The Dewatering Zone In No Less Than 48 Hours And No Longer Than 72 Hours For The Minimum Required Storage Volume. Inspect Weekly And Prior To Anticipated Rain Events. The Floating Outlet Practice Shall Only Be Removed When The Contributing Drainage Area Has Been Properly Stabilized And No Longer Contributing Sediment–laden Run–off Or When Freezing Conditions Are Anticipated.

NATURAL BUFFERS:

Preserve Existing Natural Buffers That Are Adjacent To Waters Of The State To Promote Infiltration And Provide Protection Of The Water Resource. Natural Buffers Must Be Preserved, Including The Entire Buffer Bordering And/or Surrounding The Water Resource.

Buffers:

1. 50 Feet Or More In Width Must Be Preserved To A Minimum Of 50 Feet
2. Less Than 50 Feet In Width Must Be Preserved In Their Entirety.
3. May Be Enhanced With Vegetation That Is Native And Promotes Ecological Improvements And Sustainability.

Run–off Directed To The Natural Buffer Must Be Treated With Appropriate Erosion And Sediment Control Measures Prior To Discharging To The Buffer And Managed To Prevent Erosion From Occurring Within The Buffer Area.

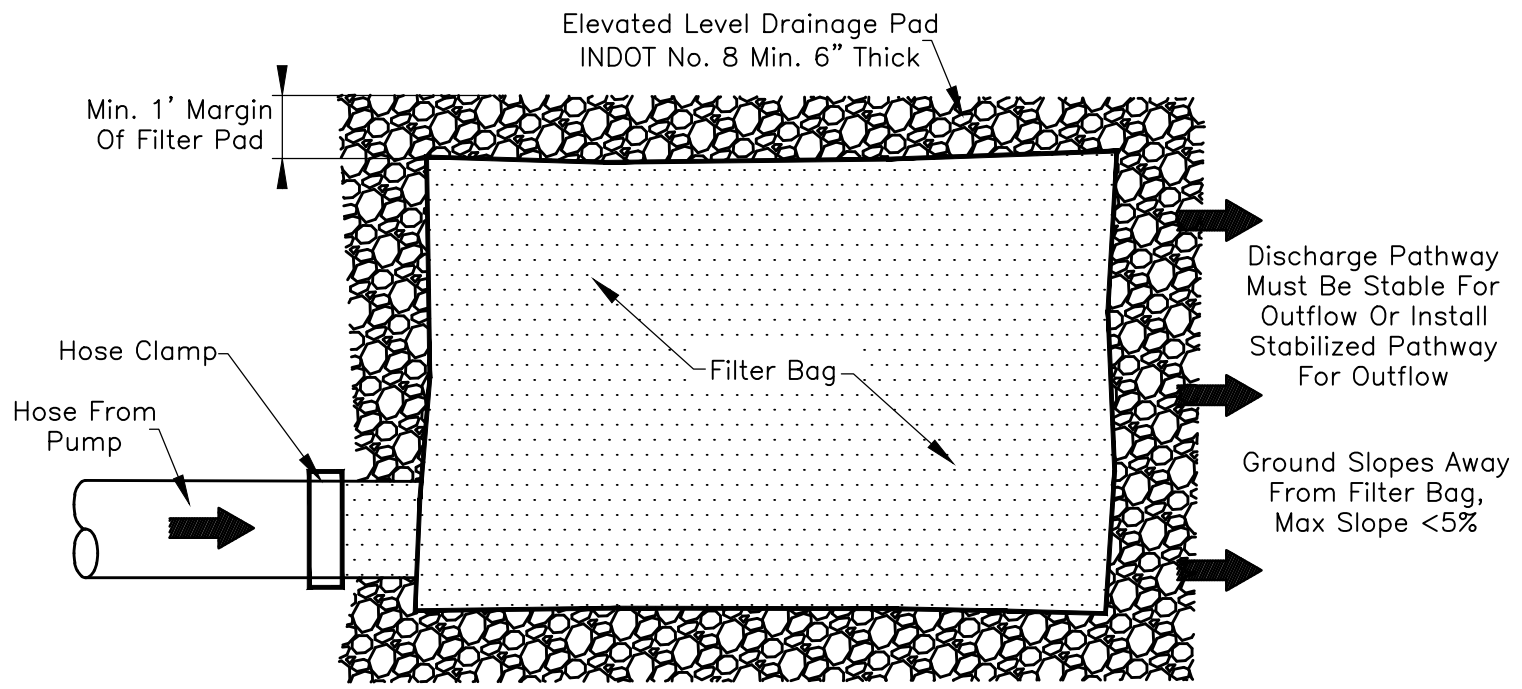
Stormwater Conveyances And Outfalls Are Allowed To Impact The Buffer And Must Be Designed To Minimize The Width Of Disturbance And Impact To The Buffer.

WASTE CONTAINERS (TRASH RECEPTACLES):

Must Be Managed To Reduce The Discharge Of Pollutants And Blowing Of Debris. If Stormwater Has The Potential To Come Into Contact With Waste, A Cover Is Required. Waste That Is Not Disposed Of In A Trash Receptacle Must Be Protected From Exposure To The Weather And/or Removed At The End Of The Day From The Site And Disposed Of Properly.

ANIONIC POLYMERS (FLOCCULANTS):

Are Authorized For Sediment Control Provided Their Use Is In Conformance With Current State Of Indiana Standards And Specifications, And The Use Is Identified In The Stormwater Pollution Prevention Plan (SWPP3). The Manufacture Representative Or Properly Trained Individual Is Required To Oversee The Use Of All Polymers. Prior To The Use Of The Polymer, An Email Notification Must Be Made To The Town Of Plainfield.



NOTES:

Dewatering Bags Are Used To Minimize The Discharge Of Sediment For Pump Induced Dewatering Activities.

Bag Size Is Dependent On The Pumping Rate And Soil Conditions.

Clamp Pump Hose With Steel Hose Clamp Over The Rigid Hose Connector Area To A Tight Secure Connection To Filter Bag.


Locate Filter Bags Where Outflows Can Easily Drain. Preferred Locations Are Areas Of Undisturbed Densely Vegetated Areas. Locate For Ease Of Access, Monitoring, Maintenance, And Removal.

MATERIALS:

Nonwoven Polyethylene Geotextile Or Geotextile Bag. Steel Hose Clamps Or Equivalent To Tightly Attach Pump Hose To The Filter Bag. Elevated Drainage Pad (No. 8 Stone, Wood Mulch, Straw Bales, Wood Pallet). Secondary Containment Berm (Optional)

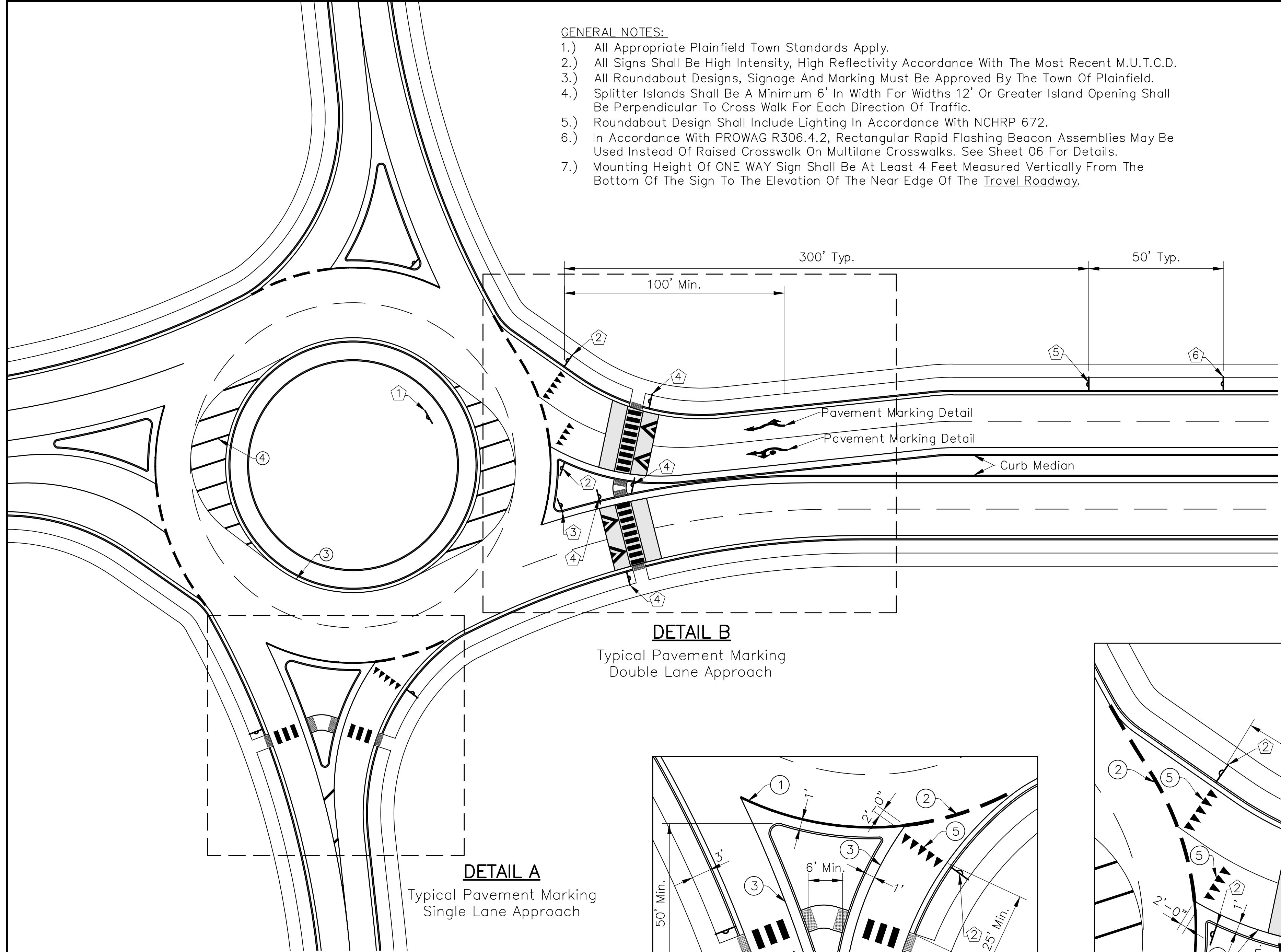
FILTER BAGS (PUMP DISCHARGE FILTER BAGS)

Not To Scale

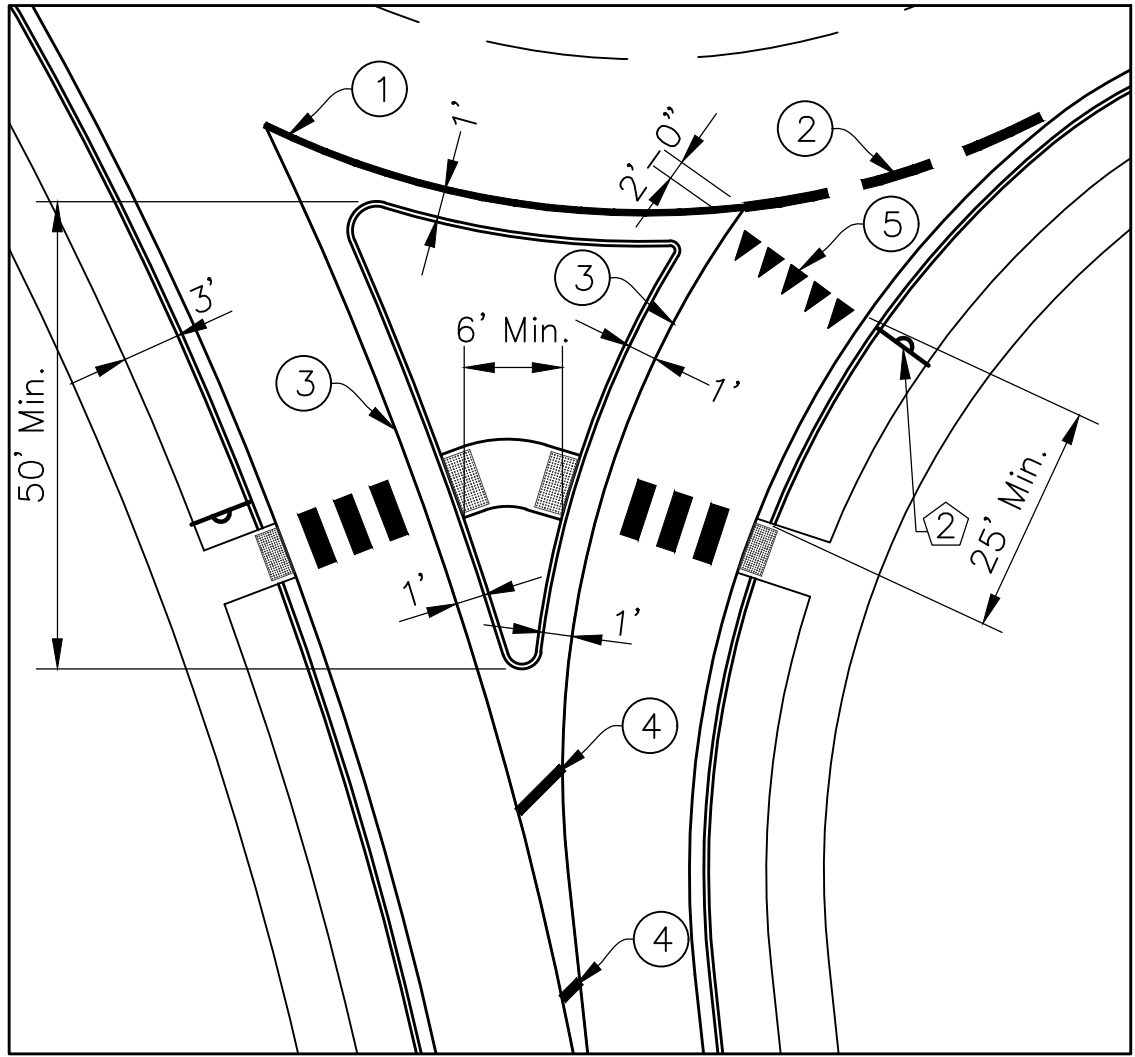
REVISIONS				RECOMMENDED FOR APPROVAL  DESIGN ENGINEER  APPROVED  EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES  APPROVED  MS4 OPERATOR	DATE  01/01/2015  DATE  01/01/2015  DATE  01/01/2015	TOWN OF PLAINFIELD  EROSION CONTROL MEASURES	SHEET 23 OF 29
Rev. No.	Description	Date					



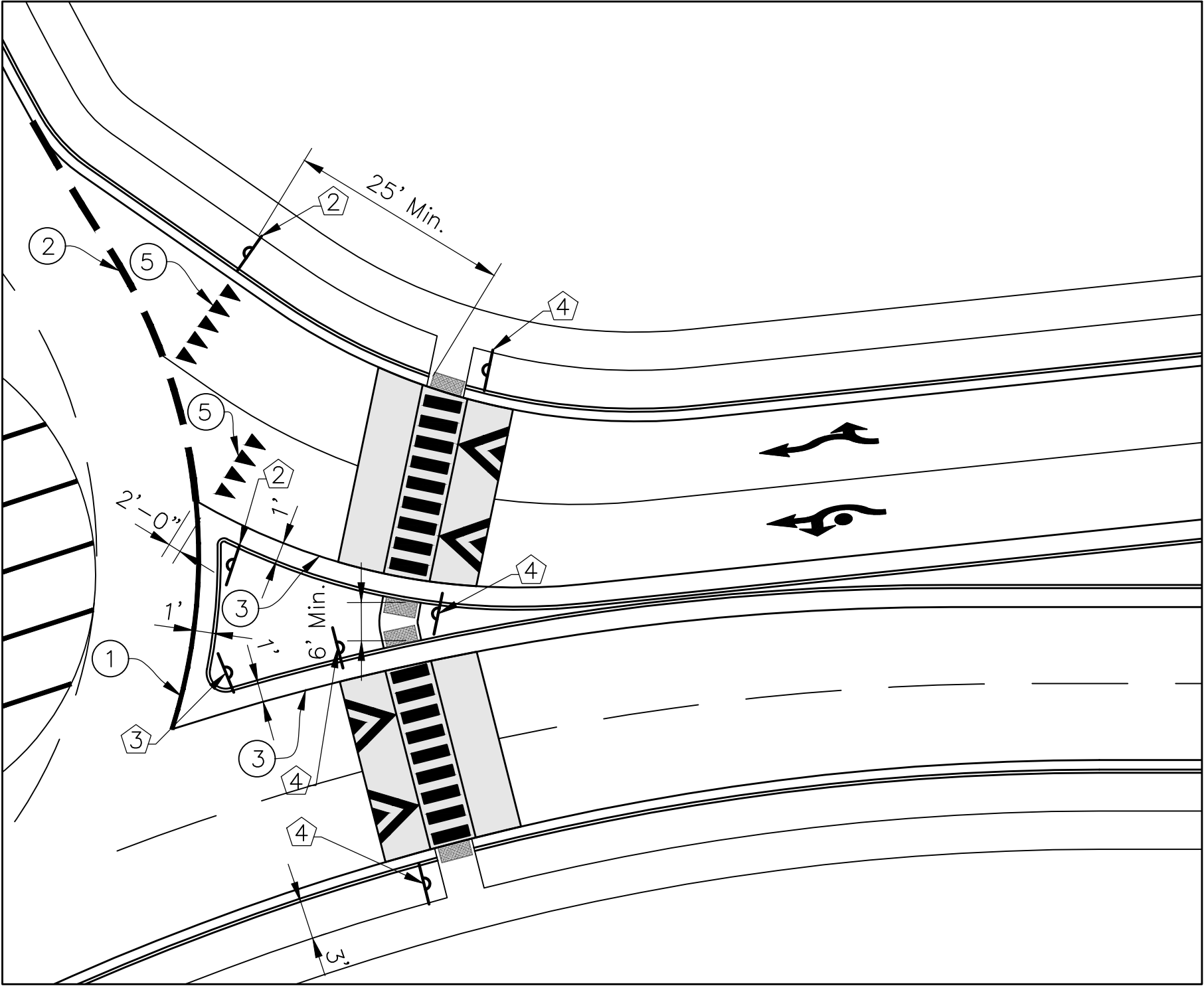
- GENERAL NOTES:
- 1.) All Appropriate Plainfield Town Standards Apply.
  - 2.) All Signs Shall Be High Intensity, High Reflectivity Accordance With The Most Recent M.U.T.C.D.
  - 3.) All Roundabout Designs, Signage And Marking Must Be Approved By The Town Of Plainfield.
  - 4.) Splitter Islands Shall Be A Minimum 6' In Width For Widths 12' Or Greater Island Opening Shall Be Perpendicular To Cross Walk For Each Direction Of Traffic.
  - 5.) Roundabout Design Shall Include Lighting In Accordance With NCHRP 672.
  - 6.) In Accordance With PROWAG R306.4.2, Rectangular Rapid Flashing Beacon Assemblies May Be Used Instead Of Raised Crosswalk On Multilane Crosswalks. See Sheet 06 For Details.
  - 7.) Mounting Height Of ONE WAY Sign Shall Be At Least 4 Feet Measured Vertically From The Bottom Of The Sign To The Elevation Of The Near Edge Of The Travel Roadway.



DETAIL B  
Typical Pavement Marking  
Double Lane Approach



DETAIL A  
Scale: 1"=20'  
Typical Pavement Marking  
Single Lane Approach

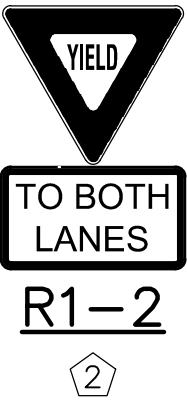


DETAIL B  
Scale: 1"=20'  
Typical Pavement Marking  
Double Lane Approach

LOW SPEED URBAN / SUBURBAN ROUNDABOUT DETAIL  
Scale: 1"=30'

LEGEND

- ① Line, Solid White, 8"
- ② Line, Broken White, 8"
- ③ Line, Solid Yellow, 4"
- ④ Crosshatch 45°, Solid Yellow, 12" (20' Spacing)
- ⑤ Shark Tooth Yield Triangle 2'W x3'H
- Sign



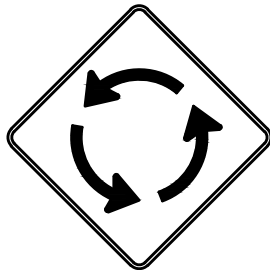
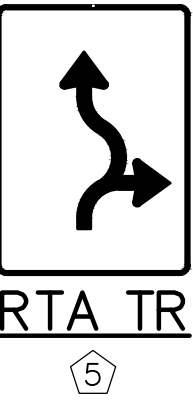
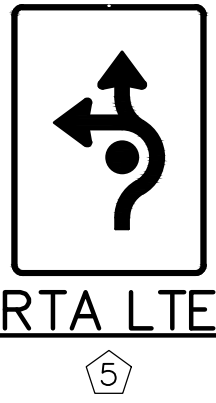
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SIGN ASSEMBLY

\* Fluorescent Yellow Green



W2-6

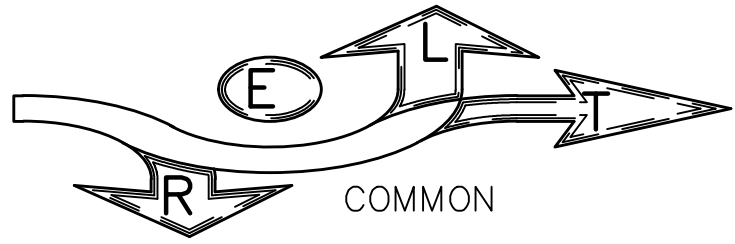
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W13-1

SIGN ASSEMBLY

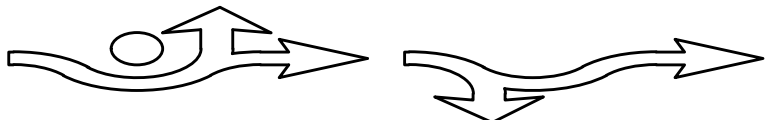
SIGN DETAILS  
See Post Detail On Sheet 06



COMPONENT KEY

The Labeled Areas Above Correspond To The Portions Needed For Each Type Of Roundabout Traffic Arrow.

For Example: The Roundabout Traffic Arrow Type Tre Requires The "Common", "T", "R", And "E" Areas.



TYPE LTE TYPE TR

MARKING DETAILS

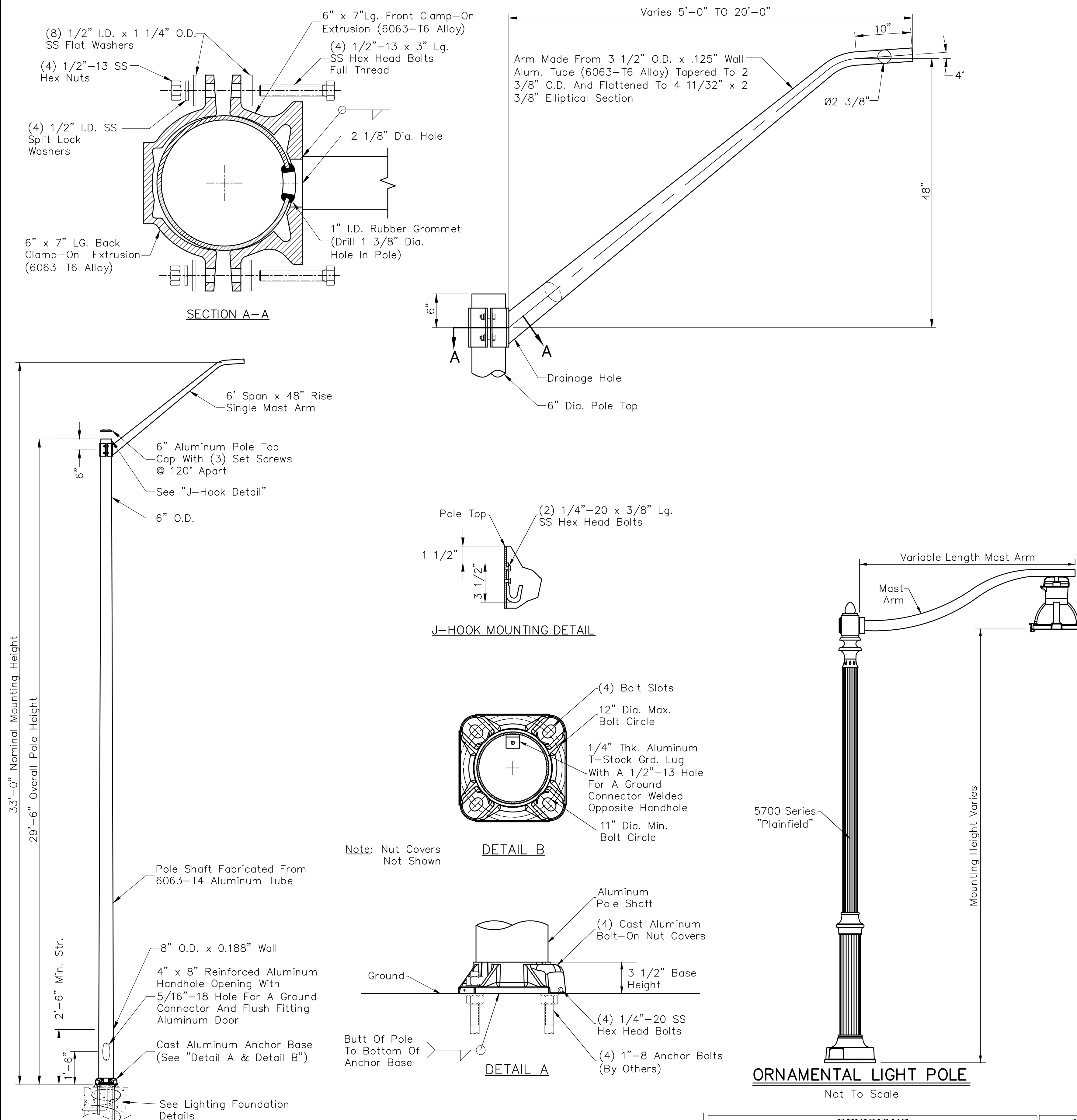
REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Lacey</i>	01/01/2015
DESIGN ENGINEER		DATE
APPROVED	<i>David Lacey</i>	01/01/2015
EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES		DATE
APPROVED	<i>Scott J. Jorgensen</i>	01/01/2015
DIRECTOR OF TRANSPORTATION		DATE

TOWN OF PLAINFIELD	SHEET
LOW SPEED URBAN / SUBURBAN ROUNDABOUT DETAIL	24 OF 29





- ### Lighting Component Notes:
1. Ornamental Light Poles Shall Be Manufactured By Valmont Electrical MFG. Co.
  2. Standard Light Poles Shall Meet INDOT Design Standards
  3. Pole Shafts Shall Be 16-S Sharp Flute Tapered. Base Diameter, Top Diameter, Shaft Length, And Gauge Thickness, 19 Ft. Maximum Vertical Clearance From Top Of Pavement To Bottom Of All Signal Heads.
  4. Base Coat Shall Be Hot Dipped Galvanized To ASTM:A123.
  5. Finish Shall Be TGIC Or Urethane Polyester Powder.
  6. Ornamental Light Pole Color Shall Be Woodland Green (Color Code: RAL 6028), Or Black Subject To Town Approval.
  7. Structural Design Is To Be Completed By The Contractor Under The Direct Supervision Of An Experienced Professional Engineer Registered In The State Of Indiana. The Successful Bidder Is To Provide Shop Drawings, Which Bear, For All Structural Components The Professional Seal And Signature Of The Engineer Responsible For The Structural Design.
  8. Prior To Fabrication, Shop Drawings For All Lighting Components Shall Be Submitted To The Engineer For Approval.
  9. Evolve Roadway Scalable Fixture Shall Be The Following:ERS1 Or ERS2 And 4000K Color Temperature. Luminaire Head Shall Be Finished To Match Pole.
  10. Technical Specification, Photometric Plan, And Visual File Shall Be Submitted To The Town Of Plainfield For Review Prior To Installation.

- Helical Foundation Notes:**
1. Finish: Hot Dip Galvanize Per ASTM—A153 (Latest Revision).
  2. Baseplate To Be Perpendicular To Shaft Axis ( $\pm 1^\circ$ ) And Hole Centerline Concentric ( $\pm .188$ ) To Shaft Axis.
  3. All Material Is To Be New, Unused And Mill Traceable Meeting The Following Specification: Baseplate: ASTM A36—(Latest Revision) Structural Steel (Conform To AASHTO Tech. Bul. #270).

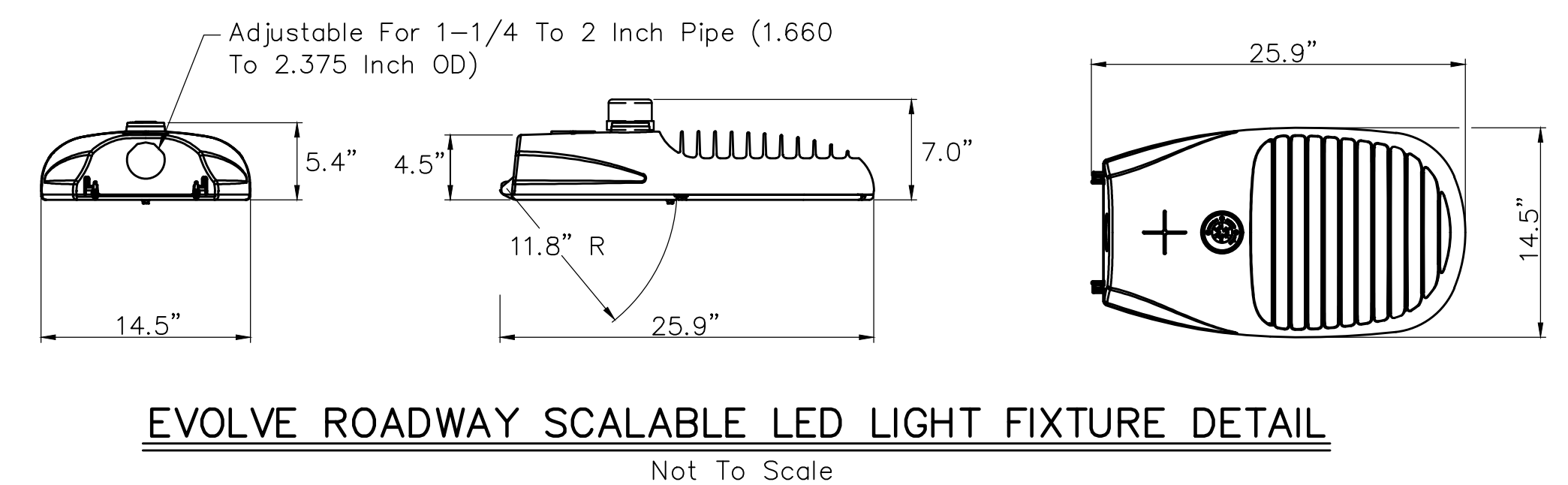
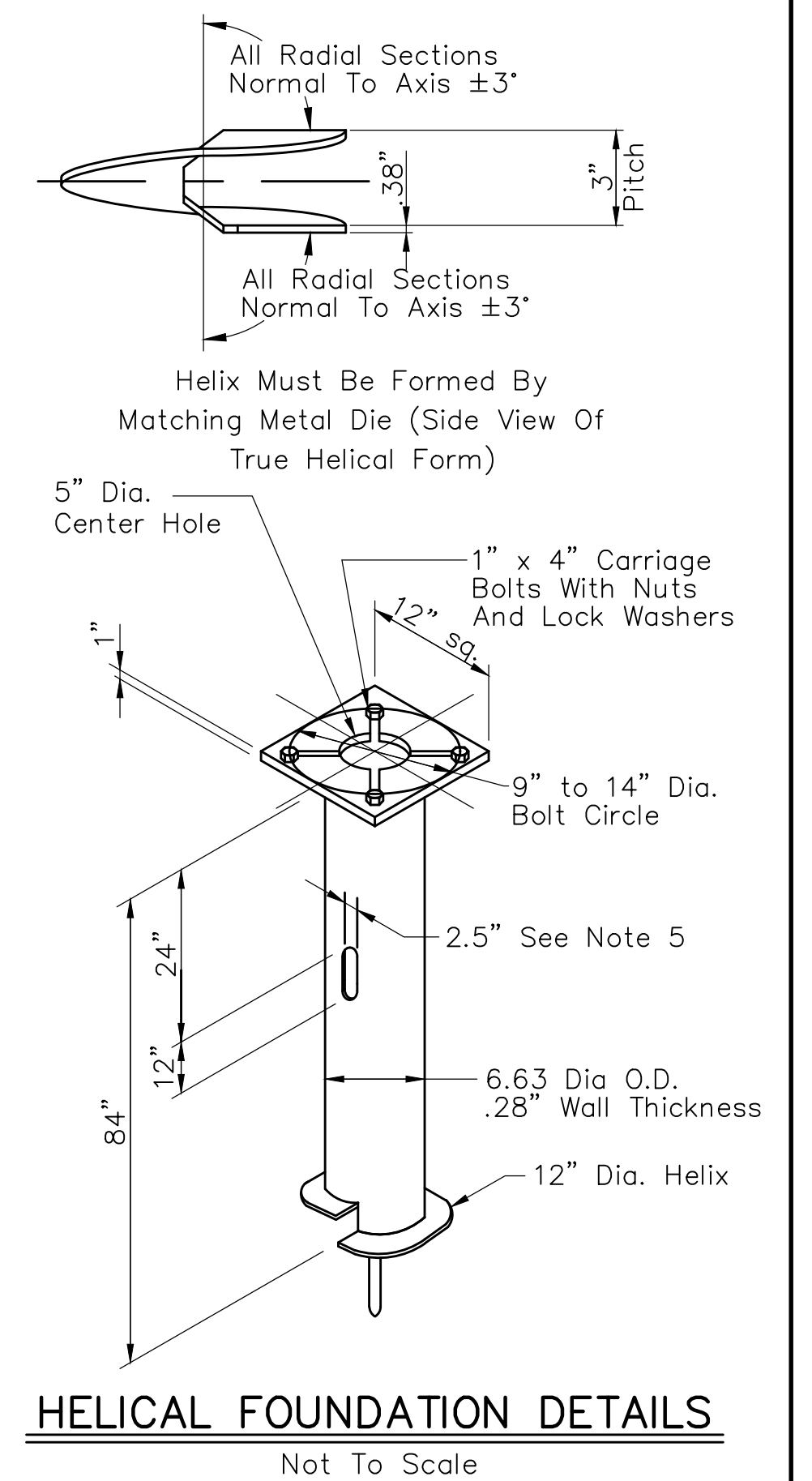
Shaft: ASTM A252—(Latest Revision) Grade 2, Steel Pipe Piles. Alternate Material: ASTM A53—(Latest Revision) Type E Or S, Grade B, Steel Pipe Or ASTM A500—(Latest Revision) Grade B, Structural Steel Tubing.

Helix: ASTM A635-(Latest Revision) 3/8" Thick Hot Rolled Steel Plate Or Coil.

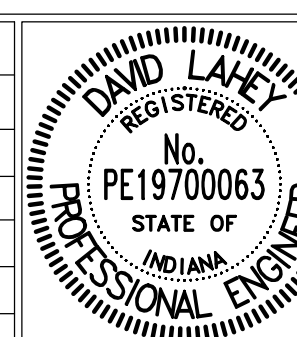
Pilot Point: ASTM A575—(Latest Revision) 1-1/4" Diameter Hot Rolled Steel Bar. Bolts: Carriage Bolts Per ANSI B-18.5, SAE J429 Grade-5.

Nuts: Heavy Hex Nuts Per ASTM A194 Grade 2H Or ASTM A563 Grade DH,  
Meeting The Supplementary Requirements Of ASTM A563; 1-8UNC-2B Per  
ANSI B18.2.2.

- Lighting Service Point Notes:
1. Lighting Service Point Enclosure Shall Be Manufacturer By APX Enclosures Inc., Or Plainfield DPW Approved Equal.
  2. Service Point Enclosure Shall Be A Single Door Configuration, NEMA 3R Rated, And Powder Coated "Black".



REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>Daniel Zukay</i> DESIGN ENGINEER	01/01/2012
APPROVED	<i>Daniel Zukay</i> EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES	01/01/2012
APPROVED	<i>Scott Livingston</i> DIRECTOR OF TRANSPORTATION	01/01/2012

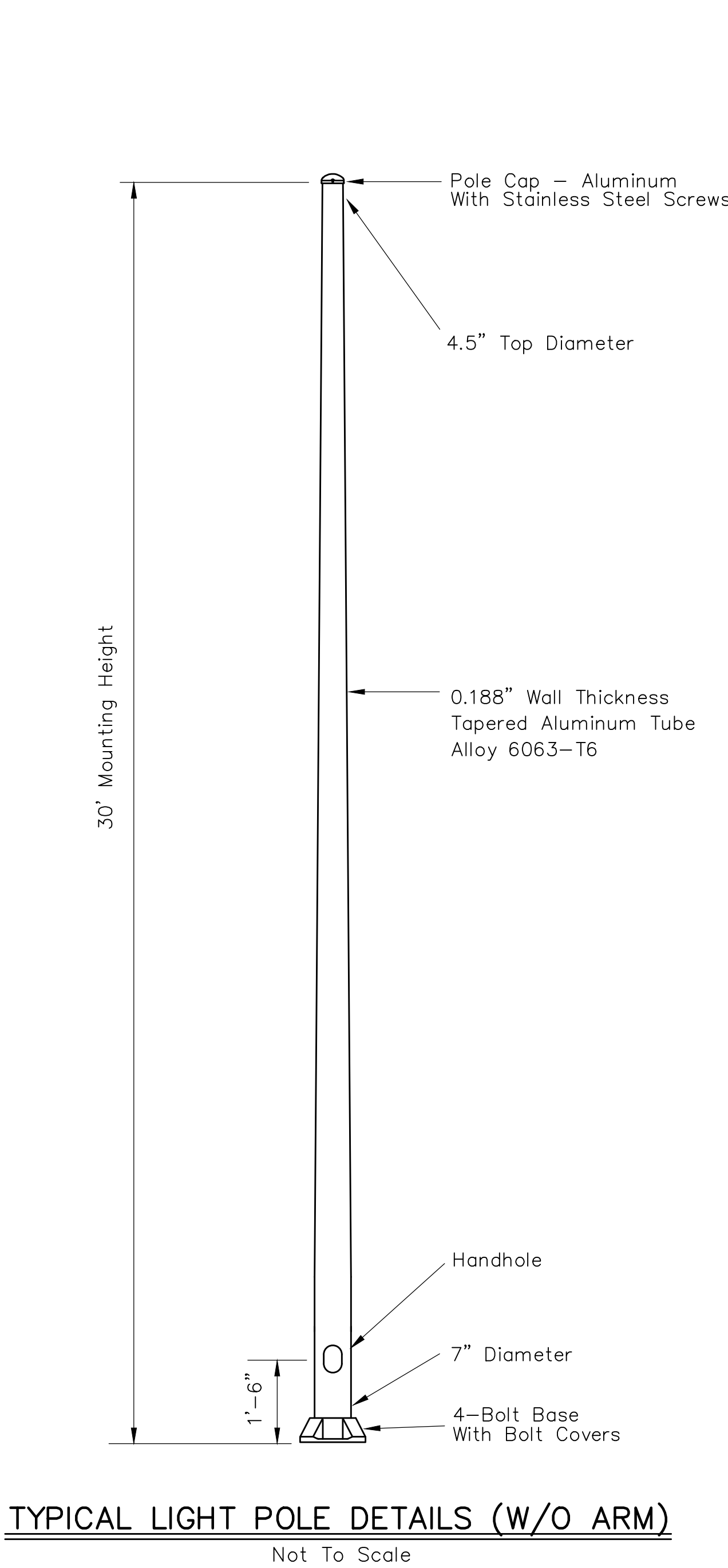
# TOWN OF PLAINFIELD

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## STREET LIGHTING DETAILS APPLICABLE TO COLLECTORS & ARTERIALS

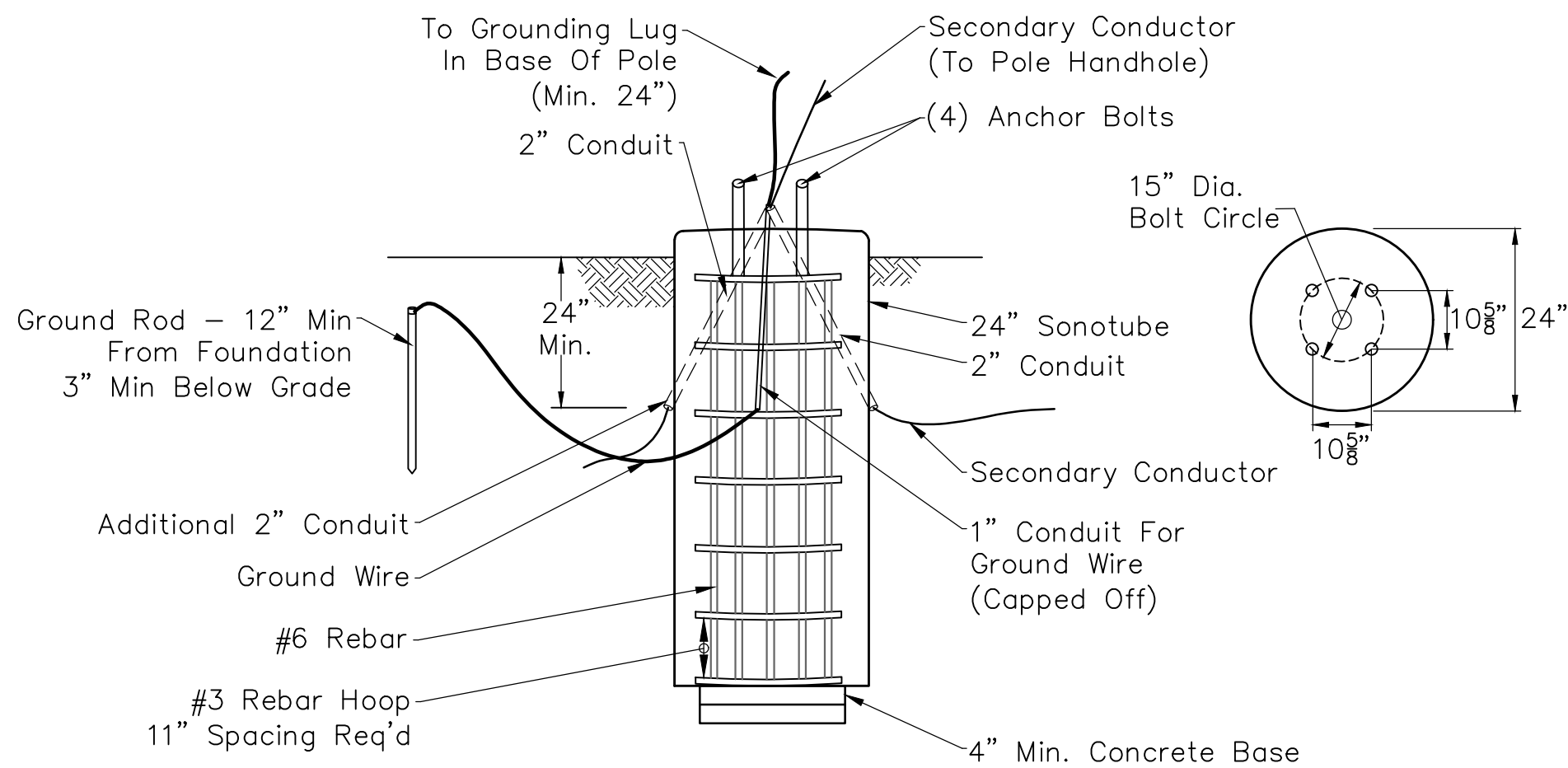
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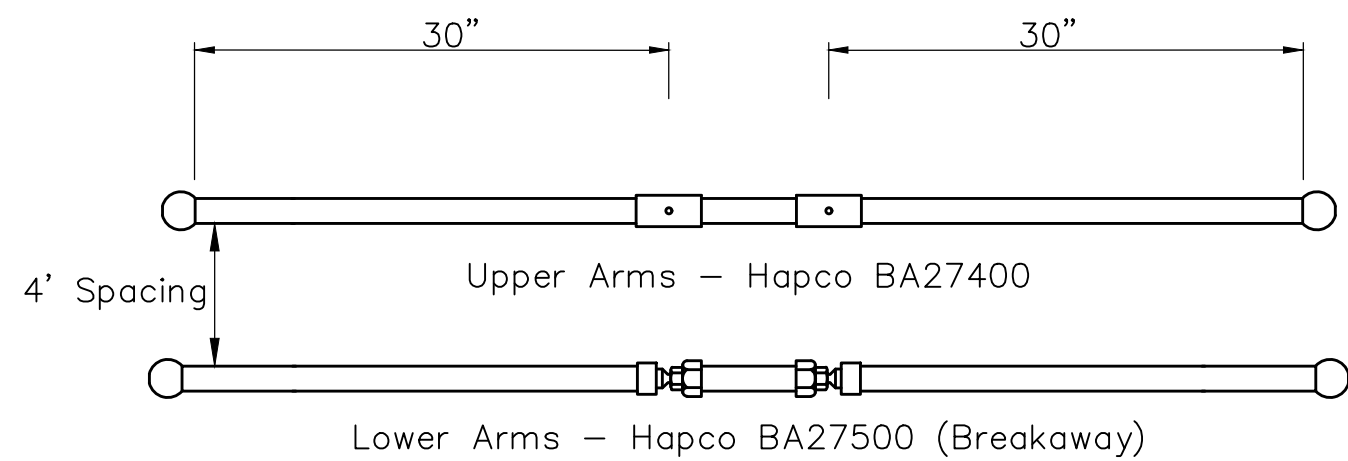
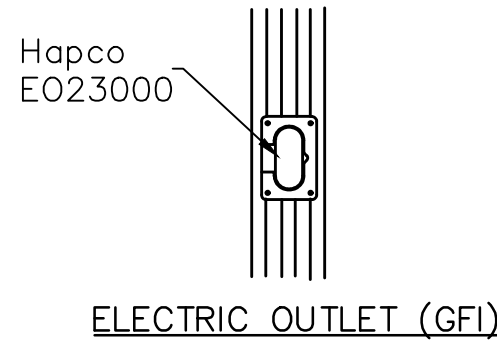


- Lighting Component Notes:**
- Standard Light Poles Shall Meet INDOT Design Standards
  - The Pole Shaft Will Be Constructed Of Seamless Extruded Tube Of 6063 Aluminum Alloy Per The Requirements Of ASTM B221. The Shaft Assembly Ahall Be Full–Length Heat Treated After Base Weld To Produce A T6 Temper.
  - Base Style Shall Be 4–Bolt Cast Aluminum Base Flange Of Alloy 356–T6 With Aluminum Bolt Covers (Alloy 356–F) And Stainless Steel Hex Head Attaching Screws.
  - Steel Anchor Bolts Shall Conform To AASHTO M314–90 Grade 55. Ten Inches (10”) Of Threaded End Will Be Galvanized Per ASTM A153.
  - Lighting Fixture Shall Be KIM Lighting Alt120 270 Watt Altitude LED And 4000K Color Temperature.
  - Structural Design Is To Be Completed By The Contractor Under The Direct Supervision Of An Experienced Professional Engineer Registered In The State Of Indiana. The Successful Bidder Is To Provide Shop Drawings, Which Bear, For All Structural Components The Professional Seal And Signature Of The Engineer Responsible For The Structural Design.
  - Prior To Fabrication, Shop Drawings For All Lighting Components Shall Be Submitted To The Engineer For Approval.
  - Technical Specification, Photometric Plan, And Visual File Shall Be Submitted To The Town Of Plainfield For Review Prior To Installation.

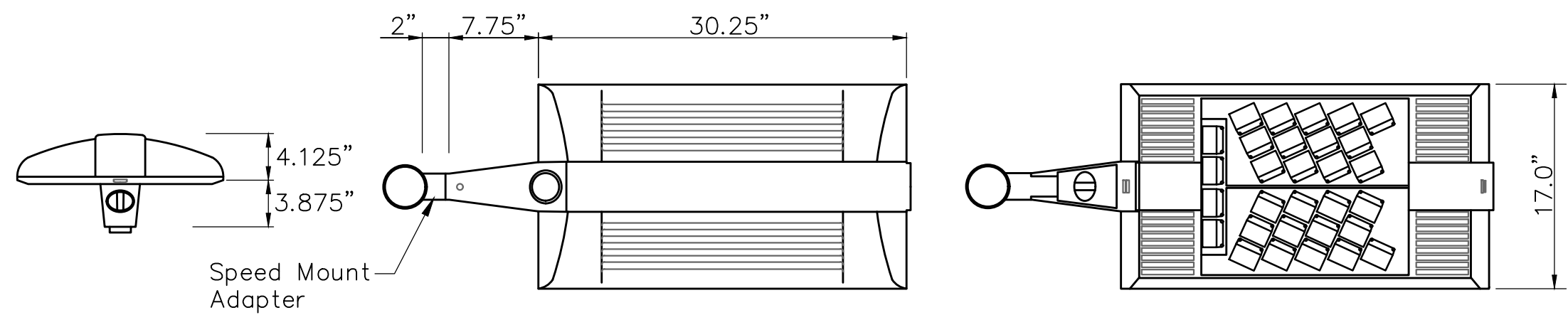
- Duke Energy 30' Style A–TB Pole Anchor Base**
- Depth Below Grade 66 Inches
  - Bolt Height 3.5” Above Concrete Surface
  - All Material Is To Be New, Unused And Meeting The Following Specifications:  
Conduit: Conduit, db–120 Heavy Wall, 2” 20’ Long, NEMA LC–8, PVC Covered  
Bar: Bar, Reinforcing, #6, 3/4” Dia x 16’–6” Long, 1.502 lb/ft, Deformed, CS, ASTM  
Rebar Hoods: Bar, Reinforcing, #3, 3/8” Dia x 18” Hoop Dia, 0.376 lb/ft, Deformed, CS ASTM  
Sonotube: Mold, Cardboard, 24” Dia x 12’ Long, Concrete, Heavy Duty Sonotube  
Conduit: Conduit, Rigid, Heavy Wall, 1”, 8’ Long, Sch 40, PVC, Rated f/ 90 Deg c Conductor  
Shim: Shim, Slotted, 1/16” Thk, Alum, f/ 3/4” Thru 1–1/4” Bolt Dia  
Ground Rod: Ground Rod, 5/8” Diameter, 8’ Long, Steel, Hot Dip Galvanized  
Clamp: Clamp, Grounding, Cable To Rod, 8 Sol–1/0 Str Cond To 5/8” Ground Rod, CU  
Ground Wire: Wire/Cable, Electrical, Bare, Ground, Sol SD, 4 Awg.  
Flush Mounted Anchor Bolts: Bolt, Anchor, 1” Diameter, 8 UNC 36” Long, W/4” Hook



**DUKE ENERGY 30' STYLE A–TB POLE ANCHOR BASE**  
Not To Scale

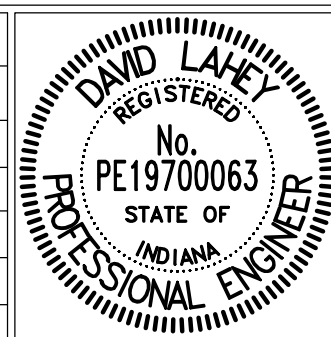


**BANNER ARMS**  
Not To Scale



**KIM LIGHTING ALT 120 ALTITUDE LED LIGHT FIXTURE DETAIL**  
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Rev. No.	Description	Date

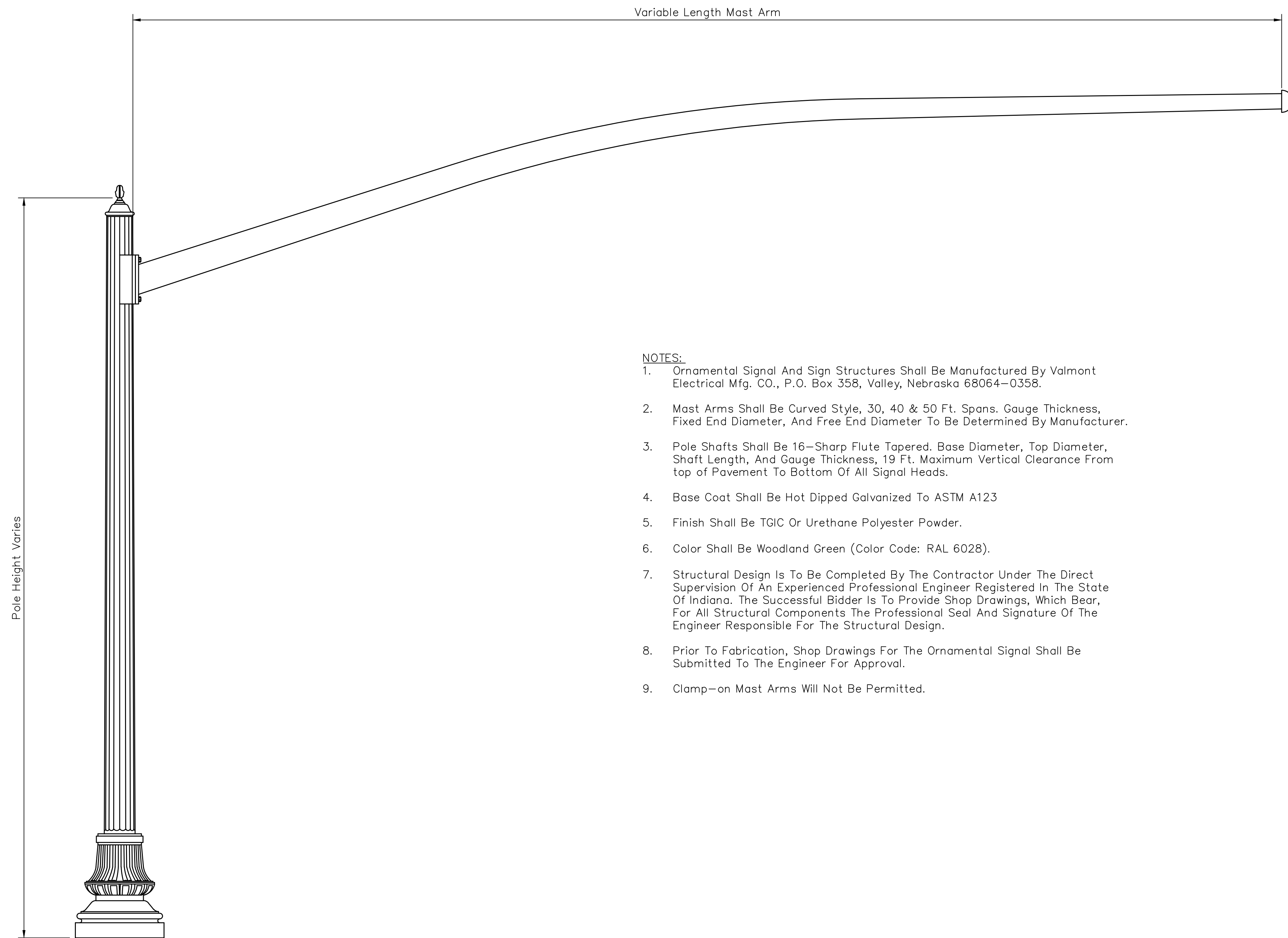


RECOMMENDED FOR APPROVAL	<i>David Loh</i>	01/01/2015
DESIGN ENGINEER		DATE
APPROVED	<i>David Loh</i>	01/01/2015
EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES		DATE
APPROVED	<i>Scott J. Hightower</i>	01/01/2015
DIRECTOR OF TRANSPORTATION		DATE

TOWN OF PLAINFIELD
STREET LIGHTING DETAILS APPLICABLE TO COLLECTORS & ARTERIALS (2)

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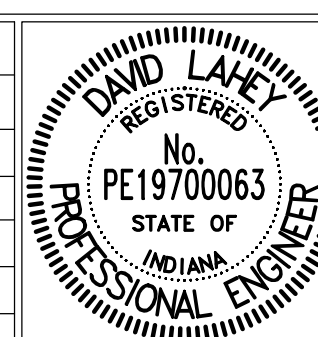
NOTES:

1. Ornamental Signal And Sign Structures Shall Be Manufactured By Valmont Electrical Mfg. CO., P.O. Box 358, Valley, Nebraska 68064-0358.
2. Mast Arms Shall Be Curved Style, 30, 40 & 50 Ft. Spans. Gauge Thickness, Fixed End Diameter, And Free End Diameter To Be Determined By Manufacturer.
3. Pole Shafts Shall Be 16-Sharp Flute Tapered. Base Diameter, Top Diameter, Shaft Length, And Gauge Thickness, 19 Ft. Maximum Vertical Clearance From top of Pavement To Bottom Of All Signal Heads.
4. Base Coat Shall Be Hot Dipped Galvanized To ASTM A123
5. Finish Shall Be TGIC Or Urethane Polyester Powder.
6. Color Shall Be Woodland Green (Color Code: RAL 6028).
7. Structural Design Is To Be Completed By The Contractor Under The Direct Supervision Of An Experienced Professional Engineer Registered In The State Of Indiana. The Successful Bidder Is To Provide Shop Drawings, Which Bear, For All Structural Components The Professional Seal And Signature Of The Engineer Responsible For The Structural Design.
8. Prior To Fabrication, Shop Drawings For The Ornamental Signal Shall Be Submitted To The Engineer For Approval.
9. Clamp-on Mast Arms Will Not Be Permitted.

DECORATIVE TRAFFIC SIGNAL POLE

Not To Scale

REVISIONS		
Rev. No.	Description	Date




RECOMMENDED FOR APPROVAL	<i>David L. Lay</i>	01/01/2015
	DESIGN ENGINEER	DATE
APPROVED	<i>David L. Lay</i>	01/01/2015
	EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES	DATE
APPROVED	<i>John J. Hightower</i>	01/01/2015
	DIRECTOR OF TRANSPORTATION	DATE

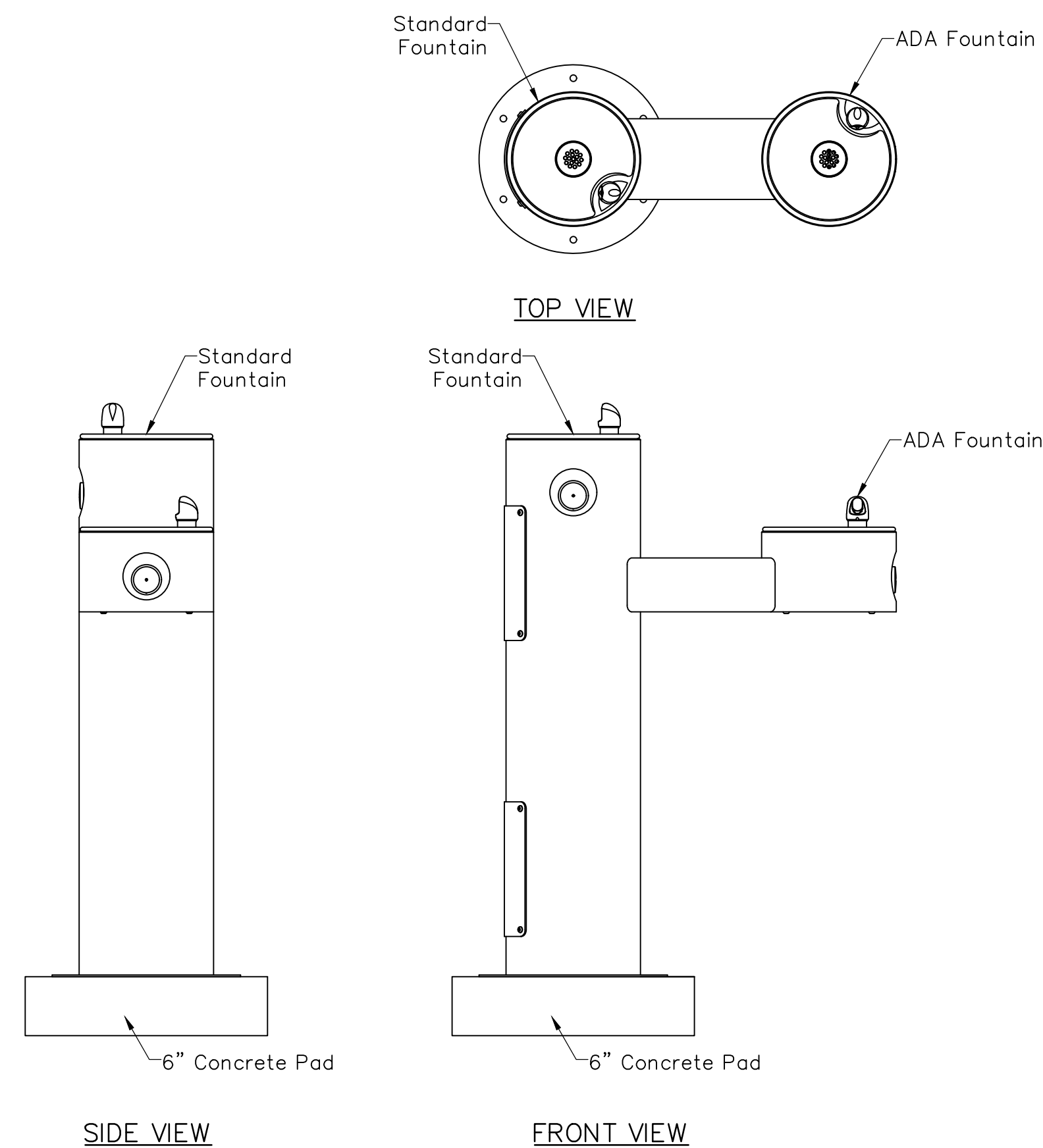
TOWN OF PLAINFIELD  
TRAFFIC SIGNAL DETAILS  
APPLICABLE TO COLLECTORS &  
ARTERIALS

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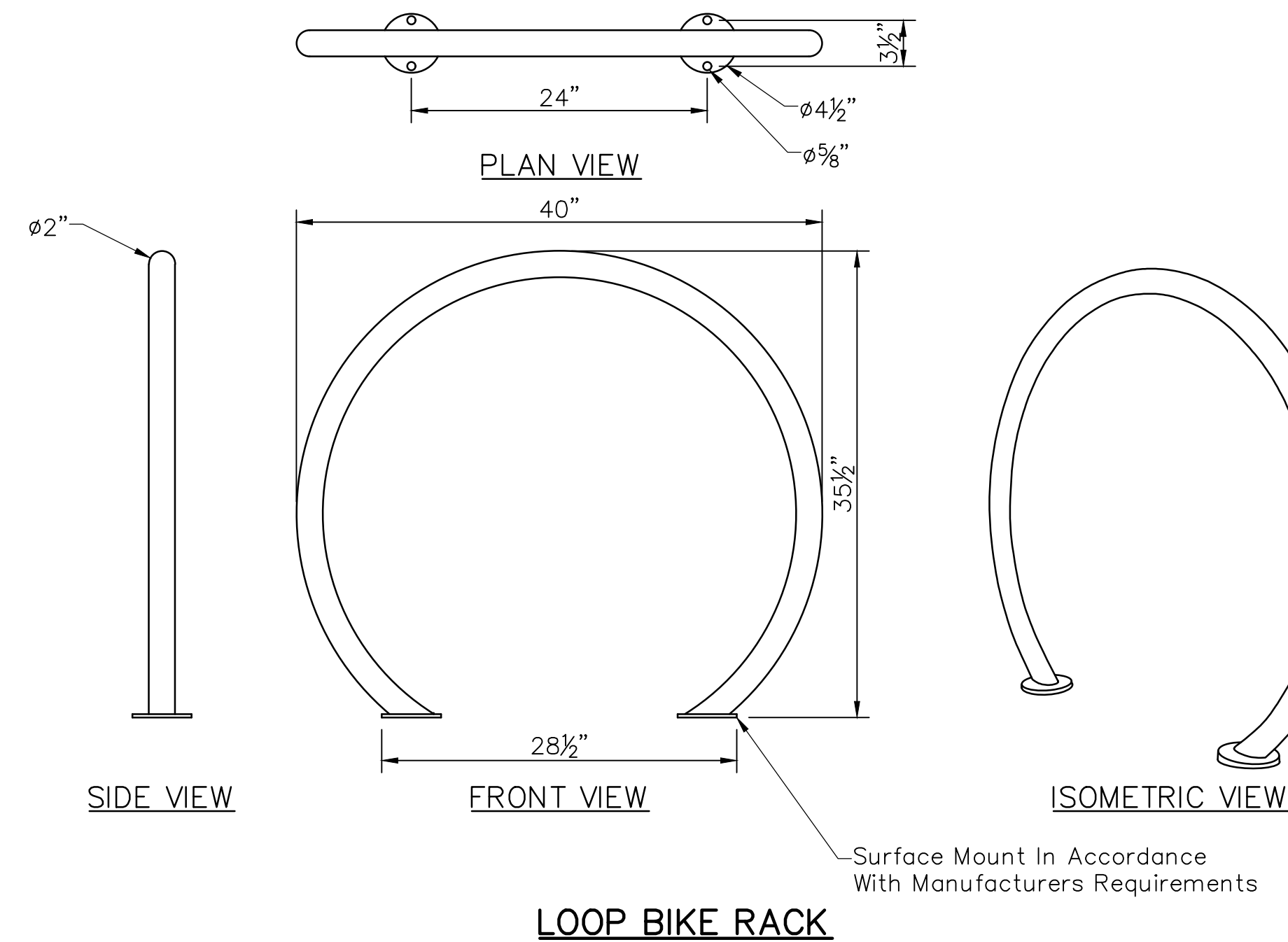
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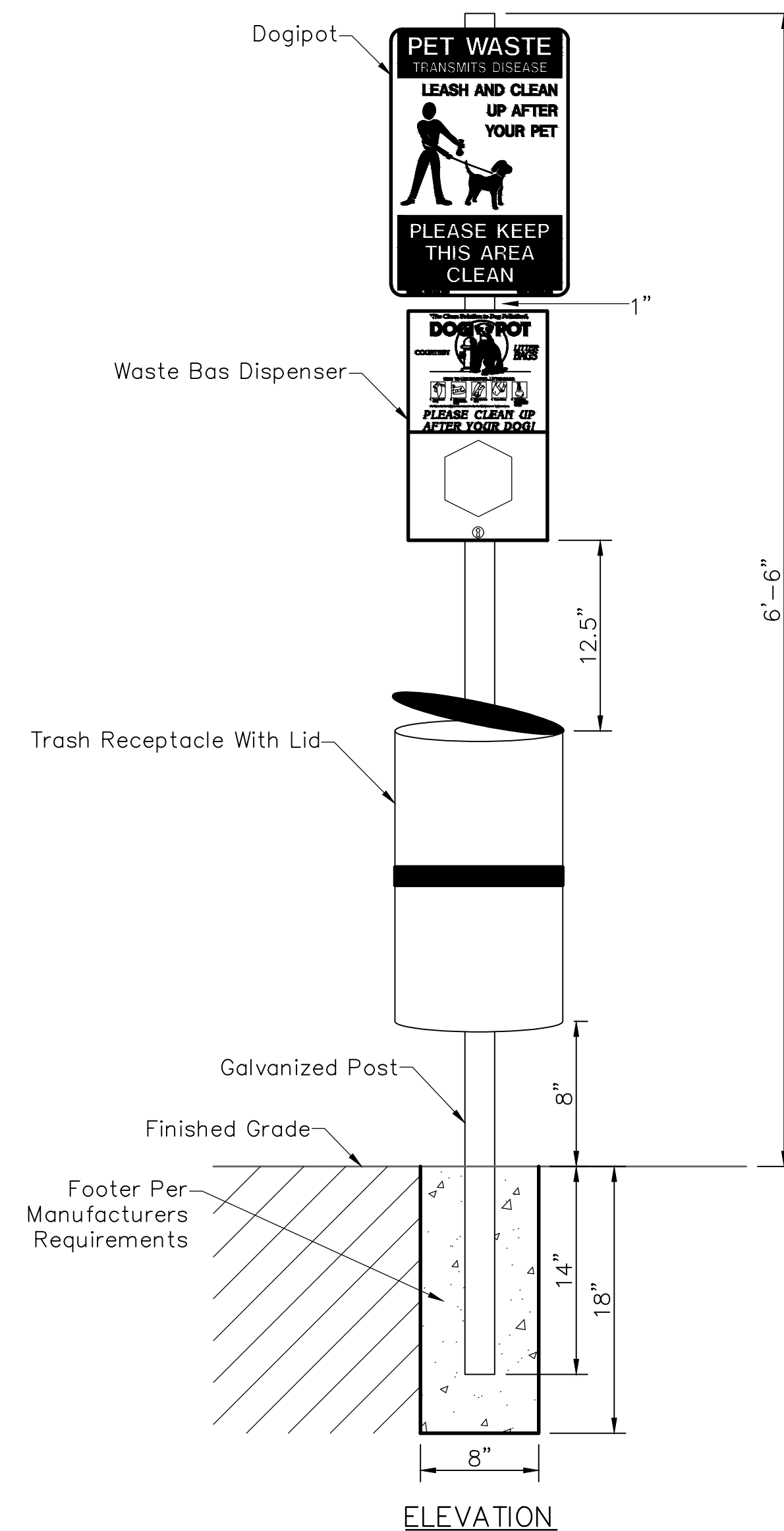
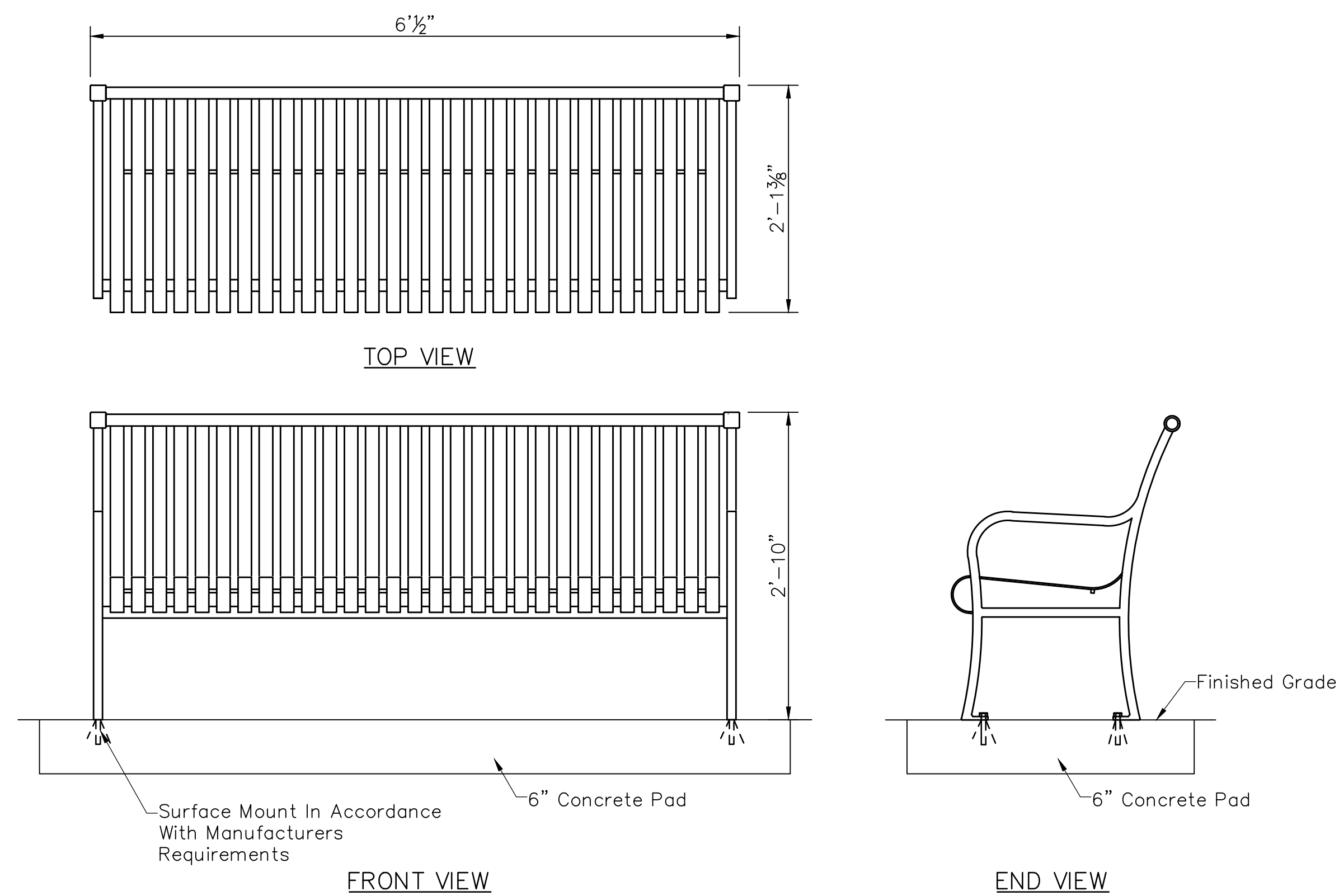
- WATER FOUNTAIN NOTES:**
1. Water Fountain Shall Be Elkay LK4420FRKEVG Evergreen Or Town Approved Equal:
  2. The Water Fountain Shall Also Include A Single  $\frac{3}{8}$ " X  $\frac{3}{4}$ " Residential Meter Pit In Accordance With DS-W03 Of The Town Standards. The Water Fountain Shall Be Equipped With Freeze Proof Valves From The Manufacturer.
  3. Each Water Fountain Shall Be Securely Mounted To The Concrete Surface Per Manufacturer's Specifications. All Plumbing, Connection, Water Service Connections, And Drain Lines Shall Be Constructed And Installed Per Manufacturer's Specifications And Guidelines.

### WATER FOUNTAIN



- PEDESTRIAN BENCH NOTES:**
1. Pedestrian Bench Shall Be Timberform Renaissance Bench, Length: 6', Evergreen Color.
  2. Each Bench Shall Be Installed Per Manufacturer Instructions And Shall Be Securely Mounted Per Manufacturer's Specifications With Either Surface Bolts, Or In-Ground Posts.
  3. Pedestrian Bench Shall Timberform Renaissance Model #2806 Bench (Evergreen).

### BENCH



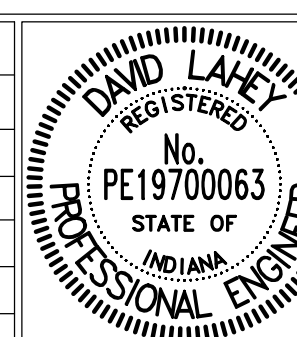
### DOGIPOT PET STATION 1003HP-L

- BIKE RACK NOTES:**
- Bike Racks Shall Be Selected From Following Three Options Or Town Approved Equal:
1. Reliance Foundry R-8224 Ring Bike Rack 20, Height: 31 1/2", Color: Dark Green
  2. Belson Outdoors Round Rack, Height: 35", Color Green
  3. Wabash Valley Open Circular Bike Rack, Height: 35 3/4", Color Green

Each Bike Rack Shall Be Surface Mounted As Per Manufacturer Instructions.

- GENERAL NOTES:**
1. All Water Fountains, Benches, And Loop Bike Racks To Be Installed On 6" Of Concrete On 6" Of No. 53 Stone.

REVISIONS		
Rev. No.	Description	Date



RECOMMENDED FOR APPROVAL	<i>David Loh</i>	01/01/2015
DESIGN ENGINEER		DATE
APPROVED	<i>David Loh</i>	01/01/2015
EXECUTIVE DIRECTOR OF DEVELOPMENT SERVICES		DATE
APPROVED	<i>Scott Fingleton</i>	01/01/2015
DIRECTOR OF TRANSPORTATION		DATE

TOWN OF PLAINFIELD

LANDSCAPE  
DETAILS

SHEET  
29  
OF  
29



The Contractor shall coordinate and check all dimensions relating to architectural features, mechanical equipment and openings, elaborate chases and overruns, etc., and notify the Architect/Engineer of any discrepancies or conflicts between drawings and field conditions.

The Structural Drawings shall be used in conjunction with the Drawings of all other disciplines and the Specifications. The Contractor shall verify the requirements of either set to disprove, chase, change, delete, add, modify, hole or alter items to be placed or in the Structural Work.

There shall be no vertical or horizontal service, hot or holes cut of any beam or column unless shown on the Structural Drawings or approved in writing by the SER.

Mechanical and electrical openings through supported slabs and walls, 8" diameter or larger than shown on the drawings shall be made at least 6" above and below the opening. A 6" diameter shall not have less than 10" clear between openings, unless approved in writing by the SER.

Verify locations and dimensions of mechanical and electrical openings through supported slabs and walls shown on the drawings. Verify that they are located as indicated on the drawings.

Do not install conduit in supported slabs, stone or grout, or concrete walls unless explicitly shown or noted on the Structural Drawings.

Do not suspend any items, such as ductwork, mechanical or electrical fixtures, ceilings, etc. from steel reinforcement bars or joists.

The Mechanical Contractor shall verify that mechanical members supported by steel framing are capable of spanning the distance between the supporting members indicated on the Structural Drawings. The Mechanical Contractor shall supply additional support framing as required.

If the Drawings and Specifications are in conflict, the most stringent requirements shall govern.

The Contractor shall be responsible for complying with all safety precautions and regulations during the work. The Contractor shall develop and implement safety and health training programs and procedures. The Structural Drawings represent the intended structure. The Contractor shall provide all temporary girding and bracing needed to erect and erect the structure in proper alignment until all permanent girding and bracing have been completed. The investigation, design, safety, adequacy and construction of the structure, including temporary supports, etc., is the sole responsibility of the SERS. The SERS shall not be responsible for the methods, techniques and sequences of procedures to perform the work. The Supervisor of the Work is the sole responsible for the Contractor.

The Contractor shall be responsible for the design of the structure. When conditions are not specifically shown, similar details of construction shall be used, subject to approval of the SERS.

All structural systems which are to be composed of components to be lifted erect shall be supervised by the Supplier during manufacturing, lifting, handling, storage, and erection in accordance with the Supplier's instructions.

Loading applied to the structure during the process of construction shall not exceed the safe load-carrying capacity of the structural members. The live loads used in the design of the structure are indicated on the drawings. The Contractor shall apply any concentrated loads and structural framing properly connected together and until all permanent bracing is in place.

As ASTM and other referenced standards and codes are the most latest editions of these publications, including amendments, the Contractor shall use the latest editions.

Shop drawings and other related details shall be submitted to the SERS for review prior to fabrication. All Shop Drawings shall be reviewed by the Contractor before the submission. The SERS review is to be for conformance with the design concept and design compliance with the relevant written documents. The SERS review does not relieve the Contractor of the responsibility to review, check, and coordinate the Shop Drawings prior to submission. The Contractor remains solely responsible for errors and omissions associated with the preparation of Shop Drawings as they pertain to member sizes, details, dimensions, etc.

Submit Shop Drawings electronically. In no case shall reproductions of the Contractor Documents be submitted. The Contractor shall use the following list of items for review:

- a. Concrete Mix Design(s)
- b. Reinforcing Steel Shop Drawings.
- c. Masonry Wall Reinforcing Steel Drawings.
- d. Structural Steel Shop Drawings.
- e. Steel Joist Shop Drawings.
- f. Steel Deck Shop Drawings.
- g. Cold-Formed Steel Framing Systems.
- h. Prefabricated Wood Truss and Wall Panel Systems.

Resubmit Shop Drawings: Resubmitted shop drawings are reviewed only for responses to comments made in the previous submittal.

When calculations are included in the submittals for components of work designed and certified by a Specialty Contractor, the Structural Engineer or Representative of SERS shall be for conformance with relevant Contractor Documents. The SERS review does not relieve the Specialty Structural Engineer or Representative for the design of the system(s) or the coordination with the elements of the structure under the contract. The Contractor shall be responsible for the design of the structure and shall coordinate and certify a warranty of the accuracy or completeness of the Specialty Structural Engineer's design. Contractors shall visit the site prior to but not ascertain conditions which may adversely affect the work or the design of the structure.

No structural member may be cut, notched, or otherwise weakened without written permission from the SERS.

When modifications are proposed to structural members under the design and certification of a Specialty Contractor, the Contractor shall submit the proposed modifications and be submitted to the SERS for review, prior to performing the proposed modifications.

placed on or in grade areas with a medium-weight roller or other suitable equipment to check for proper compaction. The Contractor shall be responsible for ensuring that the material is properly compacted and that the material is not disturbed. The Contractor shall be responsible for ensuring that the material is not disturbed. The Contractor shall be responsible for ensuring that the material is not disturbed.

Procedures for handling and disposal of materials shall be as outlined in the specifications. Proctoring operations shall be monitored by the Geotechnical Testing Agency. The Contractor shall be responsible for ensuring that the material is properly compacted and that the material is not disturbed. The Contractor shall be responsible for ensuring that the material is not disturbed.

The Standard Proctor maximum dry density (ASTM D698) at which the soil shall be stressed by foundation soils shall be approved granular materials compacted to a dry density of at least 100% (ASTM D698). The Contractor shall be responsible for ensuring that the material is properly compacted and that the material is not disturbed. The Contractor shall be responsible for ensuring that the material is not disturbed.

Compaction shall be accomplished by placing in layers of 4" and mechanically compacting each lift to at least the specified minimum dry density. For large areas of fill, density tests shall be performed for a minimum of one square foot of building area for each lift and, in addition, to insure adequate compaction, at least one square foot of building area for each lift.

Column footings and wall footings to be cast in natural soils or well-compacted building soils are assumed allowable bearing capacity of 100 PSF for column and wall footings.

It is the responsibility of the Contractor to ensure that the foundation soils are free of undesirable materials such as organic, existing fill, etc. is removed and that the foundation will bear on satisfactory material. The Geotechnical Testing Agency shall inspect the subgrade and perform any necessary remedial work. The Contractor shall be responsible for ensuring that the material is properly compacted and that the material is not disturbed. The Contractor shall be responsible for ensuring that the material is not disturbed.

The Testing Agency shall verify the bearing capacity at each spread column footing and every 10 feet on center for strip footings prior to placement of columns.

Placement of footings shall be in accordance with the specifications. It is possible the footings shall be adequately protected against any detrimental change in condition, such as from disturbance, rain and freezing.

It is the responsibility of the Contractor and each Sub-Contractor to verify the location of utilities and services shown, or not shown, and establish safe working conditions before commencing work. The Contractor shall pay for the utility and field verify all dimensions prior to excavation.

As reinforcing steel and threaded rod anchors to be installed in 2-part chemical anchoring system shall be installed as follows:

- Drill holes to be larger or at least to be embedded. Coordinate hole diameter with Manufacturer's requirements.
- Holes must be cleaned and prepared in accordance with Manufacturer's requirements.
- When reinforcing steel is encountered during drilling for installation of anchors, stop drilling and immediately remove the reinforcement from the hole. The hole shall be cleaned and prepared as possible to the original location. Contact the Structural Engineer for recommendations when the revised location is more than 2" from the original location or when the original condition of the concrete is significantly altered. When in doubt, contact the SEB for direction.
- Drill hole the minimum of 15 diameters or as shown on the Drawings.
- Use a 2-part anchoring anchoring system, like HIT-HY 200, approved equal.
- For concrete joints on hollow substrate, use HIT-HY-ATV 20, approved equal.
- Reinforcing steel products shall be ASTM A36, Grade 60, or unless noted.
- Reinforcing steel shall be ISO 9001 certified and shall be tested for fresh as noted on the Drawings. If not noted, provide cold hot-galvanized finish for stress anchors. Provide stainless steel finish for exterior applications, unless noted.

When columns and/or bolt/nut heads are corroded, or damaged by construction operations, the Contractor shall remove the material and replace with new to repair and/or replace.

- As a precaution, the affected bolt/nut shall be grout and braided after repair for the balance of the erection period.
- As an alternate to grout and braiding, the Contractor may if so option, employ a testing agency to perform pull test to confirm the strength of the repaired or replaced anchor/bolt/nut. The tensile pull load shall exceed 1.33x the design load of the original anchor without causing distress of the anchor/bolt/nut of the surrounding concrete. Reference the following table for the minimum proof loads.

1" diameter:	11.6 kips
1 1/8" diameter:	15.4 kips
1 3/8" diameter:	16.0 kips
1" diameter:	20.9 kips

Note: Values listed above are for ASTM F1554, Grade 36 material. When higher grade or strength materials are specified, refer to the AISC Manual of Steel Construction for minimum values to be multiplied by the appropriate strength reduction factor.

- When affected anchor/bolt/nut heads are a fixed moment-resisting column, such as those in moment-resisting space frame, canopies, or free-base installations, the repaired anchor/bolt/nut must be proof-loaded, or the affected column footing installed, per indicated in the entry.
- When affected anchor/bolt/nut heads are 1/2" diameter or larger, the affected column footing and pier must be replaced in its entirety.
- When affected anchor/bolt/nut heads are part of a braced frame, the affected column footing and pier must be replaced in its entirety.

Prior to erection, the controlling Contractor must provide written notification to the Steel Erection crew that has been a repair, replacement or modification of the anchor/bolt/nut for column.

**GENERAL STANDARDS:** The intended design standards and/or criteria are as follows:

Design	The 2014 Indiana Building Code
Concrete	International Building Code (IBC) with Indiana Amendments
Masonry	ACI 318
Aluminum	ACI 550 / TMS 402
Steel	AISC Manual, Allowable Stress Design (ASD)
Steel Joists/Girders	Steel Joist Institute
Steel Decks	Steel Deck Institute
Cold-Formed Steel	ASD-AISC

All referenced standards and codes, as well as ASTM numbers are for the latest editions of these publications, unless otherwise noted.

**DEAD LOADS:** Gravity Dead Loads used in the design of the structure are as computed for the materials of construction incorporated into the building, including but not limited to walls, floors, ceilings, partitions, internal partitions, finishes, cladding and other similar architectural and structural items, as well as mechanical, electrical and plumbing equipment, and materials, and material handling and feed service equipment, including the weight of cranes.

**COLLATERAL LOADS:** Unless otherwise noted, a minimum uniform collateral load of 10 PSF has been used to account for duckwork, clocks, sprinklers, lighting, etc. The collateral load is in addition to the weight of mechanical units, larger piping (greater than 7" diameter) and suspended trusses or equipment that have been specifically accounted for in the design.

**WIND LOAD / SNOW LOADS:** Gravity Live Loads used in the design of the roof structure meet or exceed the following table:

A. Snow Load	
Flat Roof Snow Load, $p_f$	20 PSF
Low-Slope Minimum Roof Snow Load, $p_f$	14 PSF
Snow Exposure Factor, $C_e$	1.0
Risk Category (IBC 2012, Table 1604.5)	1.0
Thermal Factor, $C_t$	1.0
B. Minimum Roof Live Load	20 PSF
C. Overhanging Eaves, Canopies & Projections	30 PSF

1. Drifts calculated in accordance with Section 7.7, ASCE 7. Specialty Engineers must consider drift loads in the design of pre-engineered trusses, frames, skylights, curtain walls, cold-formed metal framing, etc.

**HANDRAILS AND GUARDS:**

A. Handrail Assemblies and Guards	50 PLR applied in any direction
B. Components, Intermediate Railings, Balusters, Etc.	100 LB concentrated load applied in any direction (non-concurrent with 50 PLR load)

50 LB horizontally applied normal load on an area not to exceed 1 SF; not superimposed with those of handrail assemblies.

**LATERAL LOADS:** Lateral loads were computed using the following criteria:

A. Wind Load	
Ultimate Design Wind Speed, $V_{ult}$	115 MPH
Normal Design Wind Speed, $V_{des}$	89.1 MPH
Wind Exposure Category	II
Risk Category (IBC 2012, Table 1604.5)	1.0
Importance Factor, $C_{pe}$	+1.0
B. Seismic Load	
Site Class	D (Assumed)
Risk Category (IBC 2012, Table 1604.5)	II
Seismic Importance Factor, $I_p$	1.0
Mapped Spectral Response Acceleration Parameter, $S_s$	0.175g
Mapped Spectral Response Acceleration Parameter, $S_1$	0.081g
Seismic Design Response Acceleration Parameter, $S_{DS}$	0.167g
Seismic Design Response Acceleration Parameter, $S_{D1}$	0.046g
Seismic Design Category, SDC	Equivalent
Seismic Design Category, SDC	Equivalent
Seismic Force-Resisting System	Ordinary Reinforced Masonry
Response Modification Coefficient, R	2
Seismic Response Coefficient, $C_s$	0.093W

**SAFETY FACTORS:** This structure has been designed with "Safety Factors" in accordance with accepted principles of structural engineering. The fundamental nature of the "Safety Factor" is to compensate for uncertainties in the design, fabrication, and erection of structural building components. It is intended that "Safety Factors" be used such that the load-carrying capacity of the structure does not fall below the design load and that the building will perform under load without distress. While the design is intended to be conservative, the design is not intended to be over-engineered, such excess capacity cannot be adequately predicted and SHALL NOT BE RELIED UPON.

Where lites are not specifically shown or noted on the Structural or Architectural Drawings, provide the following lites for all openings and recesses in both interior and exterior non-load-bearing walls:

A) Brick: Masonry Opening      Angle Size  
Up to 5'-0"      Lx4x6/16  
5'-1" to 7'-0"      Lx4x6/16  
7'-1" to 12'-0"      Lx7x8/38

All angles are LVL (long vertical) unless otherwise specified. Provide 1" bearing length per span each end with minimum 8" long.

B) Block: For openings up to 8'-0" long exposed in the finished room, use lintel block filled with grout. All exposed joints and reinforcement as follows:

1. For 6" thick block: 2 - #5 bars.
2. For 8" thick block: 2 - #8 bars.
3. For 10" thick block: 2 - #10 bars.
4. For 12" thick block: 2 - #8 bars.

C) Block: For openings between 8'-1" & 12'-0" long exposed in the finished room, use lintel block with grout. Grout all exposed joints and reinforce per the "Long Masonry Lintel Detail" on page 10.

D) Stone all block and steel angle lintels over 8'-0" in length until masonry has attained its specified strength.

Details of placement of reinforcement, handling and placing of the concrete, construction of forms and fabrication of reinforcement not otherwise covered by the Plans and Specifications, shall comply with the following:

**Cold weather concreting** shall be in accordance with ACI 306. Cold weather is defined as a period when for more than 3 successive days the average daily air temperatures drops below 40°F and the specified minimum curing temperature is below the specified temperature of the placement or use.

**Hot weather concreting** shall be in accordance with ACI 305. Hot weather is defined as any combination of the following conditions that tends to impact the quality of the freshly made concrete:

- A hardened concrete high ambient temperature, high concrete temperature, low relative humidity, and/or
- A high rate of evaporation from the concrete surface.

A certified Testing Agency shall be retained to perform industry standard testing including measurement of slump, air temperature, concrete cylinder testing, etc. to ensure conformance with the Contract Documents. Submit reports to the Architect/Engineer.

**Formwork** shall be designed and constructed to meet the following operations have been completed, apply final finish as indicated below, and as described in the Division 3 Contract Plan in Place Formwork Specification of the Project Manual.

A. Floor Slabs	Hard Trowel Finish, unless noted otherwise
B. Walls and Sillwalls	Hard Trowel Finish, unless noted otherwise
C. Surfaces to Receive Topping Slabs	None - Finish
D. Surfaces to receive thick-mortar bed	None - Finish
D. Surfaces to cementitious toppings	None - Finish

Specify finishes: See the Specifications for sample and mockup requirements, if any. Coordinate finishes with the Architectural Finisher.

**Formwork** shall be designed and constructed to meet the following operations have been completed, apply final finish as indicated below, and as described in the Division 3 Contract Plan in Place Formwork Specification of the Project Manual.

A. Sides of Footings & Piles	Rough Form Finish
B. Sides of Grade Beams	Rough Form Finish
C. Surfaces not exposed to public view	Rough Form Finish
D. Surfaces exposed to public view	Smooth Form Finish

The Contractor shall consult with the Engineer with respect during concrete work to determine a satisfactory placing schedule and to determine the location of construction joints so as to minimize the effects of shrinkage and temperature stresses.

**Construction Joints** shall be located in accordance with the following:

Sawn or rolled control/joint reinforcement shall be provided in all slabs on ground. For a framed structural slab that shall be located on all column lines. Provide intermediate joints spaced at a maximum of 30 ft between the column lines. For slabs and interior walls without columns, slabs shall have a maximum joint spacing of 36 times the nominal slab thickness, or 40 feet, so that the maximum depth (ratio of long side to short side) does not exceed 1:5.

Where vinyl/plastic material, vinyl sheet goods, thick-skip epoxy tapes, or other similar material is the surface of the concrete, the Contractor shall coordinate the location of control/joint reinforcement and construction joints with the Finish Floor Contractor. Submit a dimensional plan showing joint locations and proposed sequence of four pours.

Unless specifically noted on the Plans, do not provide sawn control joints in composite and non-composite concrete slabs and medium density concrete slabs.

Joints in slabs to receive a finish floor may remain unsealed, unless required by the Finish Floor Contractor. All exposed slabs shall be filled with sealant specified in Division 7, or as follows: At slab industrial, manufacturing, or warehouse applications subject to wheeled traffic, shall be filled with sealant specified in Division 7, or as follows: At all other applications, shall be filled with sealant specified in Division 7, or as follows: At all other applications, as long as possible, preferably a minimum of 4 to 6 weeks after the slab has been cured. Prior to fill removal of debris from the slab joints, the fill in accordance with the manufacturer's recommendations.

Refer to the Architectural Drawings for locations and details of rebar ("I" maximum depth) in exposed concrete. Submit a detail of the rebar and details of rebar for corners of concrete. Where not indicated, provide 3/4" diameter exposed corner bars with 180° hook and 6" concrete stubbing minimum.

Refer to the Architectural Drawings for exact locations and dimensions of recessed slabs, ramps, stairs, thickened slabs, etc. Slope slabs to drains shown on the Architectural and Plumbing Drawings.

Stairways, sloops, aprons, drives, exterior walling/walks, and other slab concrete are not indicated on the Architectural Drawings. Refer to the Architectural Drawings for locations, dimensions, elevations, slopes, and finishing details.

FOOTINGS	
COMPRESSIVE STRENGTH	4000 PSI
MAXIMUM WATER/CEMENT RATIO	0.58
AIR CONTENT	0 - 3 PERCENT
WATER-REDUCING ADMIXTURE	OPTIONAL
SLUMP	4" +/- 1"
FOUNDATION WALLS, RETAINING WALLS, PIERS, GRADE BEAMS & TIE BEAMS	
COMPRESSIVE STRENGTH	4000 PSI
MAXIMUM WATER/CEMENT RATIO	0.50
AIR CONTENT	0 - 3 PERCENT
WATER-REDUCING ADMIXTURE	REQUIRED
SLUMP	4" +/- 1"
INTERIOR CONCRETE SLABS ON GRADE & SUSPENDED SLABS	
COMPRESSIVE STRENGTH	4000 PSI
MINIMUM CEMENTITIOUS MATERIAL CONTENT	517 LBC/UYD
AIR CONTENT	0 - 3 PERCENT
SLUMP	4" +/- 1"
WATER-REDUCING ADMIXTURE	REQUIRED
ES INTERNAL CURE ADMIXTURE	REQUIRED
EXTERIOR CONCRETE SUBJECT TO FREEZE-THAW	
COMPRESSIVE STRENGTH	4000 PSI
MINIMUM CEMENTITIOUS MATERIAL CONTENT	564 LBC/UYD
AIR CONTENT	6 +/- 1 PERCENT
WATER-REDUCING ADMIXTURE	REQUIRED
SLUMP	5" +/- 1"
COARSE AGGREGATE	CRUSHED STONE
INCREASE COMPRESSIVE STRENGTH TO 4500 PSI FOR EXTERIOR REINFORCED CONCRETE SUBJECT TO THE USE OF DEICERS	
LEAN CONCRETE FILL	
COMPRESSIVE STRENGTH	2000 PSI
MAXIMUM WATER/CEMENT RATIO	0.65
AIR CONTENT	OPTIONAL
WATER-REDUCING ADMIXTURE	OPTIONAL
SLUMP	4" +/- 1"

SUMP

MIXES CONTAINING TYPE I AGRD	5' MAXIMUM
MIXES CONTAINING MID-RANGE AGRD	6' MAXIMUM
MIXES CONTAINING HIGH-RANGE AGRD	5'-8"

SPECIFIED MINIMUM CEMENTITIOUS MATERIAL QUANTITIES ARE BASED ON THE USE OF WATER-REDUCING ADJUTANTS.

INCLUDE AN AIR-ENTRAINING ADJUTANT FOR ALL CONCRETE EXPOSED TO FREEZING AND THAWING IN SERVICE AND FOR ALL CONCRETE EXPOSED TO COLD WEATHER DURING CONSTRUCTION.

CONCRETE SHALL BE PLACED AND FINISHED TO MEET DESIGN COMPRESSIVE STRENGTH. REF. 304 FOR DEFINITION OF COLD WEATHER.

CLASS C FLY ASH MAY BE USED AS A CEMENT SUBSTITUTE WITH A MAXIMUM 20% SUBSTITUTION.

PROPORTION CONCRETE MIXES TO PROVIDE WORKABILITY AND CONSISTENCY TO PERMIT CONCRETE TO BE WORKED READILY INTO THE CORNERS AND ANGLES OF THE FORMS AND AROUND REINFORCEMENT BY THE METHODS OF PLACEMENT AND CONSOLIDATION.

ADJUSTMENTS TO THE APPROVED MIX DESIGNS MAY BE REQUESTED BY THE CONTRACTOR WHEN JOB CONDITIONS, WEATHER, TEST RESULTS, OR OTHER FACTORS WARRANT SUCH. REVISED MIX DESIGNS SHALL BE SUBMITTED TO THE ARCHITECT/ENGINEER FOR APPROVAL PRIOR TO USE.

Reinforcement, other than ASTM design bars for slabs and welded wire fabric, shall be deformed surfaces in accordance with ASTM A306.

Reinforcing steel shall conform to ASTM A618, Grade 60, unless noted.

Welded wire fabric shall conform to ASTM A1064, unless noted.

Where fabric is indicated, provide standard shapes per ACI and CRSI for all bars unless other fabric is specified.

Reinforcing steel shall conform to the following:

Reinforcement in footings, walls and beams shall be continuous. Lap bars in maximum of 36 diameters, unless noted otherwise.

Reinforcement shall be developed and secured against displacement in accordance with the Concrete Reinforcing Steel Institute's "Manual of Standard Practice."

Details of reinforcing steel fabrication and placement shall conform to ACI 315 Details and Detailing of Reinforced Concrete Structures, published by the American Concrete Institute, and Engineering and Placing Drawings for Reinforced Concrete Structures, unless otherwise indicated.

Spread reinforcing steel around small openings and lesions in slabs and walls, where possible, and around large openings in slabs and walls, where necessary. Reinforcing steel shall be placed in areas where necessary, and provide an area of reinforcement, equal to the interrupted reinforcement, in full length bars, distributing on each side of the opening. Where shrinkage and temperature reinforcement is required, provide a minimum of 1/4" x 4" bars on each side of the opening.

Provide individual bars in both faces, at each corner of openings larger than 12" in any direction. Provide reinforcement in all corners of openings.

Provide diagonal bars with, support bars, as required for the support of top reinforcement for supported slabs. Do NOT provide braces.

Provide max-in-place concrete wheels to provide required concrete cover for vertical wall reinforcement. When provided, the wheels shall extend downwards for the wall. Wheels shall be the same size and spacing as the vertical wall reinforcement, unless noted otherwise, with lips placed as shown on the application sections. Install dowels in the footing forms before concrete is placed. Do NOT stick dowels into the footing.

Field bending of reinforcing steel is prohibited, unless noted on the drawings.

Minimum concrete cover over reinforcing steel shall be as follows, unless noted otherwise on plan, section or detail.

		MINIMUM COVER
SLABS AND JOISTS		
TOP & BOTTOM BARS FOR DRY CONDITIONS:		
#11 BARS & SMALLER		3/4"
#14 & #18 BARS		1 1/2"
FORMED CONCRETE SURFACES EXPOSED TO EARTH, WATER, OR WEATHER, AND OVER OR IN CONTACT WITH SEWAGE AND FOR BOTTOMS BEARING ON WORK MAT, OR SLABS SUPPORTING EARTH COVER:		
#5 BARS & SMALLER		1 1/2"
#6 THROUGH #18 BARS		2"
BEAMS & COLUMNS, FORMED		
FOR DRY CONDITIONS:		
STIRRUPS, SPIRALS & TIES		1 1/2"
PRINCIPAL REINFORCEMENT		2"
EXPOSED TO EARTH, WATER, SEWAGE, OR WEATHER:		
STIRRUPS & TIES		2"
PRINCIPAL REINFORCEMENT		2 1/2"
WALLS		
FOR DRY CONDITIONS:		
#11 BARS & SMALLER		3/4"
#14 & #18 BARS		1 1/2"
FORMED CONCRETE SURFACES EXPOSED TO EARTH, WATER, SEWAGE, WEATHER, OR IN CONTACT WITH GROUND		
FOOTINGS & BASE SLABS		
AT FORMED SURFACES & BOTTOMS BEARING ON CONCRETE WORK MAT		2"
AT UNFORMED SURFACES & BOTTOMS IN CONTACT WITH EARTH		3"
TOP OF FOOTINGS		SAME AS SLABS
OVER TOP OF PILES		2"

Structural steel construction shall conform to the American Institute of Steel Construction Specification for Structural Steel Buildings.

A. All structural wide flange members and channels shall be ASTM A990, Fy = 50 ksi.

B. All plates, bars, angles, and rods shall be ASTM A572, Grade 50 unless noted.

C. All rectangular, square, and round structural tube members shall be ASTM A583, Grade C, Fy = 50 ksi unless noted.

D. Details for erection, fabrication and erection of all structural steel will be in accordance with the latest AISI Standards unless otherwise noted or specified.

E. Provide temporary erection jacking and bracing as required.

F. Unless otherwise ordered or noted on the Drawings, provide 8" minimum bearing edge and end for all base plates and beams.

G. For loose bolts, masonry shell angles and/or other items generally not shown on the Structural Drawings refer to the following notes:

1. Bolts shall be galvanized. See general notes on Items above for size, strength, etc.

2. Steel columns below grade shall be encased in a minimum of 4" concrete or paralled with 2 coats of asphaltum paint, unless otherwise shown.

**Typical beam-to-beam and beam-to-column connections shall be bearing type using A325 bolts, unless noted otherwise.**

**Welded connections, unless otherwise shown, may be either bolted or welded. All connections shall be bolted unless otherwise shown on the Structural Drawings.**

**Connections shall be designed by the Steel Fabricator to support the reactions shown on the framing plans.** Simple span connections without reactions listed on the Structural Drawings shall be designed in accordance with AISC 358, Section 14.4 of the AISC "Manual of Steel Construction, 14th Edition." For composite beams where reactions are not indicated, design connections for 75% of the Maximum Total Uniform Load ASD value for the applicable beam size and span length. For non-composite beams, design connections for 50% of the tabulated ASD value. The minimum shear connection load shall be 35 kips.

**Submit calculations for connections not allowed on the Structural Drawings and not covered by the AISC 358, 358.10 and not limited to:**

- A. Moment Connections
- B. Bracing Connections including Collectors and Drag Struts
- C. Splayed Shear Connections
- D. Girder To Truss Splices
- E. Truss-to-Column and Truss-to-Truss Connections
- F. Truss Web-to-Chord and Web-to-Gusset Connections

**All beam-to-beam connections shall be double angle, unless shown or noted otherwise.**

**All beam-to-column connections shall be double angle, unless shown or noted otherwise.**

**Shear lap connections to tube columns are permitted unless otherwise noted or detailed.**

**Typical bearing type beam-to-beam, and beam-to-column field connections may be tightened to the snug-tight condition, unless otherwise shown or noted.**

**Connections to be welded shall include, but not be limited to, gables and sub columns, crane connections, and those designated "PT" (pretensioned). Drawings for both pretensioned joints utilizing tension-control (TC) bolts and PT detection indicators. Holes for bolts in pretensioned joints shall be drilled and reamed to the full depth of the bolt hole. Welds shall be approved by the Testing Agency.**

**Connect bracing members from two components of steel structures approved by the SEER. Provide a minimum 2-bolt or welded field connection.**

**Locate connections of all vertical bracing members on column connections in vertical plane and on column and beam connections in horizontal plane, unless otherwise shown on the Structural Drawings.**

**All welding shall be in accordance with AWS D1.1, using E70XX electrodes, unless shown or noted otherwise.**

**Connections shall be designed to perform by welds in accordance with the weld type and positions involved according to the current edition of AWS D1.1. Perform all AESS welds with care to provide a clean, uniform appearance.**

**Backup bars required for welded connections shall be continuous.**

**Holes in steel shall be drilled or punched. All drilled holes shall be provided with smooth edges.**

**Burrs and slag from structural steel shall be removed without approval of the SEER.**

**The minimum thickness of all connection material shall be 5/16", unless noted.**

**Continuous beam plate and angle side columns, roof edges, diaphragm chords, etc. around perimeter of floor and roof, as well as around openings shall be welded with a minimum 1/4" fillet weld x 3" long x 12" wide.**

**For continuous perimeter angles and plates perpendicular to and connected to the tops of chords of floor and roof, provide a minimum 3/4" x 1/4" weld at each edge. Continuous angle and beam plates may be shop-welded to the chord, but only when they are required to perform inspection in writing by the Testing Agency.**

**A qualified independent Testing Agency shall be retained to perform inspection and testing of structural steel weldments as follows:**

WELD TYPE	VT	MT	UT	PT	RT	COMMENTS
FILLET (SINGLE PASS)	25%	--	--	--	--	ROOT PASS AND FINISHED WELD
FILLET (MULTIPLE PASS)	50%	25%	--	--	--	
FLARE BEVEL/ FLARE V	25%	--	--	--	--	
GROOVE (PARTIAL PENETRATION)	100%	--	100%	--	--	REFERENCE NOTE 'E' BELOW
GROOVE (FULL PENETRATION)	100%	--	100%	--	--	ALL FULL PENETRATION WELDS

- A) Test procedures:
  - PT = Visual Test (inspection)
  - MT = Magnetic Particle Test, ASTM E109, cracks or incomplete fusion or penetration not acceptable. UT = Ultrasonic Test, ASTM E646.
  - PT = Penetrant Test, ASTM E165.
  - MT = Magnetic Particle Test, ASTM E109 and ASTM E142, min. quality level 2-1.
- B) Acceptance standards in AWS D1.1 shall be followed for each test procedure.
- C) Test procedures may be substituted to meet feasibility requirements of test based upon weld geometry or other factors with the approval of the SER.
- D) Samples shall call out random locations, additional tests may be required at locations noted on the Drawings.
- E) Groove welds include square, bevel, V and J grooves including single and double pass type.
- F) Partial penetration square groove welds at end seal plates of tubular members do not require inspection.
- G) Weld Performance Specifications (WPS) shall be produced and maintained in accordance with AWS D1.1. The independent Testing Agency shall have access to all WPS during the course of testing and inspection.
- H) For highly-restrained welded joints, especially in thick plates and heavy structural shapes, detail the welds and the thickness openings as much as possible in the direction the force is applied. Refer to the ASCE 10-10, Section 10.10.2.2 for details on how to detail joints that reduce the possibility for brittle tearing. Members subjected to repetitive high-restrained connections shall be tested by the independent Testing Agency by Ultrasonic Testing to determine the commercial welding.
- I) In addition to inspection requirements for fillet welds in Table above, 100% of field welding of diagonal bracing members to gusset plates shall be visually inspected (VT).

A Specialty Structural Engineer (SSE) is defined as a Professional Engineer licensed in the State of Indiana who is a member of the American Institute of Steel Construction, Inc. (AISC) and is duly qualified by training and experience to perform the duties of a Structural Engineer or Record Sizer, who performs Structural Engineering functions necessary for the construction to be completed and who shows a knowledge of engineer or trainee in the specific specialty.

The SSE is responsible to review the Construction Drawings and Specifications to determine if the drawings and specifications are appropriate scope of engineering.

The SSE will submit a list of the Drawings and Specifications to provide sufficient information for the SSE to perform his design and analysis. If the Engineering drawings are detailed, in projects, or unduplicated project limits which conflict with the Engineering requirements as described in the project documents, the SSE and the client will contact the SER for resolution of conflicts.

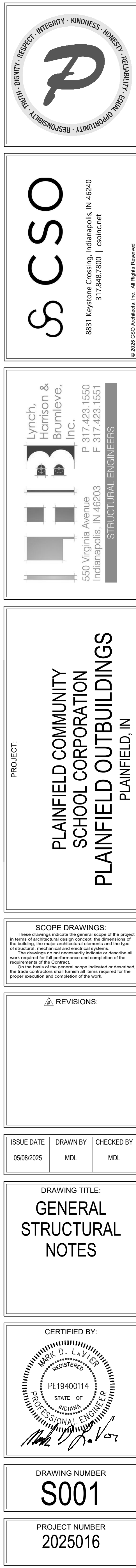
The SSE shall include drawings to the SER for review. Such documents shall bear the stamp of the SSE and shall conform to the following:

1. Drawing indicating engineering input, such as defining the configuration or structural capacity of structural components and/or their assembly into structural systems.
2. Calculations.
3. Construction details which are an acceptable substitute for manual calculations provided they are accompanied by sufficient design assumptions and identified input and output information to permit their proper evaluation. Such information shall bear the stamp of the SSE as an indication that the design is the responsibility of the SSE.

Contractors are required to specify the specific technical specification sections and the structural drawings for those elements requiring Structural Engineering. Examples of components requiring Structural Engineering are listed below:

- a. Temporary and Permanent Retention Systems, if required.
- b. Shoring and Bracing Systems, if required.
- c. Structural Steel Connections.
- d. Prefabricated Wood Joists.
- e. Steel Stairs.
- f. Handrails and Guards.
- g. Cold-Formed Steel Framing.
- h. Curtain Wall Systems.

When modifications are proposed to elements under the design and construction of the SSE, written requests for change shall be obtained and submitted to the SER for review prior to performing the proposed modification.





TRENCH FOOTING SCHEDULE				
FTG. MARK	FOOTING SIZE		FOOTING REINFORCING	
	WIDTH	DEPTH	LONGITUDINAL	TRANSVERSE
TF24	2'-0"	2'-0"	(2) #5 x CONTIN. T&B	#4 x 1'-6" @ 96" O.C. T&B
NOTES: 1. CENTER FOOTINGS BENEATH WALLS, U.N.O. 2. REF. S401 FOR TYP. WALL FOOTING CONSTRUCTION JOINT DETAIL. 3. LAP FOOTING REIN. A MIN. OF 30 BAR DIAMETERS. PROVIDE BOTH TOP & BOTTOM REIN. AT ALL FOOTINGS.				

DIAPHRAGM NAILING SCHEDULES		
ROOF DIAPHRAGM		
LOCATION	SIZE	SPACING
BOUNDARY	8d	6"
PANEL EDGE	8d	6"
FIELD	8d	12"
1. 1-1/2" MINIMUM PENETRATION INTO FRAMING. 2. DIAPHRAGMS ARE UNLOCKED, U.N.O. 3. ALL NAILS ARE COMMON NAILS. REF. SCHEDULE THIS SHEET FOR MIN. LENGTHS AND SHANK DIAMETERS.		

DIMENSIONS OF COMMON NAILS			
PENNYWEIGHT	MIN. LENGTH, IN. INCHES	SHANK DIA., IN. IN.	
6d	2	0.113	
8d	2.5	0.131	
10d	3	0.148	
16d	3.5	0.162	
20d	4	0.192	
1. NAILS CALLED OUT IN PLAN, SECTION, DETAIL, OR SCHEDULE ARE ALWAYS COMMON NAILS. NAIL DIAMETER (S) PER ESR-1538, NDS, AND THE TABLE ABOVE EXCEPT THAT NAIL LENGTH WILL ALWAYS BE 3" MINIMUM WHEN NAILING 2x FRAMING MEMBERS TOGETHER AND 3 1/2" WHEN NAILING LVL'S. 2. FOR CONNECTIONS NOT SHOWN, REFER TO IBC TABLE 2304.9.1 FOR MINIMUM FASTENING REQUIREMENTS. 3. FOR FASTENING OF MULTIPLE LVL PILES, FOLLOW THE LVL MANUFACTURER'S REQUIREMENTS. 4. FOR FASTENING OF SIMPSON AND OTHER HARDWARE, FOLLOW THE HARDWARE MANUFACTURER'S REQUIREMENTS. FILL ALL FASTENER HOLES WITH THE REQUIRED FASTENERS, U.N.O.			

### WOOD STRUCTURAL DATA

- All interior non-load bearing or shear wall studs, top and bottom wall plates shall be #1 Spruce - Pine - Fir - Northern per N.L.S.A. or better:  
Fb = 875 psi  
Ft = 450 psi  
Fv = 70 psi  
Fc (PER) = 425 psi  
Fc (PAR) = 1,150 psi  
E = 1,400,000 psi  
Fb = 850 psi  
Ft = 500 psi  
Fv = 85 psi  
Fc (PER) = 625 psi  
Fc (PAR) = 1,400 psi  
E = 1,400,000 psi
- All exterior and shear wall studs, top and bottom wall plates shall be #1 Douglas Fir-Larch, or better:  
Fb = 1,000 psi  
Ft = 675 psi  
Fv = 75 psi  
Fc (PER) = 375 psi  
Fc (PAR) = 1,450 psi  
E = 1,600,000 psi
- All rafters, ceiling joists, ridge and valley boards, headers and misc. blocking shall be #2 hem-fir, or better (alternate: Douglas Fir-Larch, see table above).  
Fb = 1,000 psi  
Ft = 675 psi  
Fv = 75 psi  
Fc (PER) = 375 psi  
Fc (PAR) = 1,450 psi  
E = 1,600,000 psi
- All laminated veneer lumber (L.V.L.) shall be manufactured by Trus-Joist McMillan, Alpine Structures, Inc., Or Mike Wood Products:  
Fb = 2,800 psi  
Ft = 265 psi  
E = 2,000,000 psi  
NOTES:  
A. Refer to specifications for additional information.  
B. Alternate grades and species may be proposed. Any substitutions must be approved in writing by the architect/engineer.  
C. Allowable stresses for dimensional lumber listed above are for 2'-4" thick x 2" & wider. Size adjustment factors (Cf), repetitive member factor (Cd), duration of load factor (Cd), etc. have been applied in the design of structural elements.

### WOOD FRAMING NOTES

- For wood connections not specifically noted or detailed, follow the requirements of IBC 2006 Table 2304.9.1 or ESR 1539.
- All nails are common nails unless noted otherwise. All nails shall be carefully driven and not overdriven. Submit all proposed fasteners for approval prior to construction. Installation of all fasteners shall meet the requirements of NDS and ISANTA guidelines, including those in ESR 1539, and Section 2303.6 of the IBC.
- All nails are common nails unless noted otherwise. All nails shall be carefully driven and not overdriven. Submit all proposed fasteners for approval prior to construction. Installation of all fasteners shall meet the requirements of NDS and ISANTA guidelines, including those in ESR 1539, and Section 2303.6 of the IBC.
- Wall plates are to be #1 or #2 Spruce-Pine-Fir (SPF) with stud spacing 16" o/c maximum.
- Use double top plates on all walls, including non-load-bearing walls, with all splices and corners lapped. At "T" intersections do not top plate of intersecting wall cutting the top plate of the continuous wall, rather use a metal tie plate as described in the exception to Section 2306.9.2.1 of the IBC.
- Coordinate final roof framing including joist or truss layout & truss member configuration with Mechanical, Electrical, & Plumbing (MEP) drawings. Obtain additional MEP information as needed for complete coordination. Keep all mechanical chases free of framing. Do not locate joists or trusses at parallel blocking walls.
- Design roof joists or trusses to support the weight of snow drifting where it applies, as well as rooftop mechanical units, exhaust fans, access hatches, etc. Confirm weights & locations before final design and show the loads for these substitutions on the sealed drawings. The Contractor shall ensure the units are installed at their design locations.
- Where framing supported by a joist or truss can cause uplift on that joist or truss (such as at cantilevered balcony framing) the designer shall consider a load case that maximizes the uplift load in combination with no live load applied to the joist or truss supporting the uplift.
- All hardware to be Simpson Strong Tie or approved equal. Where hardware is not specifically designated, submit proposed hardware for approval. Where more than one type of fastener or fastener pattern is allowed by the hardware manufacturer, hardware fasteners are to be of the type, size, and quantity to maximize the load capacity of the hardware in the specific application shown on these plans, unless noted otherwise.
- Reference the Architectural Plans for layout of all walls, openings, wall types, etc. Verify all dimensions prior to design of wall panels & immediately notify the Architect and Engineer of any discrepancies.
- Where a Specialty Structural Engineer (SSE) designs framing (such as trusses), the roof designer shall provide the Wall Panel Designer the loads/reactions and locations of all girder or beam bearing points. The Wall Panel Designer shall specify and the Wall Panel Manufacturer shall install sufficient columns/studs to support all such loads from the girder or beam bearing location down to the supporting foundation or postum framing. The Contractor shall ensure the presence of such columns/studs. Similarly, where walls are field-framed, the Framing Contractor shall install the columns/studs for support of girders and beams. As a minimum, the number of studs shown on the plans shall be used, with a minimum of (2) 2x6 or (3) 2x4 studs.

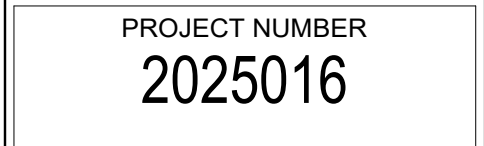
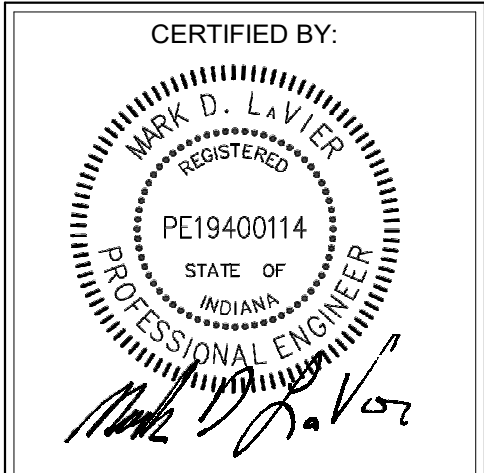
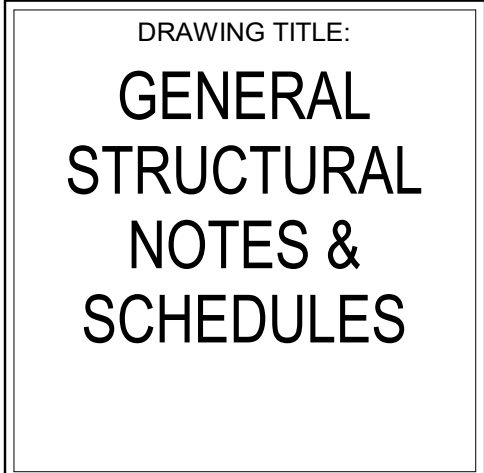
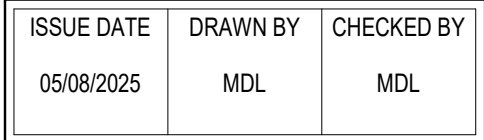
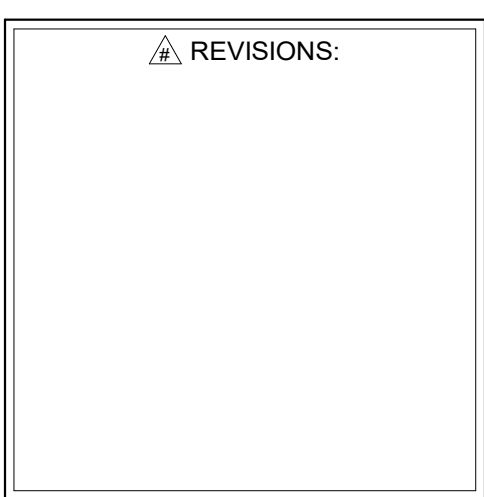
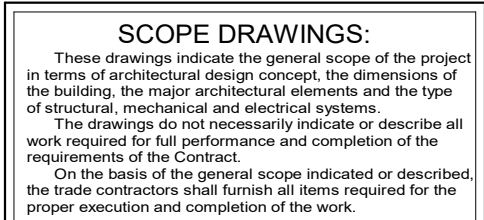
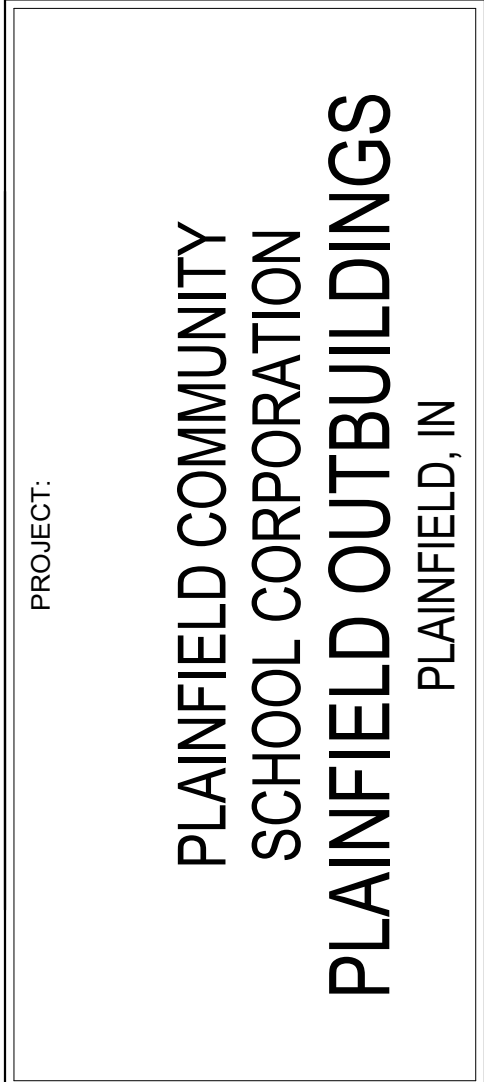
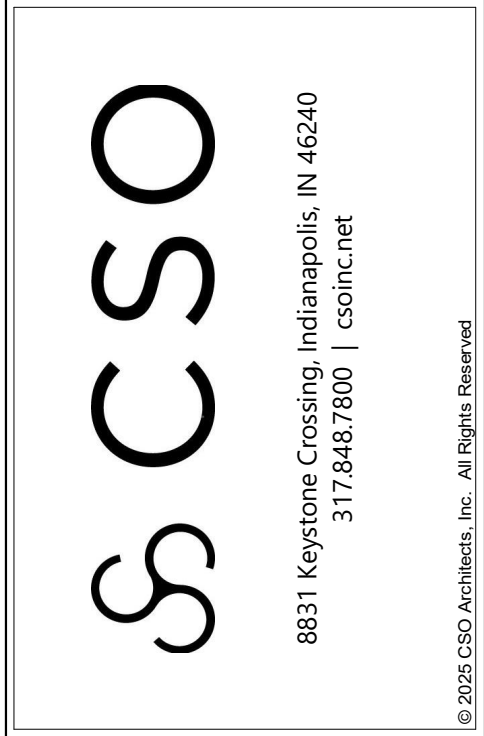
### PREFABRICATED WOOD TRUSSES

- Wood trusses shall be designed by the manufacturer to support the following loads:  
A. Roof Trusses  
Top chord loading:  
Live (snow) load : 20 psf + drift loads  
Dead loads : 12 psf  
Additional 5 psf at overbuild framing areas  
Bottom chord loading:  
Add live load : 10 psf  
Dead load : 5 psf
- Wood trusses shall be designed by the manufacturer in accordance with the applicable provisions of the latest edition of the National Design Specification of the National Forest Products Association. The design specification for the metal plate connected wood trusses of the Truss Plate Institute (TPI), Wood Truss Council of America (WTCA) and the Indiana Building Code, 2008.
- Wood materials shall be Southern Pine, or Douglas Fir-Larch and shall be kiln-dried and used at 19% maximum moisture content. Provide grade no. 2, or as required to satisfy stress requirements.
- Connector plates shall be not less than 0.036 inches (20 gage) in coated thickness, shall meet or exceed ASTM Grade A or higher and shall be hot-dipped galvanized according to ASTM A 653 (coating G60). Minimum steel yield stress shall be 33,599 psi.
- Trusses shall be fabricated in a properly-equipped manufacturing facility of a permanent nature. Trusses shall be manufactured by experienced workmen, using precision cutting, jigg and pressing equipment under the requirements in quality control standard QST-86 of the Truss Plate Institute.
- Secondary bending stresses in truss top and bottom chords due to dead, live, and wind loads shall be considered in the design. Load duration factors shall be per the National Design Specification for Wood Construction.
- All top and bottom chords shall be a minimum of 2x6, with the exception of valley overbuild trusses, cap trusses, and non-spanning gable-end trusses, unless approved by the Structural Engineer of Record.
- All girder trusses supporting other trusses of 2x4 framing members shall be a MINIMUM of 2-piles, unless otherwise approved. Refer to the Mfr's Truss Design Drawing for girder ply-to-ply connection requirements. Attach framing members or loads only after all girder piles are in place and properly fastened together, and the girder truss is properly braced to prevent lateral displacement. Refer to BCSI-89 "Multi-Ply Girders" as published by WTCA and TPI for additional information.
- Truss-to-girder connection information shall be on the Mfr's Truss Design Drawing of the carried-truss, girder truss, or the Mfr's Truss Placement Drawing. Unless otherwise approved, all joist hangers, strapping, ties, etc. shall be as manufactured by the Simpson Strong Tie Company.
- Unless otherwise shown or noted, ALL truss bearings shall be anchored using a mechanical fastener. As a minimum, provide H-1 wind/seismic anchors as manufactured by the Simpson Strong Tie Company.
- Wood trusses shall be erected in accordance with the truss manufacturer's requirements. This work shall be done by a qualified and experienced contractor. Truss erection by an inexperienced or non-qualified contractor can result in construction collapse and/or serious injury damage.
- The contractor shall provide all temporary and permanent bracing as required for safe erection and performance of the trusses. The guidelines set forth by the following joint publications of the Truss Plate Institute (TPI) and Wood Truss Council of America (WTCA) shall be adhered to unless otherwise noted in the Contract Documents:  
BCSI-81 GUIDE FOR HANDLING, INSTALLING AND BRACING OF METAL PLATE CONNECTED WOOD TRUSSES  
BCSI-82 TRUSS INSTALLATION AND TEMPORARY BRACING  
BCSI-83 WEB MEMBER PERMANENT BRACING/WEB REINFORCEMENT  
BCSI-84 CONSTRUCTION LOADING  
BCSI-85 TRUSS DAMAGE, JOBSITE MODIFICATIONS AND INSTALLATION ERRORS  
BCSI-86 GABLE END FRAME BRACING  
BCSI-87 TEMPORARY AND PERMANENT BRACING FOR PARALLEL CHORD TRUSSES  
BCSI-88 TOE-NAILING FOR UPLIFT REACTIONS  
BCSI-89 MULTI-PLY GIRDERS  
BCSI-910 POST FRAME TRUSS INSTALLATION AND BRACING  
BCSI-911 FALL PROTECTION AND WOOD TRUSSES
- Unless otherwise shown or noted, permanent bracing shall consist of 2x4 stress-graded members spanning a minimum of four trusses and nailed at each intersection with a minimum of (2) 16d nails. Lap continuous bracing a minimum of 2'-0" (2 trusses).
- Refer to Mfr's Truss Design Drawing for web members requiring web member permanent bracing/web reinforcement. Continuous lateral bracing must ALWAYS be diagonally braced for rigidity.
- Refer to the Truss Bracing Schematics, and the structural framing plan and sections for permanent bracing requirements. Wherever possible, the temporary erection bracing as described in BCSI-82 shall be left in place to function as permanent bracing.
- Trusses which are too tall for delivery to the jobsite in one piece may be manufactured in two or more sections and "jigged/braced" at the jobsite. The contractor MUST install temporary and permanent bracing for the lower supporting trusses as shown on the Mfr's Truss Design Drawing and/or the Contract Documents BEFORE installing the cap trusses. Provide 2x4 sleepers laid flat on the top chord of the supporting trusses, spaced at 24" on center and nailed to the top chord with a minimum of (2) 16d nails.
- Truss members and components shall not be cut, notched, drilled nor otherwise altered in any way without the written approval of the Truss Mfr's Engineer (See Specialty Structural Engineer notes).
- Submit complete shop drawings for all wood trusses showing member sizes, species, grade, moisture content, span, camber, dimensions, chord pitch, bracing requirements and loading. Shop drawings shall be submitted to the Engineer and shall bear the seal of a Professional Engineer registered in Indiana.

### WOOD PANEL/SHEATHING NOTES

- All plywood construction shall be in accordance with the American Plywood Association (APA) specification.
- All roof panel sheathing shall be 5/8" (nom.) APA-rated sheathing. Suitable edge support shall be provided by use of panel clips or blocking between framing unless otherwise noted. Fasten roof sheathing with 6d common nails spaced 6" o.c. at supported edges and 12" o.c. at intermediate supports.
- The use of heavily loaded dollyed carts or similar conveyances to transport building materials and/or debris can exceed the APA PS2 concentrated load test standard capacity. In areas subject to cart traffic, the contractor shall place a temporary second layer of 23/32" dry wood structural panel to help avoid failures of the floor panels. Refer to APA Technical Note TT-424, February 2008.
- Unless otherwise noted or shown, install plywood sheathing with the long dimension of the panel across supports and with panel continuous over two or more spans. Stagger panel end joints. Allow 1/8" spacing at panel ends and edges unless otherwise recommended by the sheathing manufacturer.
- All nailing shall be carefully driven and not overdriven. The use of staples and pneumatic nail guns are prohibited from use unless approved. Submit proposed staples and/or pneumatic fasteners for approval prior to construction.
- Provide 2x4 blocking at unsupported panel edges as follows:  
Roofs and floors - only where indicated on plan  
Walls - per the shearnail schedule.

ABBREVIATION LEGEND					
ABBR	DEFINITION	ABBR	DEFINITION	ABBR	DEFINITION
AB	ANCHOR BOLT	GA	GAGE (GAUGE)	PAF	POWDER-ACTUATED FASTENER
ABV	ABOVE	GALV	GALVANIZED	PARTN	PARTITION
ACI	AMERICAN CONCRETE INSTITUTE	GB	GRADE BEAM	PC	PRECAST CONCRETE
ACP	AUGERED CAST IN PLACE PILE	GC	GENERAL CONTRACTOR	PCF	POUNDS PER CUBIC FOOT
ADDL	ADDITIONAL	GLULAM	GLUE LAMINATED WOOD	PCI	PRECAST CONCRETE INSTITUTE
ADDM	ADDITIONAL	GRAN	GRANULAR	PDF	POWER-DRIVEN FASTENER
ADJ	ADJUSTABLE	GWB	GYPSPUM WALL BOARD	PERM	PERIMETER
AESS	ARCHITECTURALLY EXPOSED STRUCT. STL.			PL	PLATE
AFF	ABOVE FINISHED FLOOR			PLBG	PLUMBING
AGG	AGGREGATE	H	HIGH (HEIGHT)	PLF	POUNDS PER LINEAL FOOT
AISC	AMERICAN INSTITUTE OF STEEL CONSTR.N.	HAS	HEADED ANCHOR STUD	PLYWD	PLYWOOD
ASIS	AMERICAN IRON & STEEL INSTITUTE	HC	HOLLOW CORE	PNL	PANEL
ALT	ALTERNATE	HD	HOLD DOWN	PREFAB	PREFABRICATED
ANCH	ANCHORAGE	HK	HOOK	PROJ	PROJECTION
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE	HORIZ	HORIZONTAL	PSF	POUNDS PER SQUARE FOOT
APA	AMERICAN PLYWOOD ASSOCIATION	HP	HIGH POINT	PSI	POUNDS PER SQUARE INCH
APPROX	APPROXIMATE			PSL	PARALLEL STRAND LUMBER
ARCH	ARCHITECT (URAL)			PST	PRESSURE PRESERVATIVE TREATED
ASSY	ASSEMBLY	IBC	INTERNATIONAL (INDIANA) BUILDING CODE	PSC	PRESTRESSED CONCRETE
ASTM	AMERICAN SOCIETY FOR TESTING & MATLS.	ID	INSIDE DIAMETER	PT	POST TENSIONED
AVG	AVERAGE	IF	INSIDE FACE	PTD	PAINTED
AWS	AMERICAN WELDING SOCIETY	INCR	INCREASE	PRTN	PARTITION
		INFO	INFORMATION	PVMT	PAVEMENT
		INDOT	INDIANA DEPARTMENT OF TRANSPORTATION		
B/X	BOTTOM OF REFERENCED ITEM	INSUL	INSULATE (INSULATION)		
BB	BOND BEAM	INT	INTERIOR	QTR	QUARTER
BFF	BELOW FINISHED FLOOR	NV	INVERT	QTY	QUANTITY
BLDG	BUILDING	ISO	ISOLATION		
BLKG	BLOCKING	IT	INVERTED TEE BEAM		
BLW	BELOW			R	RADIUS
BM	BEAM			RB	RECTANGULAR BLOCK (PRECAST)
BOT	BOTTOM	JBE	JOIST BEARING ELEVATION	REF	REFER TO (REFERENCE)
BP	BASE PLATE	JST	JOIST	REQD	REQUIRED
BRD	BRIDGING	JT	JOINT	REV	REVISION (REVISED)
BRS	BEARING			RF	ROOF
BS	BOTH SIDES			RO	ROUGH OPENING
BTWN	BETWEEN	KP	1,000 POUNDS	RTU	ROOF TOP UNIT
		KO	KNOCK OUT	RTN	RETURN
		KSF	KIPS PER SQUARE FOOT	RW	RETAINING WALL
		KSI	KIPS PER SQUARE INCH		
CC	CENTER TO CENTER			SBCA	STRUCTURAL BUILDING COMPONENTS ASSN.
CAIS	CAISSON			SBCT	STRUCTURAL CLAY TILE
CAPY	CAPACITY	L	LONG (LENGTH)	SCHED	SCHEDULE
CANT	CANTILEVER	Ld	TENSION DEVELOPMENT LENGTH	SCHD	STEEL DECK INSTITUTE
CB	CONCRETE BEAM	Lb	"L" BEAM	SE	SLAB EDGE
CC	CONCRETE COLUMN	LBS	POUNDS	SER	STRUCTURAL ENGINEER OF RECORD
CFS	COLD-FORMED STEEL	LOSF	LIGHT GAUGE STEEL FRAMING	SECT	SECTION
CF	CAST IN PLACE	LOW	LIVE LOAD	SHT	SHEET
CJ	CONTROL JOINT	LLH	LONG-LEG HORIZONTAL	SHM	SHIMLAR
CNJ	CONSTRUCTION JOINT	LLS	LONG-LEG OUTSTANDING	SJ	STEEL JOIST INSTITUTE
CL	CENTERLINE	LLV	LONG-LEG VERTICAL	SLL	SLOPE
CLR	CLEAR(ANCE)	LNTL	UNTEL	SOG	SLAB ON GRADE
COL	CONCRETE MASONRY UNIT	LONG	LONGITUDINAL	SPA	SPACE (SPACING)
COL	COLUMN	LONG	LONG	SPECS	SPECIFICATIONS
COLL	COLLATERAL	LP	LOW POINT		
CONC	CONCRETE	LVL	LAMINATED VENEER LUMBER		
CONSTR	CONSTRUCTION	LW	LONG WAY	SQ	SQUARE
CONT	CONTINUOUS	LWC	LIGHTWEIGHT CONCRETE	SS	STAINLESS STEEL
CRSI	CONCRETE REINFORCING STEEL INSTITUTE			SSE	SPECIALTY STRUCTURAL ENGINEER
CTR	CENTER			STD	STANDARD
CTRD	CENTERED	MATL	MATERIAL	STIFF	STIFFENER
CW	CONCRETE WALL	MAX	MAXIMUM	STL	STEEL
		MC	MOMENT CONNECTION	STR	STRENGTH
		MECH	MECHANICAL	STRUCT	STRUCTURAL
D	DEEP (DEPTH)	MEZZ	MEZZANINE	SW	SHORT WAY
DB	DEFORMED BAR ANCHOR	MFR	MANUFACTURER	SW	SHEAR WALL (OCCASIONAL)
DEG	DEGREE	MIN	MINIMUM	SYMM	SYMMETRICAL
DIA	DIAMETER	MISC	MISCELLANEOUS		
DAG	DIAGONAL	MO	MASONRY OPENING		
DM	DIMENSION	MM	MOMENT		
D&D	DEAD LOAD	MTL	METAL	TIX	TOP OF REFERENCED ITEM
DN	DOWN			T&B	TOP AND BOTTOM
DP	DRILLED PIER			T&G	TONGUE & GROOVE
DT	DOUBLE TEE	NDS	NATIONAL DESIGN SPECN. FOR WOOD	TB	TIE BEAM
DTL	DETAIL	NIC	NOT IN CONTRACT	TD	TRENCH DRAIN
DWG	DRAWING	NO	NUMBER	TEMP	TEMPERATURE
DWL	DOWEL	NOM	NOMINAL	TF	TRENCH FOOTING
		NRC	NOISE REDUCTION COEFFICIENT	THK	THICKNESS
		NS	NEAR SIDE	TOPG	TOPPING
EA	EACH	NTS	NOT TO SCALE	TP1	TRUSS PLATE INSTITUTE
ECC	ECCENTRIC	NWC	NORMAL WEIGHT CONCRETE	TRANSV	TRANSVERSE
EF	EACH FACE			TYP	TYPICAL
EIFS	EXTERIOR INSULATION & FINISH SYSTEM				
EJ	EXPANSION JOINT	O/O	OUT TO OUT	UNO	UNLESS NOTED OTHERWISE
EL	ELEVATION	OA	OVERALL	UNEXC	UNEXCAVATED
ELEC	ELECTRICAL	OC	ON CENTER		
ELEV	ELEVATOR	OD	OUTSIDE DIAMETER		
ENG	ENGINEER	OF	OUTSIDE FACE	VERT	VERTICAL
EQ	EQUAL	OH	OPPOSITE HAND		
EQ SPA	EQUALLY SPACED (EQUAL SPACING)	OPNG	OPENING		
EQUIV	EQUIVALENT	OPP	OPPOSITE	W	WIDE (WIDTH)
ES	EACH SIDE	OSB	ORIENTED STRAND BOARD	WI	WITH
EW	EACH WAY			WD	WOOD
EX	EXISTING			WF	WALL FOOTING
EXC	EXCAVATE (EXCAVATION)			WP	WORKING POINT
EXT	EXTERIOR			WPS	WELD PROCEDURE SPECIFICATION
				WRDA	WATER REDUCING ADMIXTURE
				WTCA	WOOD TRUSS COUNCIL OF AMERICA
				WWF	WELDED WIRE FABRIC
FABR	FABRICATE (FABRICATOR)				
FD	FLOOR DRAIN				
FDN	FOUNDATION				
FIN	FINISH				
FF	FINISHED FLOOR				
FLG	FLANGE				
FS	FAR SIDE				
FT	FOOT (FEET)				
FTG	FOOTING				
FV	FIELD VERIFY				





1 FOUNDATION PLAN - CLARKS CREEK  
S201 SCALE: 1/8" = 1'-0"

3 ROOF FRAMING PLAN - CLARKS CREEK  
S201 SCALE: 1/8" = 1'-0"

2 FOUNDATION PLAN - BRENTWOOD  
S201 SCALE: 1/8" = 1'-0"

4 ROOF FRAMING PLAN - BRENTWOOD

### FOUNDATION PLAN NOTES

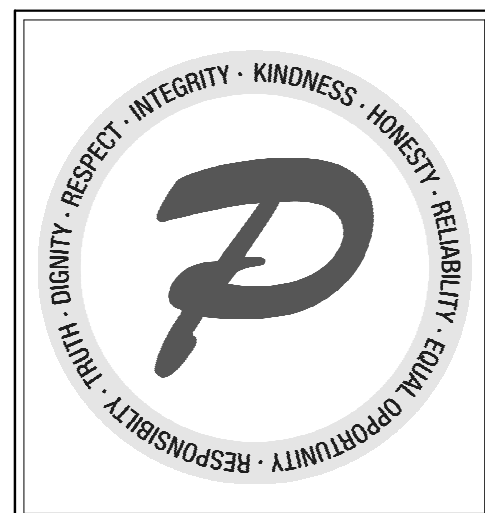
- [illegible]

FF	DENOTES FIN FLOOR
TT	DENOTES TOP OF F/G. SLAB, PER. ETC.
BT	DENOTES BOTTOM OF F/G. SLAB, BEAM, ETC.
C/J	DENOTES SLAB ON GRADE
	CONTROL/CONTRACTION JOINT
TF24 -1'-4"	DENOTES TRENCH FOOTING MARK & TOP OF FOOTING ELEVATION (REF. WALL SCHEDULE)
	DENOTES WALL FOOTING WITH STEPS
	REF. TYP. DETAIL ON 401
	DENOTES CMU FOUNDATION WALL
	DENOTES CMU FDN. WALL HOLD DOWN AT OPENINGS
5" CONC. S/DG TISLAB +/-0"	DENOTES SLAB ON GRADE THICKNESS & TISLAB ELEVATION. ALL SLABS ON GRADE TO BE PLACED ON 6" MIN. COMPACTED GRANULAR FILL & VAPOR BARRIER/STANDARD PER BRCS. PROVIDE THE FOLLOWING WELDED WIRE REINFORCING:
	5" SLAB: 6x6-W#2-1' W/WF

### FRAMING PLAN NOTES

1. REF. S0014 & S002 FOR STRUCTURAL NOTES, DESIGN DATA, SCHEDULES & LEGENDS
2. REF. THE S400 SERIES FOR TYPICAL FRAMING AND MASONRY DETAILS
3. ALL CONTRACTORS ARE REQUIRED TO COORDINATE THEIR WORK WITH ALL DISCIPLINES TO AVOID CONFLICTS. THE MECHANICAL, ELECTRICAL, AND PLUMBING EFFECTS ARE NOT IN THE SCOPE OF THESE DRAWINGS.
4. THEREFORE, ALL REQUIRED MATERIALS AND WORK SHALL NOT BE NOTICED.
5. ALL ELEVATIONS ARE REFERENCED FROM THE FIRST FLOOR FIN FLOOR ELEVATION +0'-0". VERIFY UGS ELEVATION WITH CIVIL DWGS.
6. ALL WALLS SHALL BE LAID OUT FROM THE ARCHITECTURAL DRAWINGS.
7. REF. ARCH. DRAWINGS FOR ALL DIMENSIONS NOT SHOWN. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CORRECT ANY DISCREPANCIES IMMEDIATELY NOTIFY ARCHITECT ENGINEER OF ANY DISCREPANCIES.
8. COORDINATE EXACT SIZE & LOCATION OF ANY MECHANICAL OPENINGS IN FLOOR SLAB, ROOF DECK, OR WALLS WITH THE MEYER CONTRACTOR(S).
9. ALL EXISTING OPENINGS IN FLOOR SLABS, GRILLES, ETC. SHALL BE VERIFIED PRIOR TO CONSTRUCTION.
10. ALL ELEVATIONS SHOWN ON PLAN INDICATE TOP OF STEEL BEAM UNLESS NOTED OTHERWISE.
11. PROVIDE REINFORCING AS NOTED ON PLANS. IF NOT SHOWN ON PLANS OR DETAILS, PROVIDE MINIMUM CMU WALL REINFORCING TO BE 1#5 VERTS @ 48" O.C. PROVIDE OPEN-CORE BOND BEAMS AT TOPS OF WALLS, AT CHANGES IN CMU THICKNESS, AND WHERE INDICATED ON PLANS & SECTIONS 10" O.C. MAX. COARSE SPACING. PROVIDE REINFORCING AT JOINTS & JAMBS OF OPENINGS AND PROVIDE ADDITIONAL VERTS AT ENDS OF WALLS.
12. ALL MASONRY BOND BEAMS, OTHER THAN BOND BEAM UNITS/LOVS OVER OPENINGS, SHALL BE "OPEN-CORE" BOND BEAMS & ALLOW VERTICAL REINFORCING TO PASS THROUGH. PROVIDE REINFORCING AS NOTED.
13. REF. ARCH. DWGS. FOR MASONRY CONTROL & EXPANSION JOINT LOCATIONS.
14. PLAN LEGEND:  

FF	DENOTES FIN FLOOR
T/FX	DENOTES TOP OF STEEL, SLAB, ETC.
U/FX	DENOTES TOP OF UNITS, ETC.
BP	DENOTES BEAM BEARING PLATE ON CMU WALL



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**B** Lynch, Harrison & Brumleve, Inc.

550 Virginia Avenue  
Indianapolis, IN 46203

P 317.423.1550  
F 317.423.1551

**STRUCTURAL ENGINEERS**

STRUCTURAL ENGINEERS

PROJECT: PLAINFIELD COMMUNITY  
SCHOOL CORPORATION  
PLAINFIELD OUTBUILDINGS  
PLAINFIELD, IN

**SCOPE DRAWINGS:**  
These drawings indicate the general scope of the project in terms of architectural design concept, the dimensions of the building, the major architectural elements and the type of structural, mechanical and electrical systems.  
The drawings do not necessarily indicate or describe all work required for full performance and completion of the requirements of the Contract.  
On the basis of the general scope indicated or described, the trade contractors shall furnish all items required for the


**REVISIONS:**

ISSUE DATE	DRAWN BY	CHECKED BY
05/08/2025	MDL	MDL

DRAWING TITLE:

FOUNDATION  
AND ROOF  
FRAMING PLANS

CERTIFIED BY:

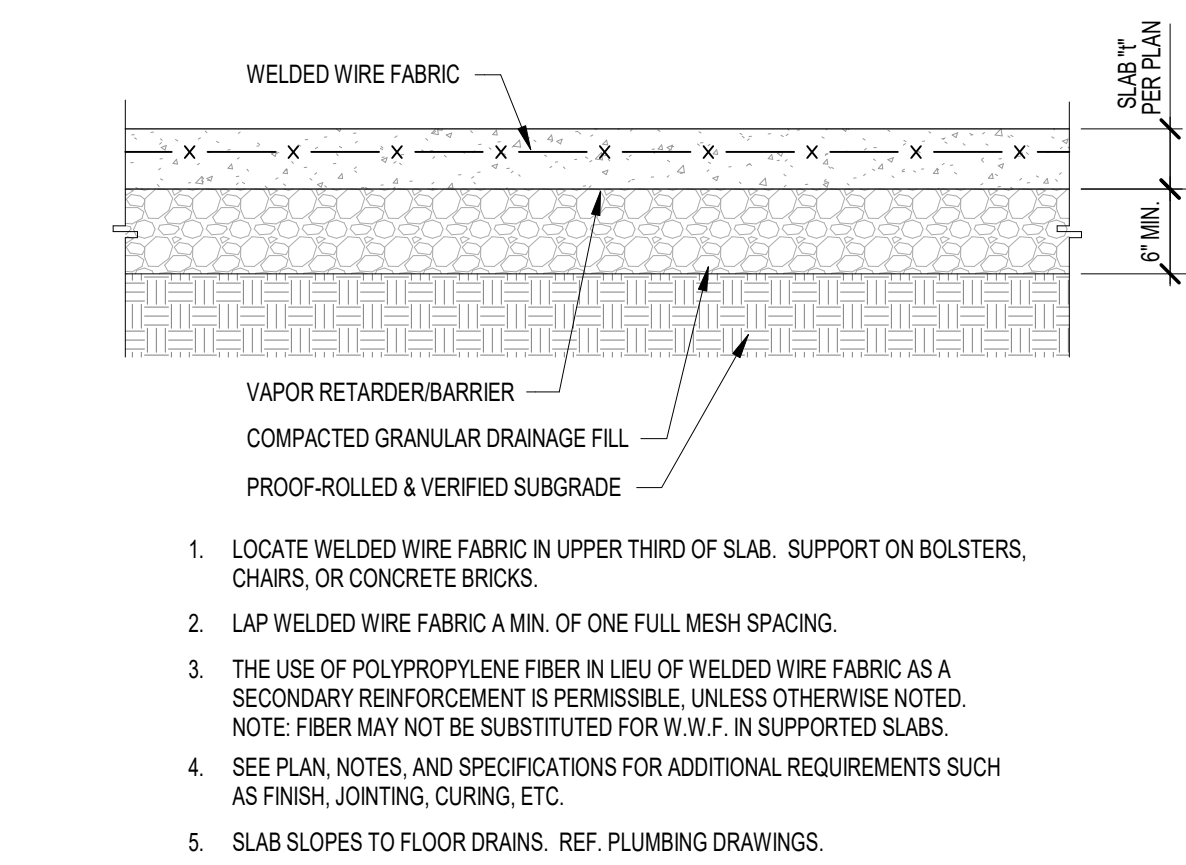


MARK D. LAVIER  
REGISTERED  
PE19400114  
STATE OF  
INDIANA  
PROFESSIONAL ENGINEER

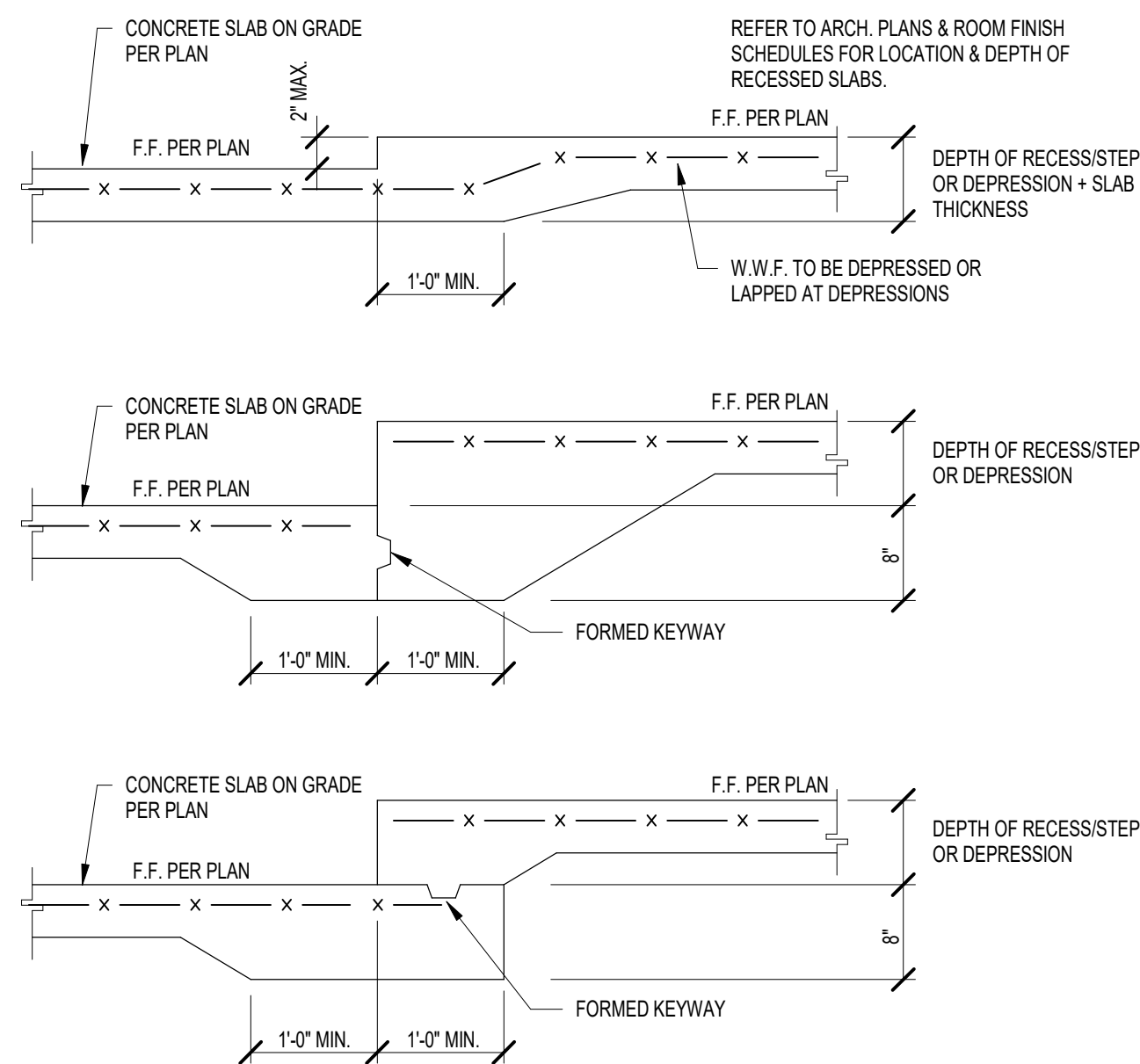
DRAWING NUMBER  
**S201**

PROJECT NUMBER  
2025016

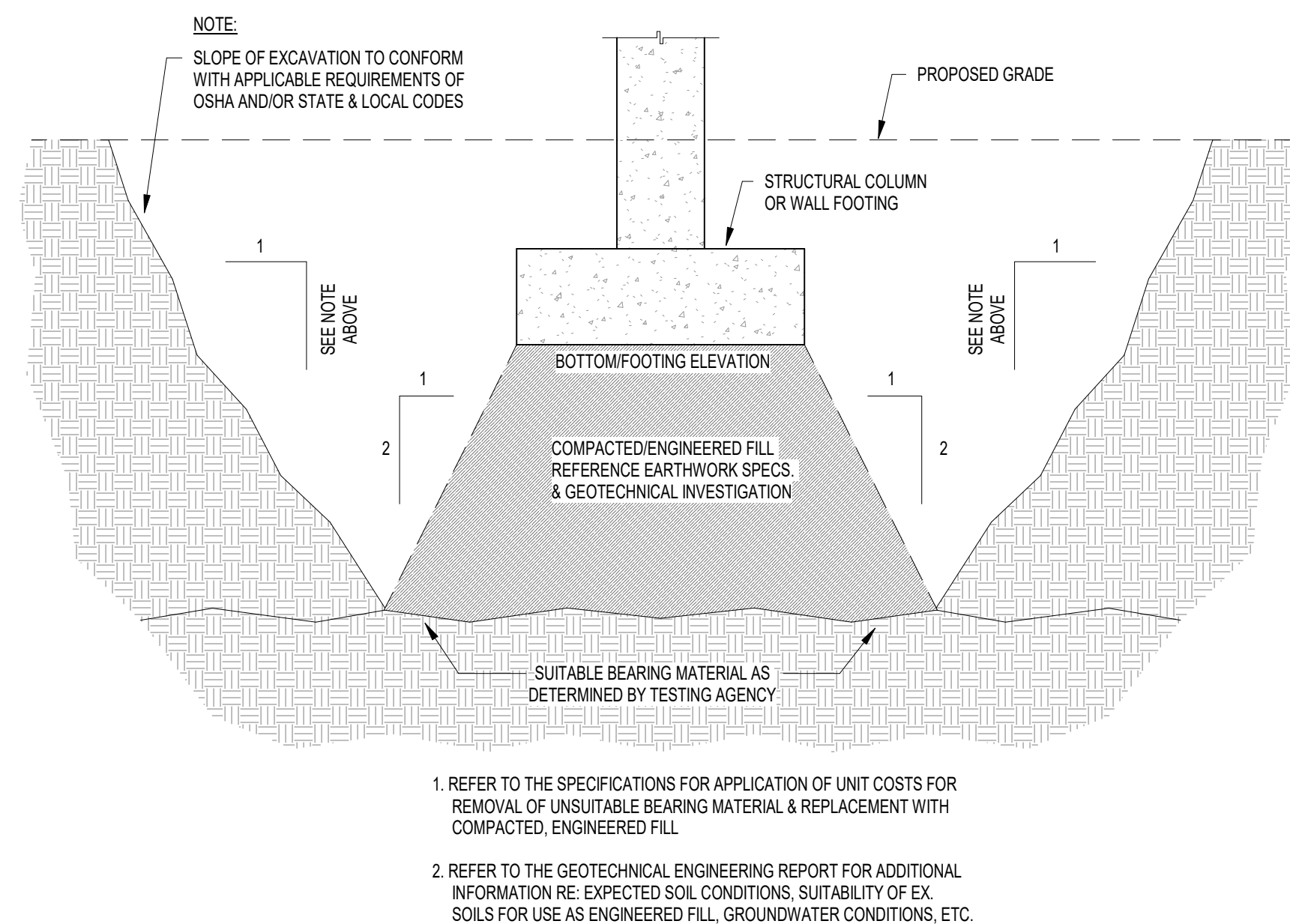




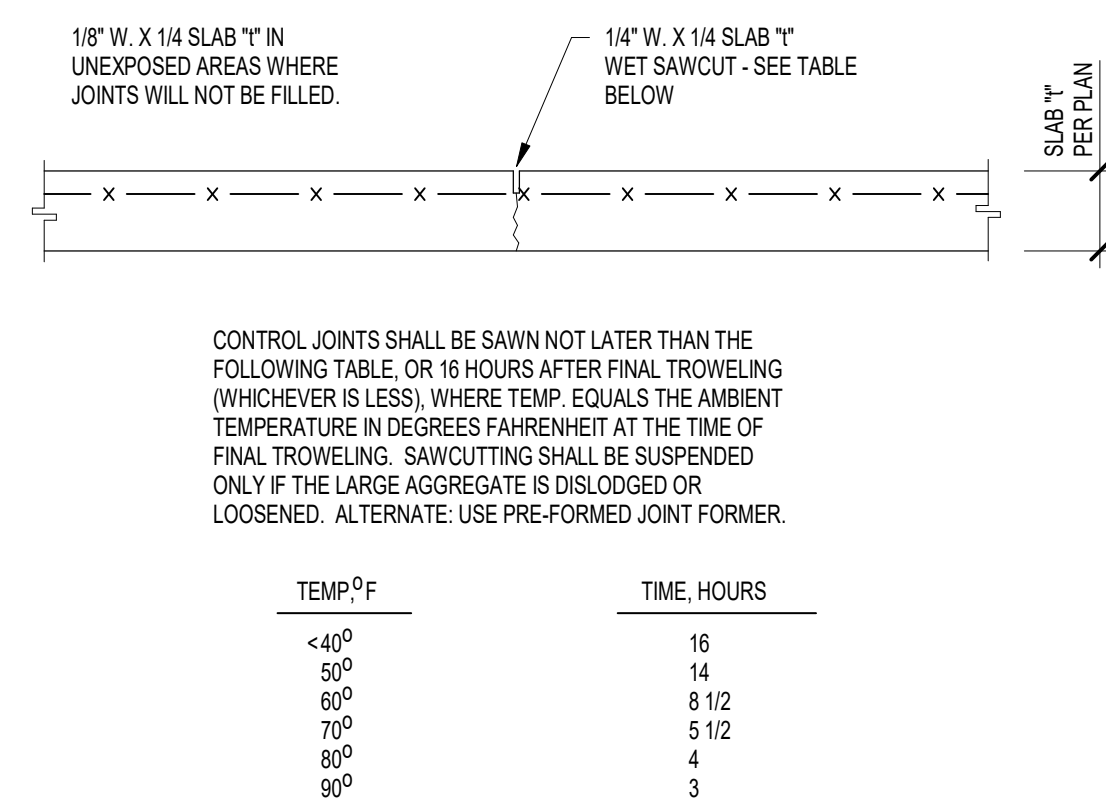
1 SLAB ON GRADE CONSTRUCTION  
S401 SCALE: NONE



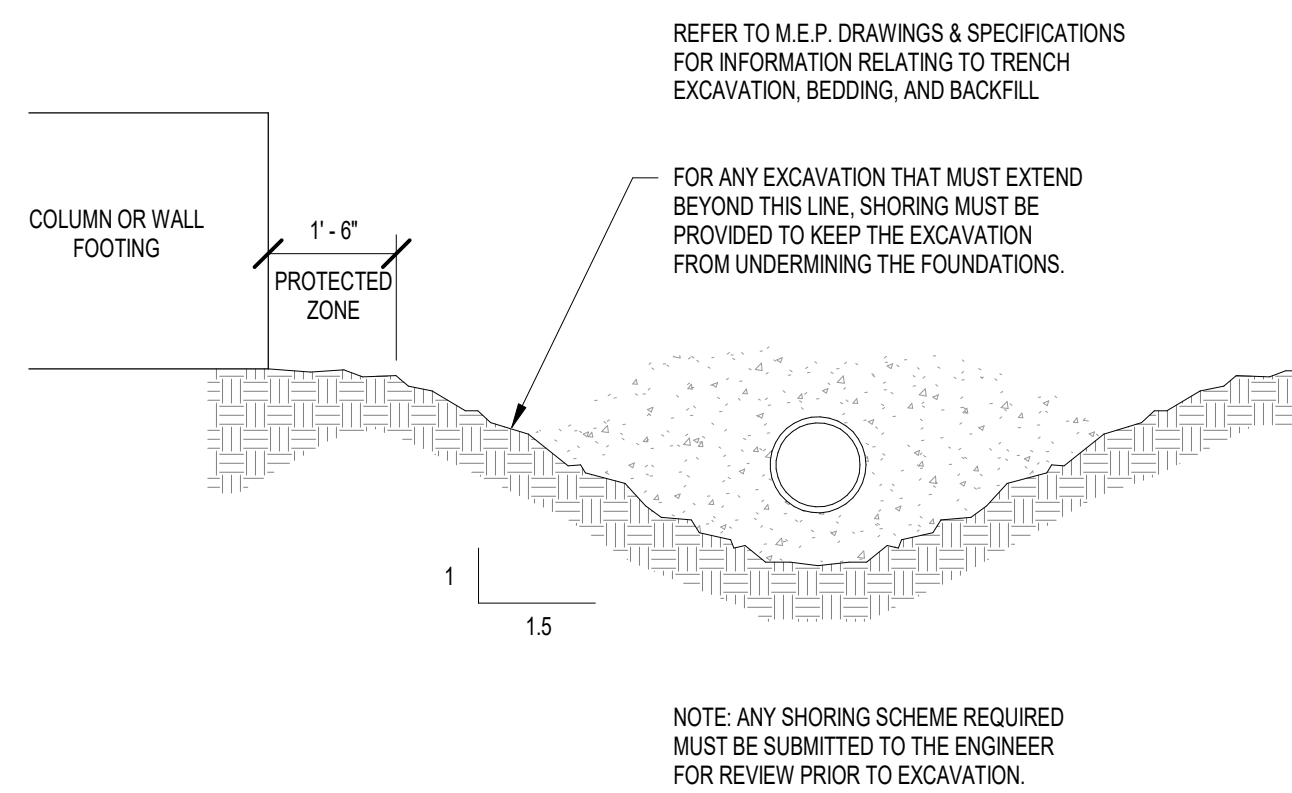
6 RECESSED SLAB DETAIL  
S401 SCALE: NONE



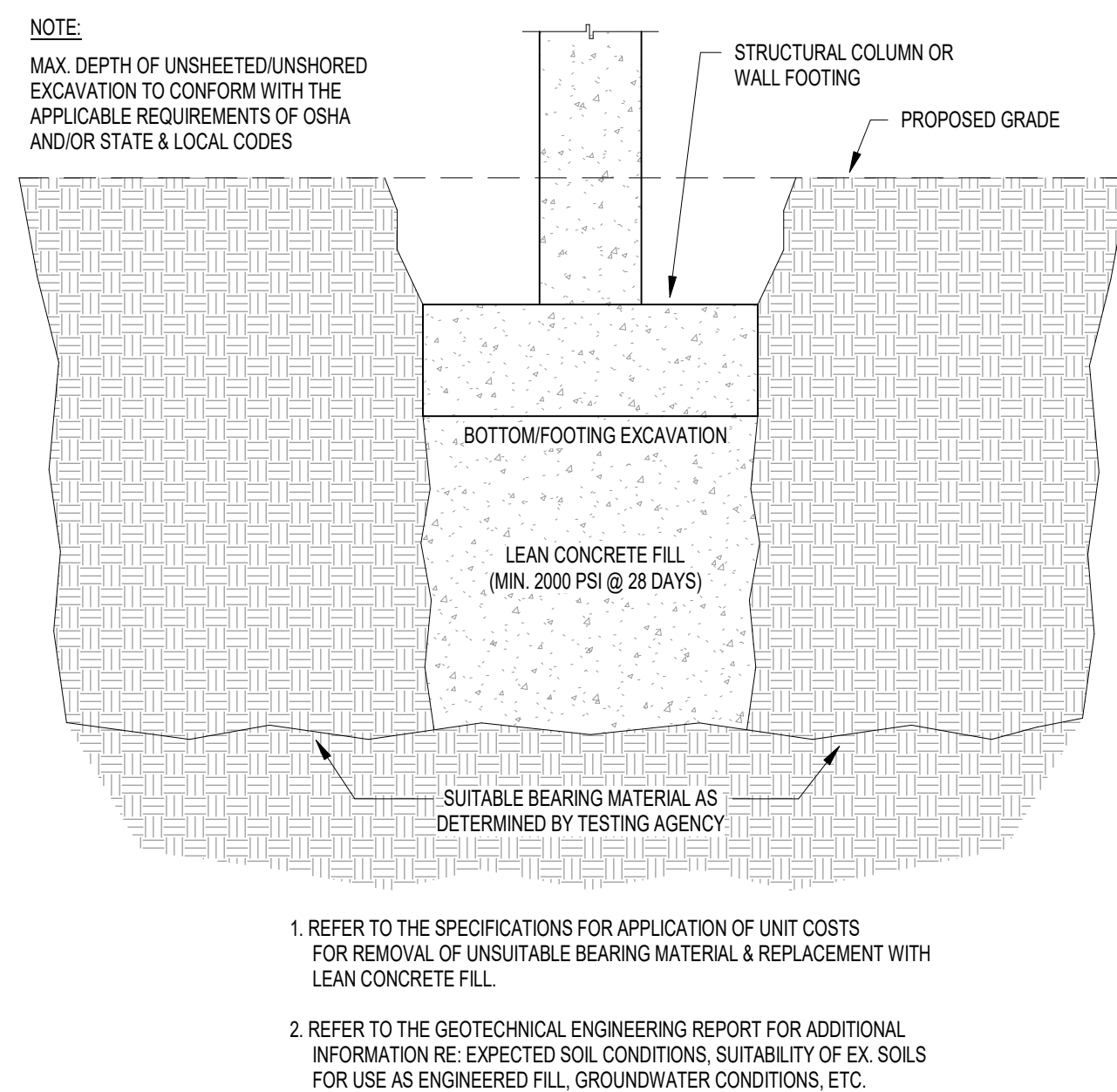
12 OVEREXCAVATION DETAIL - ENGINEERED FILL  
S401 SCALE: 3/4\"/>



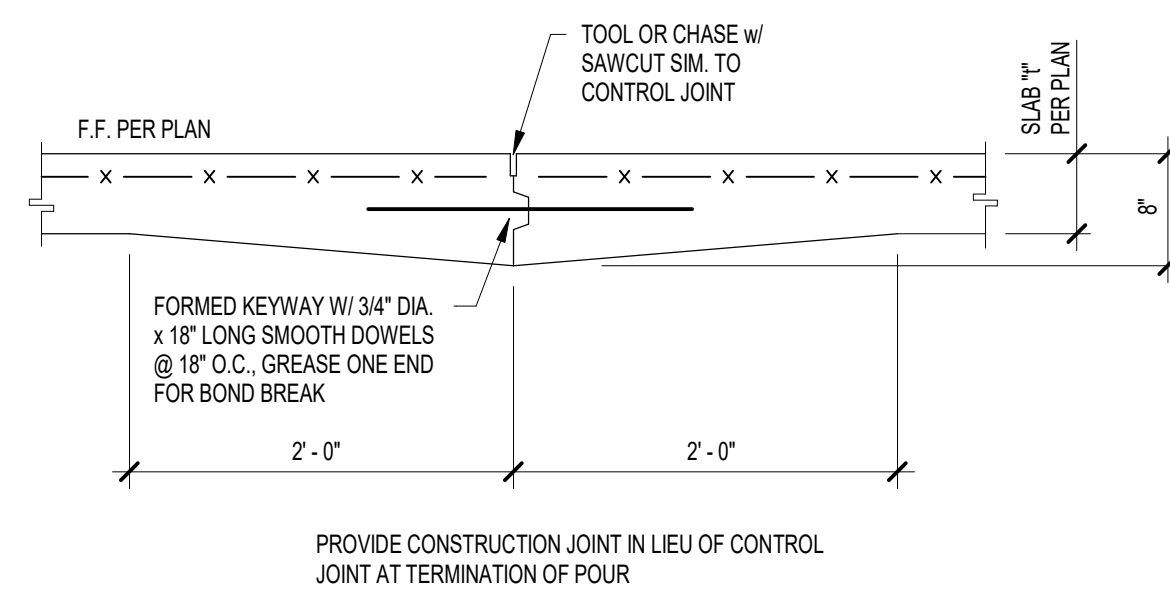
2 SLAB CONTROL/CONTRACTION JOINT  
S401 SCALE: NONE



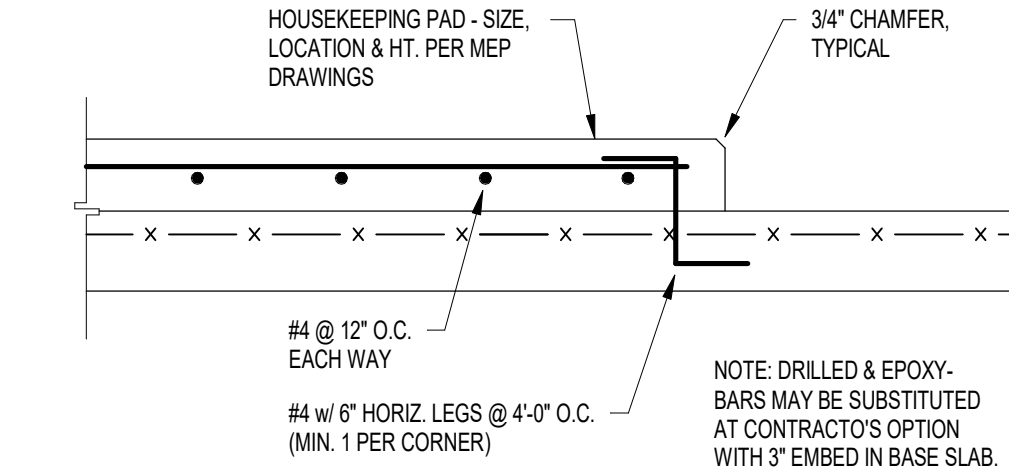
7 EXCAVATION LIMITS DETAIL  
S401 SCALE: NONE



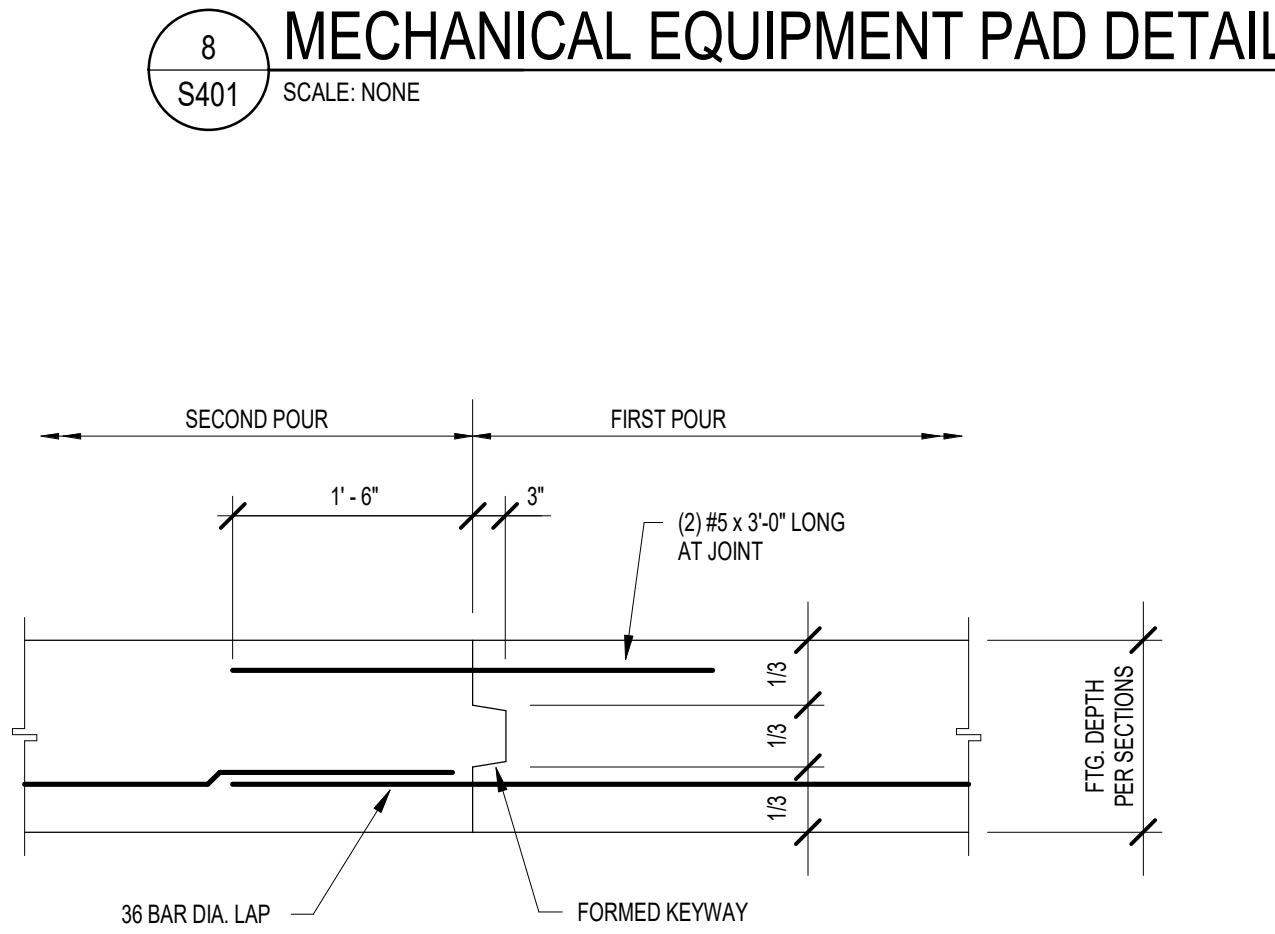
13 OVEREXCAVATION DETAIL - LEAN CONC. FILL  
S401 SCALE: 3/4\"/>



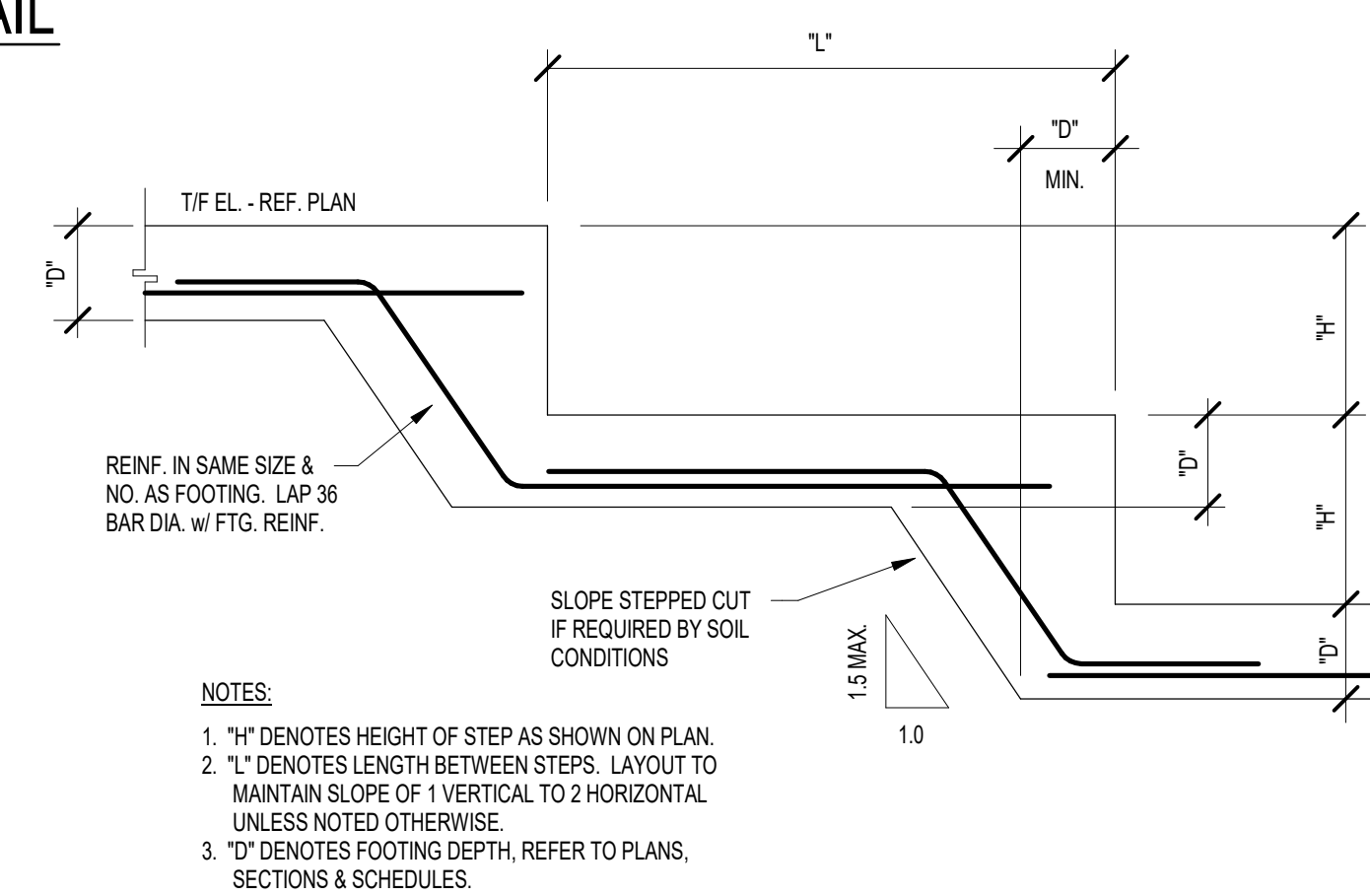
3 SLAB CONSTRUCTION JOINT  
S401 SCALE: NONE



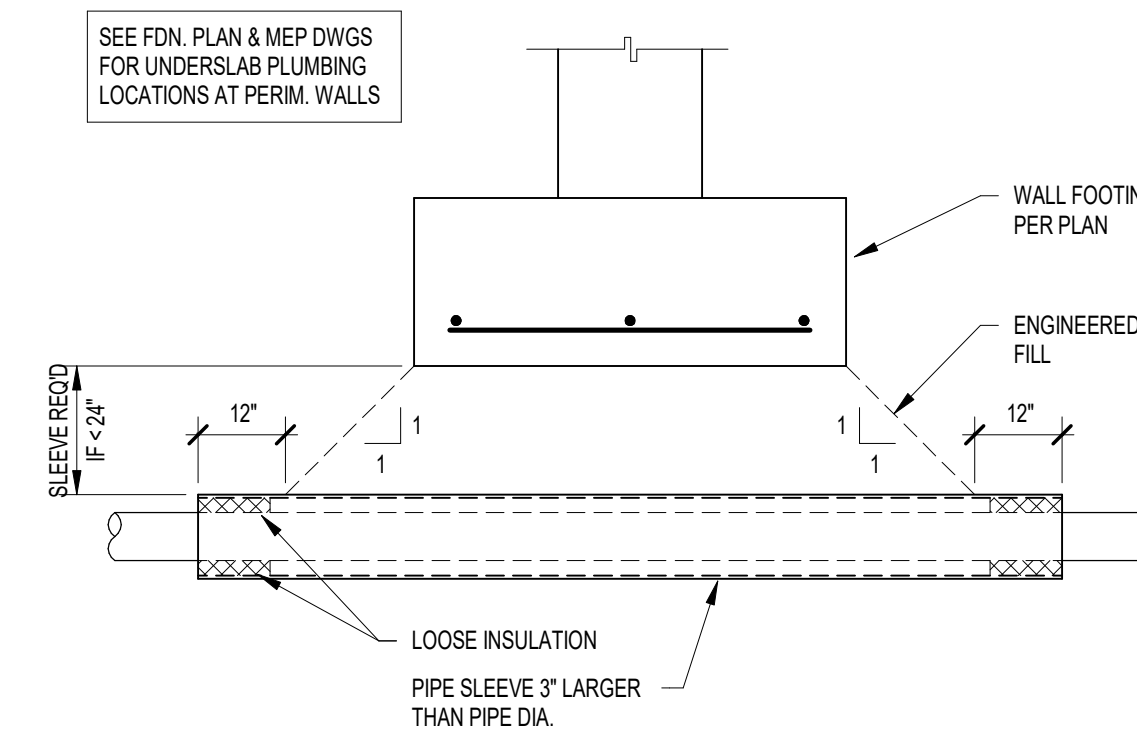
4 THICKENED SLAB DETAILS  
S401 SCALE: NONE



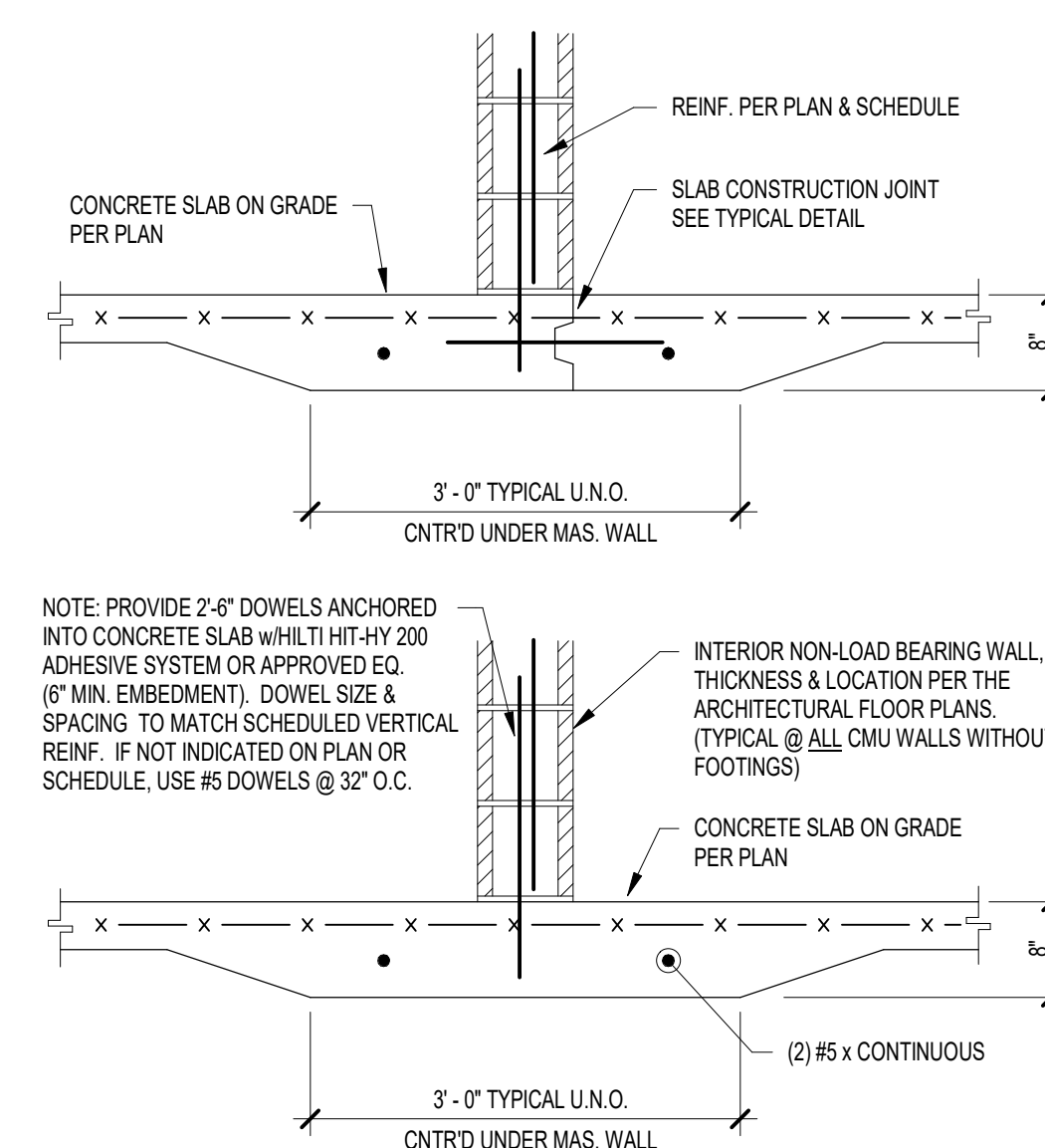
8 MECHANICAL EQUIPMENT PAD DETAIL  
S401 SCALE: NONE



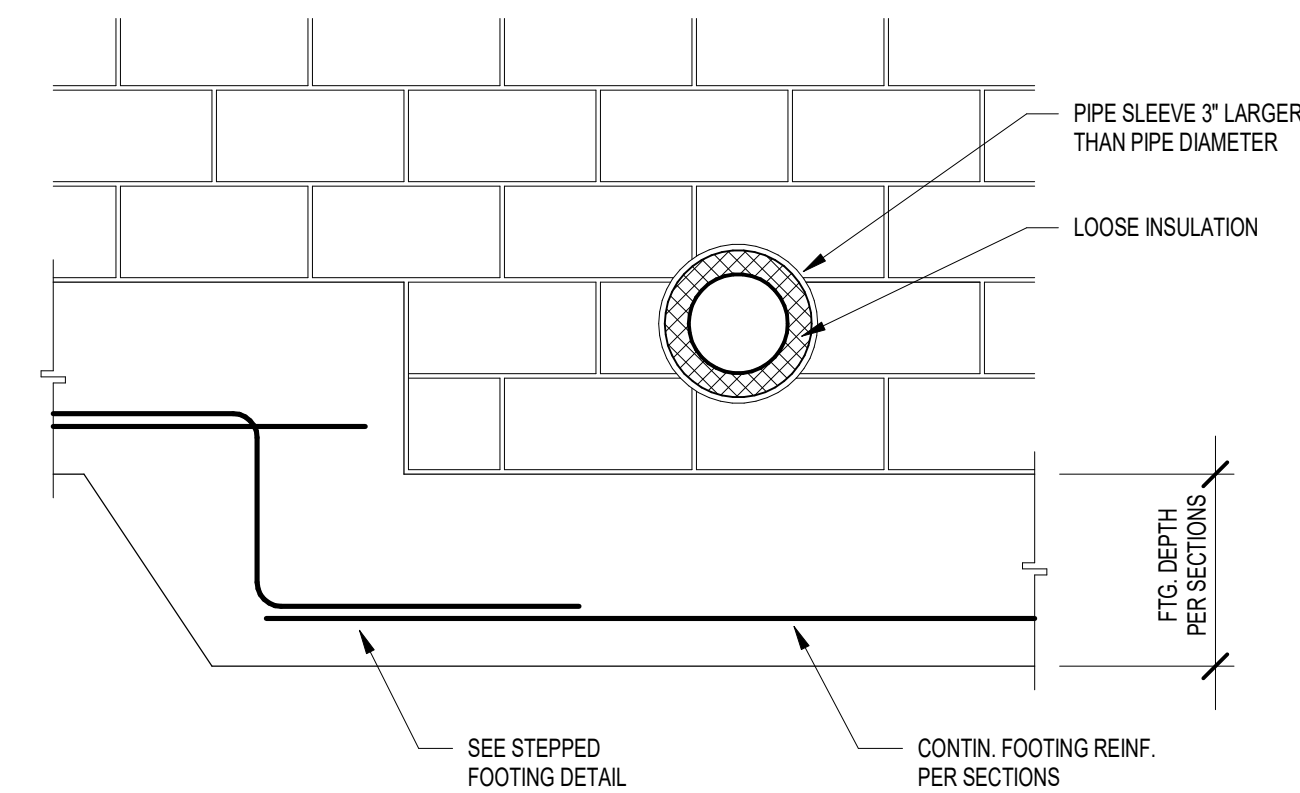
10 STEPPED WALL FOOTING DETAIL  
S401 SCALE: NONE



14 SLEEVE UNDER WALL FOOTING  
S401 SCALE: 3/4\"/>



5 WALL FOOTING SLEEVE DETAIL  
S401 SCALE: NONE



11 CMU FOUNDATION WALL SLEEVE  
S401 SCALE: NONE



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Indianapolis, IN 46203  
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STRUCTURAL ENGINEERS

PROJECT:  
**PLAINFIELD COMMUNITY SCHOOL CORPORATION**  
**PLAINFIELD OUTBUILDINGS**  
PLAINFIELD, IN

SCOPE DRAWINGS:  
These drawings indicate the general scope of the project in terms of mechanical design concept, the arrangement of structural, mechanical and electrical systems. The drawings do not necessarily indicate or describe all work required for full performance and completion of the requirements of the Contract.  
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REVISIONS:

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CHECKED BY MDL

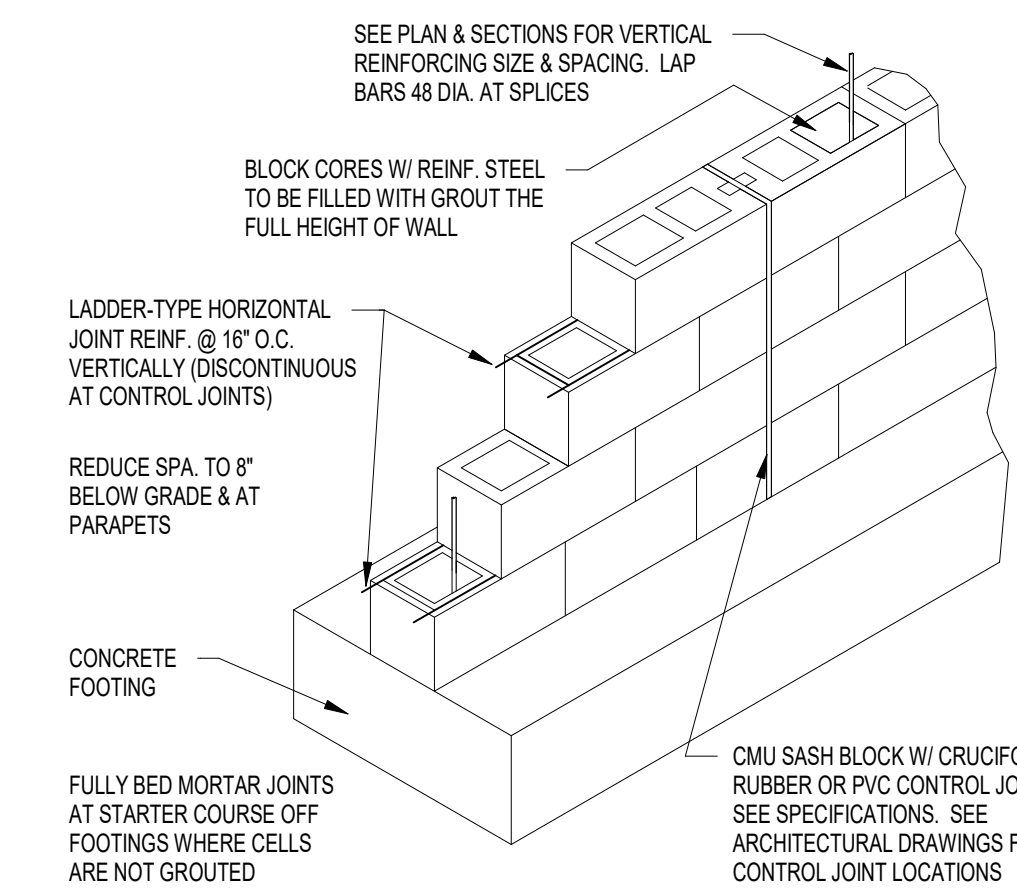
DRAWING TITLE:  
**TYPICAL DETAILS**

CERTIFIED BY:  
MAX D. L. L. L.  
REGISTERED  
PE19400114  
STATE OF INDIANA  
PROFESSIONAL ENGINEER

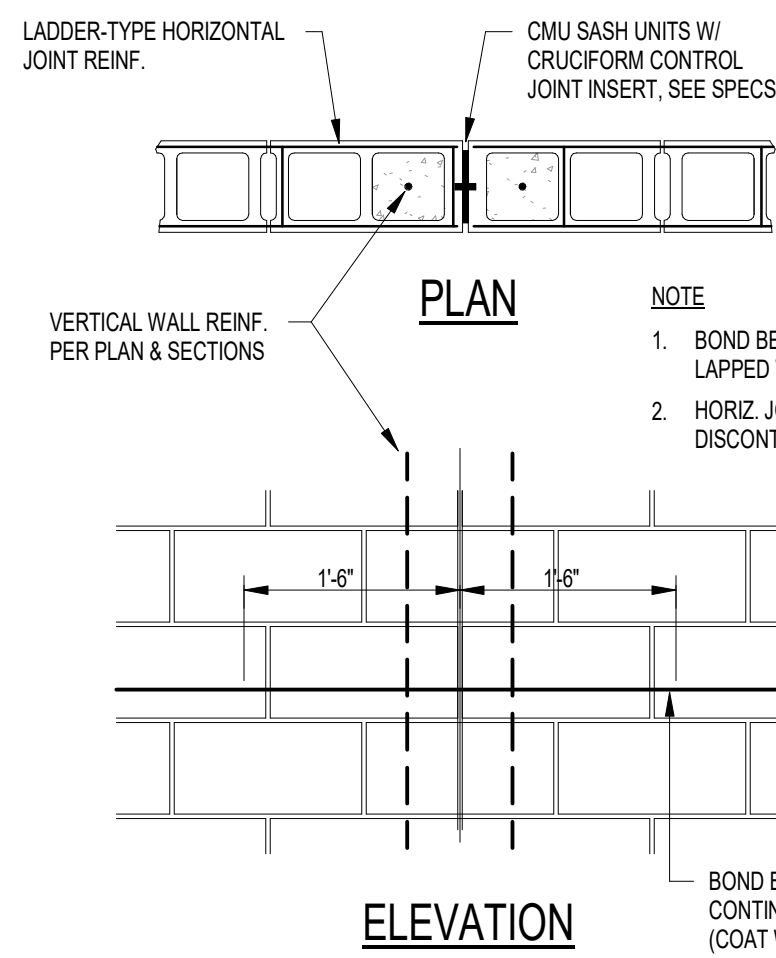
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**S401**

PROJECT NUMBER  
**2025016**

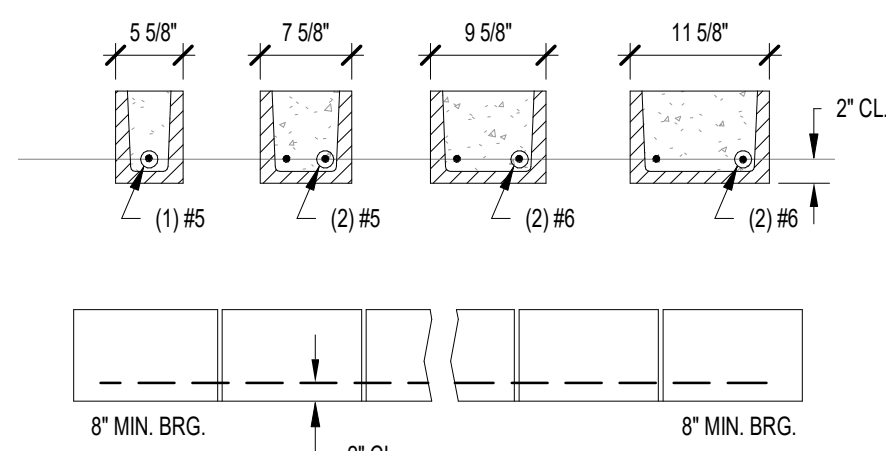




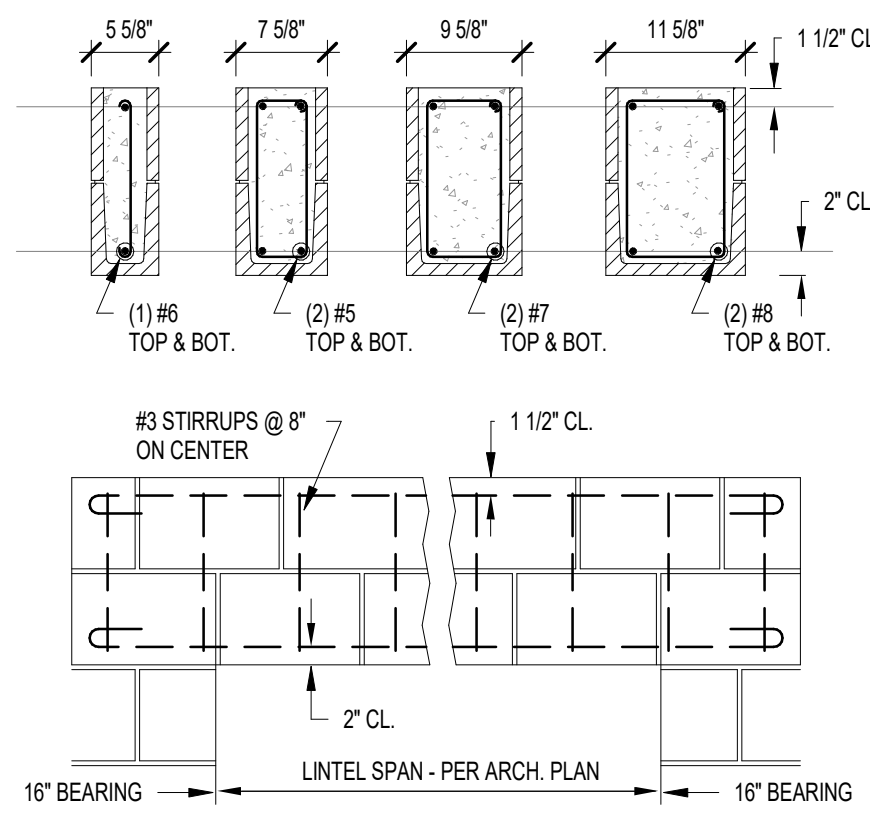
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S402 REINFORCED MASONRY DETAIL  
SCALE: NONE



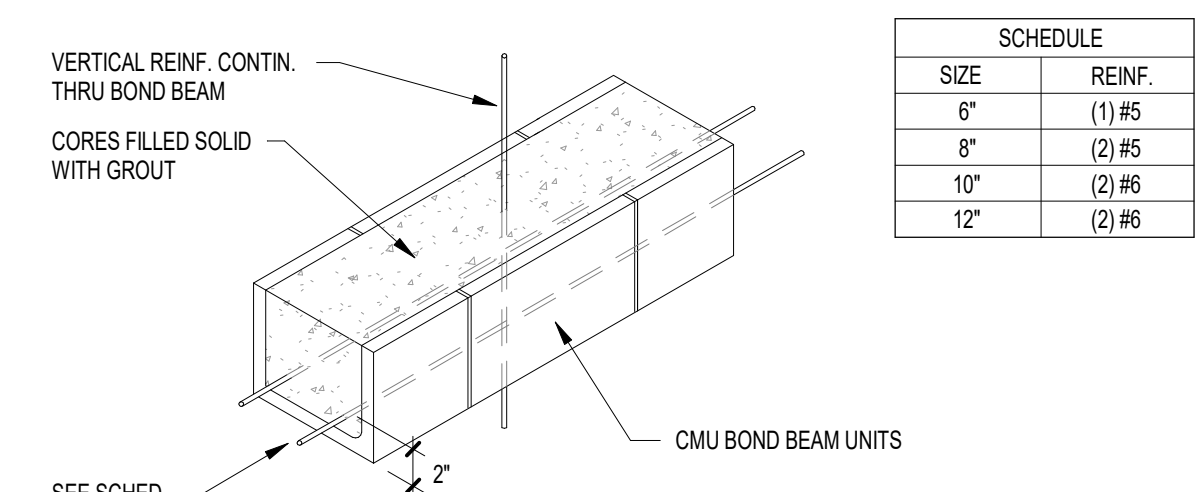
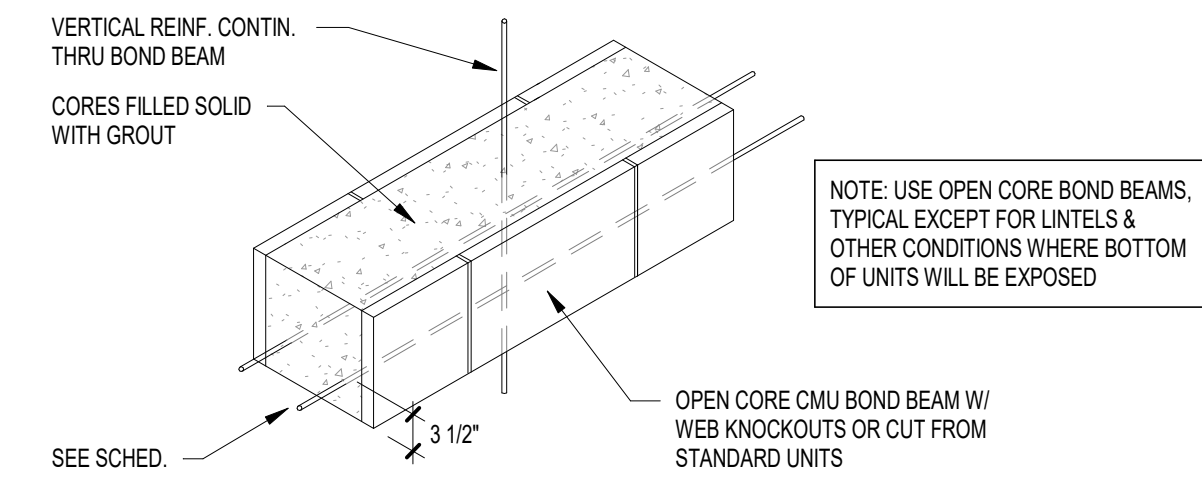
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S402 CMU CONTROL JOINT DETAIL  
SCALE: NONE



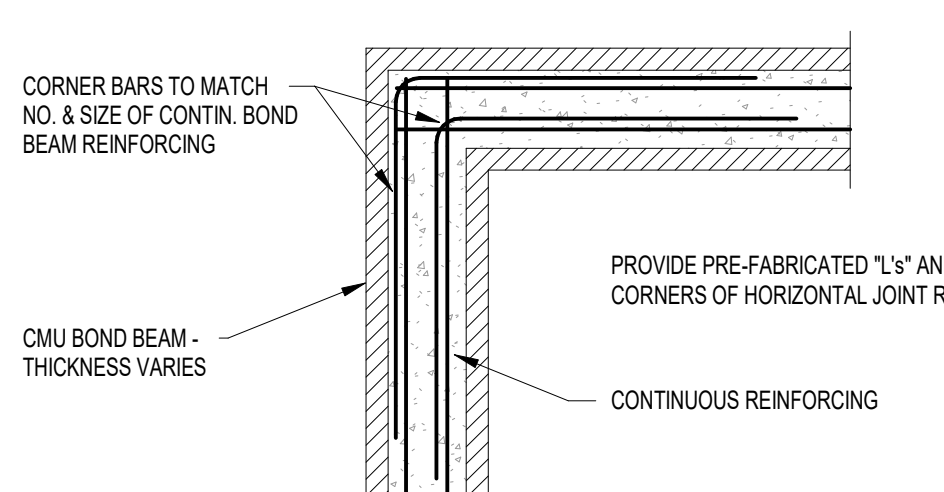
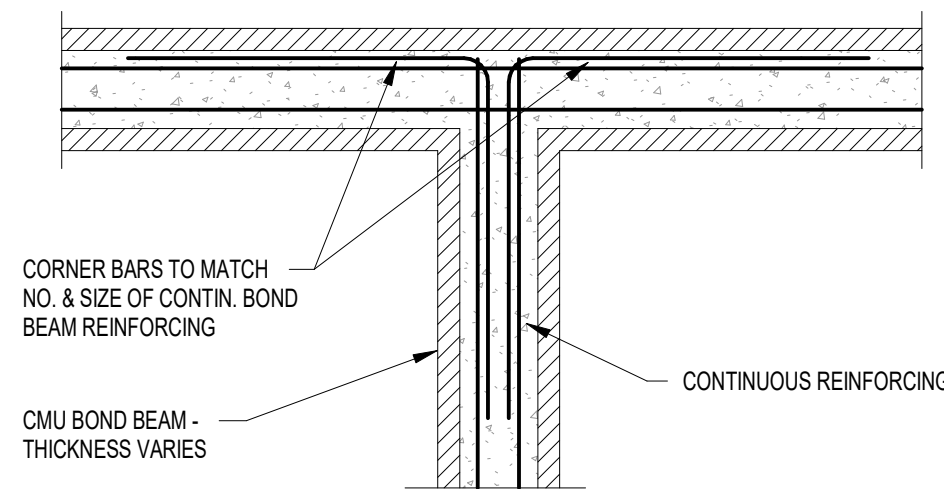
3  
S402 TYPICAL CMU LINTEL DETAILS  
SCALE: NONE



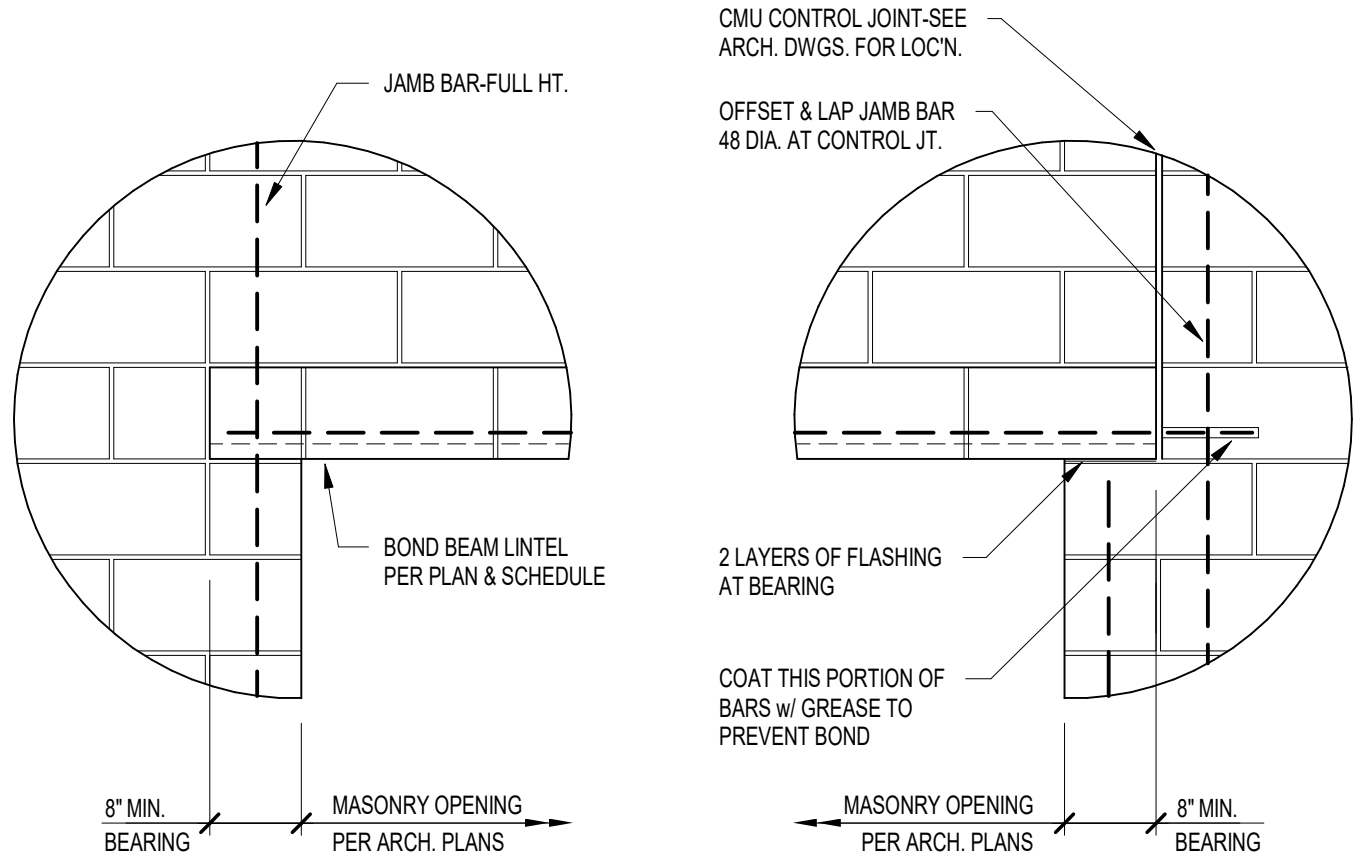
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S402 LONG CMU LINTEL DETAILS  
SCALE: NONE



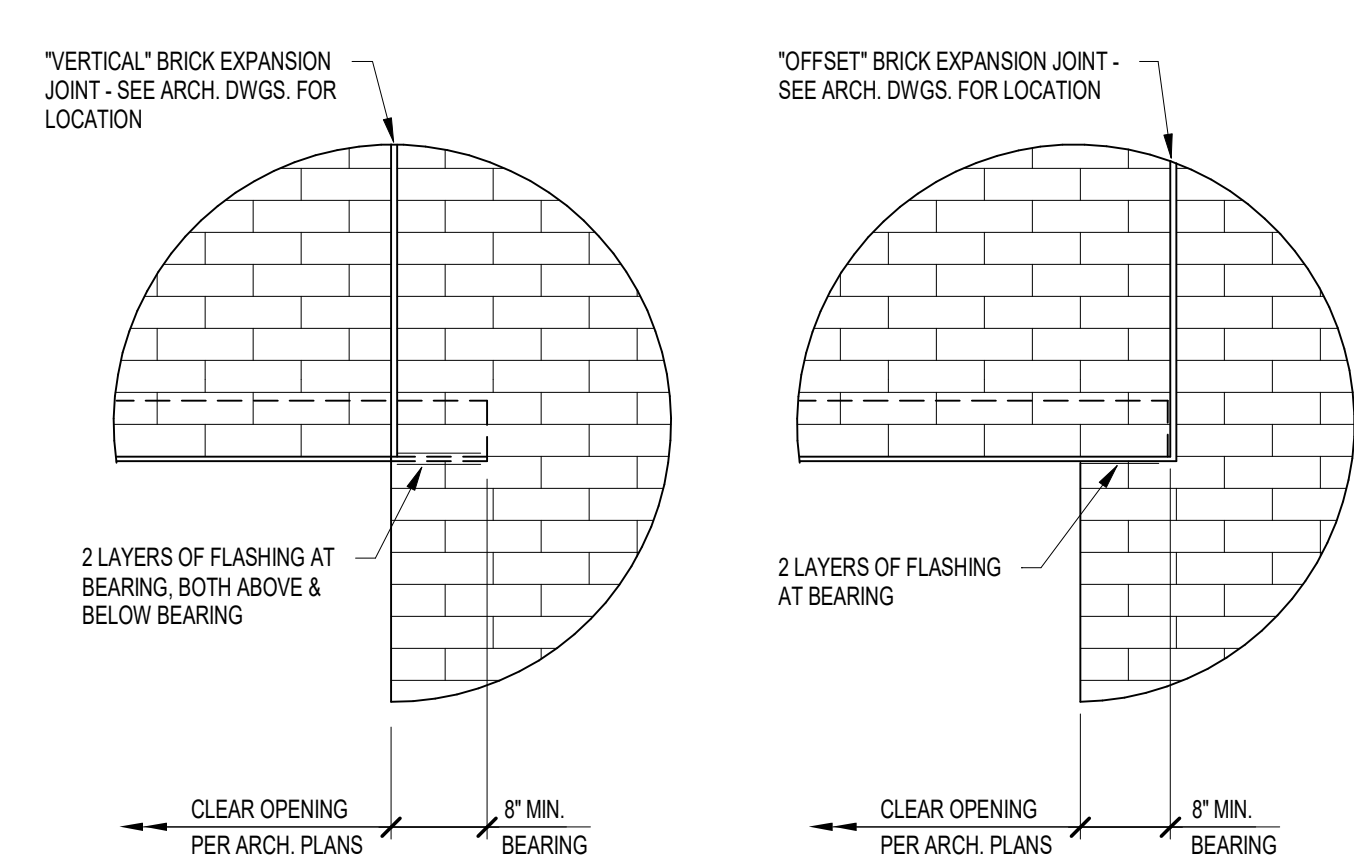
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S402 CMU BOND BEAM DETAILS  
SCALE: NONE



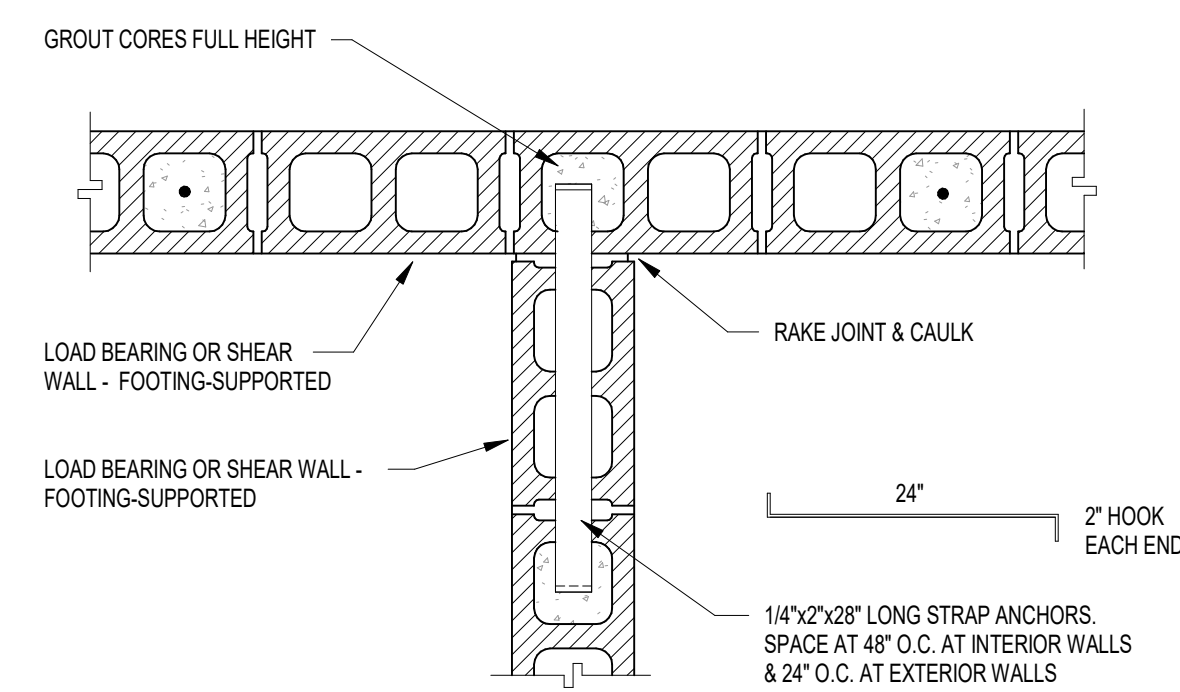
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S402 BOND BEAM INTERSECTION DETAILS  
SCALE: NONE



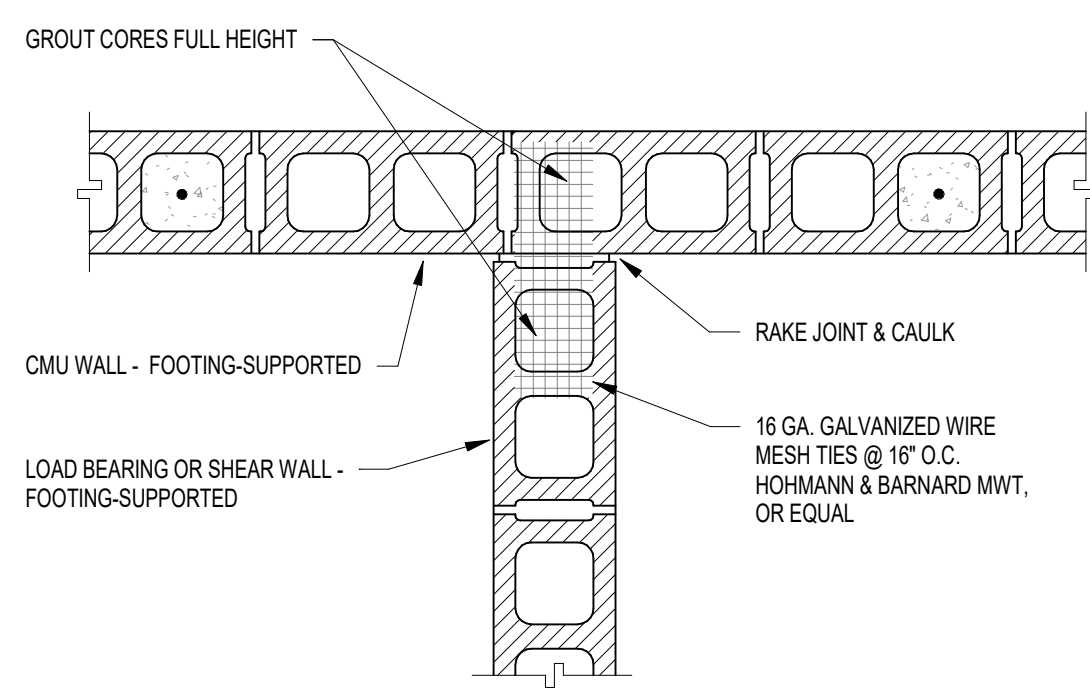
7  
S402 BOND BEAM LINTEL BEARING DETAILS  
SCALE: NONE



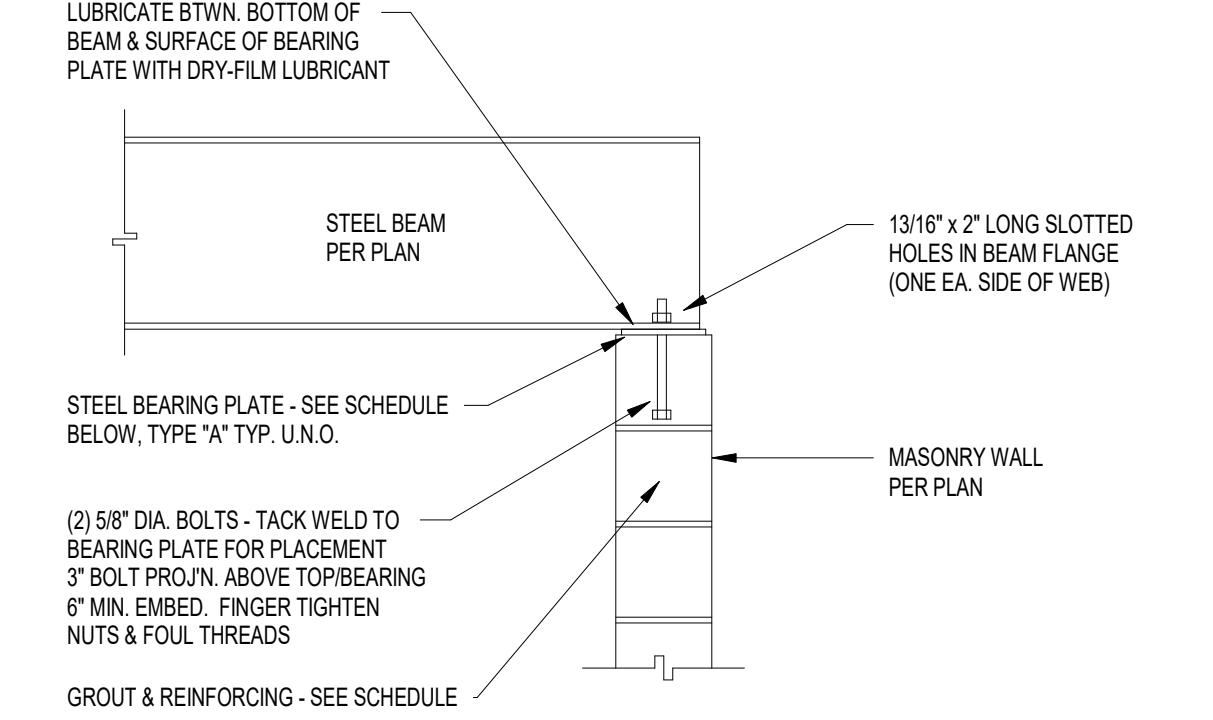
8  
S402 LINTEL BEARING DETAILS  
SCALE: NONE



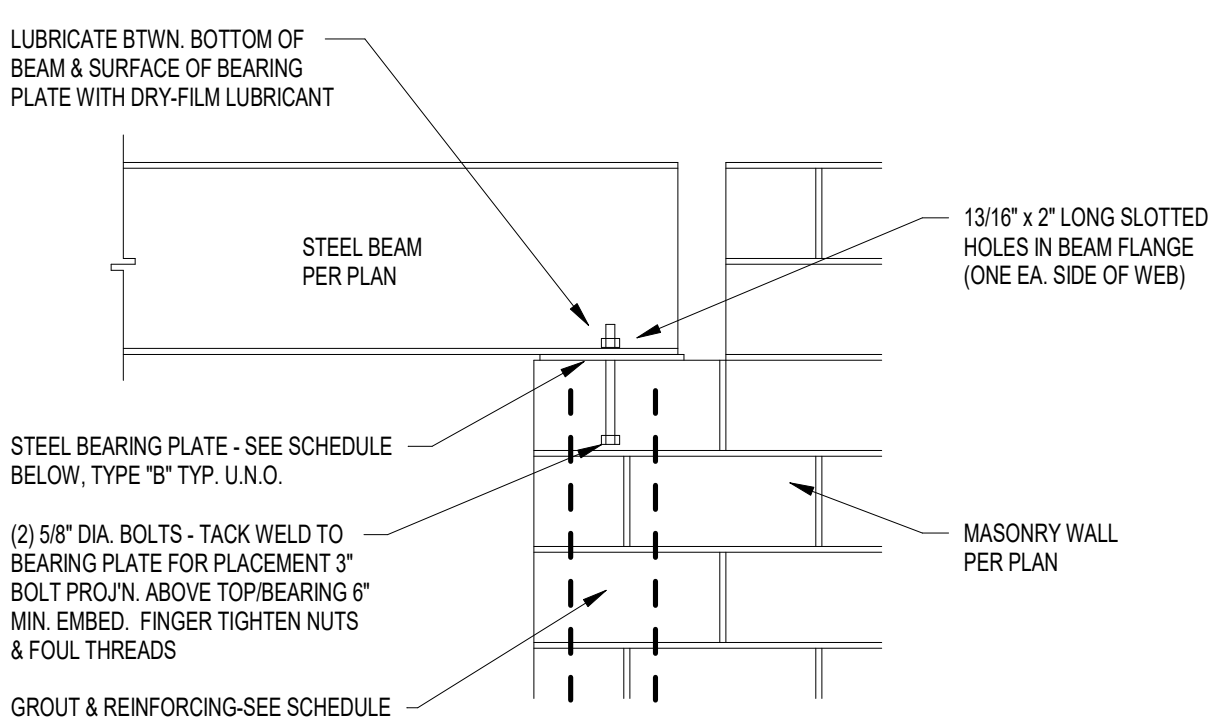
9  
S402 WALL INTERSECTION DETAIL  
SCALE: NONE



10  
S402 WALL INTERSECTION DETAIL  
SCALE: NONE



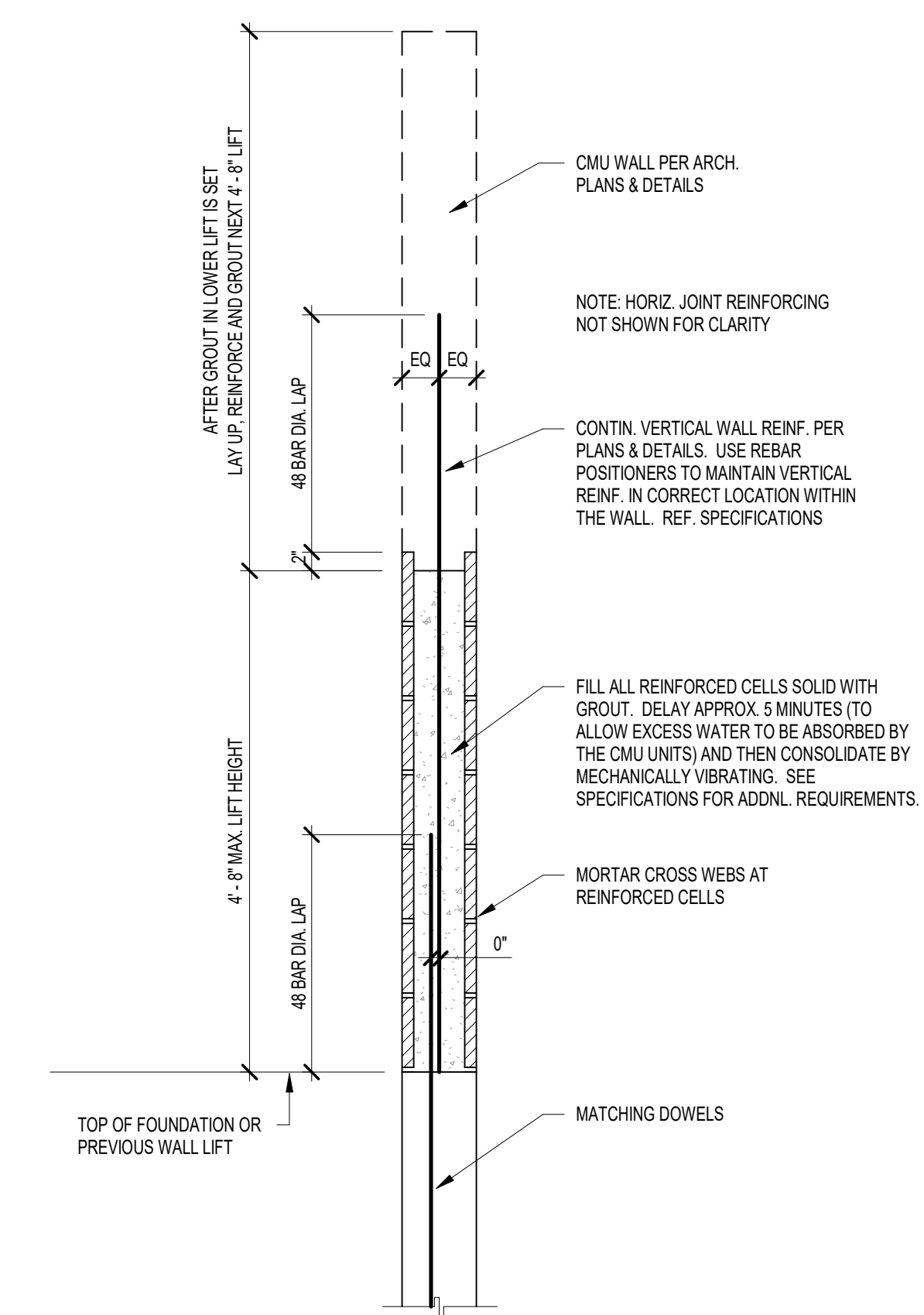
BEAM PERPENDICULAR TO WALL



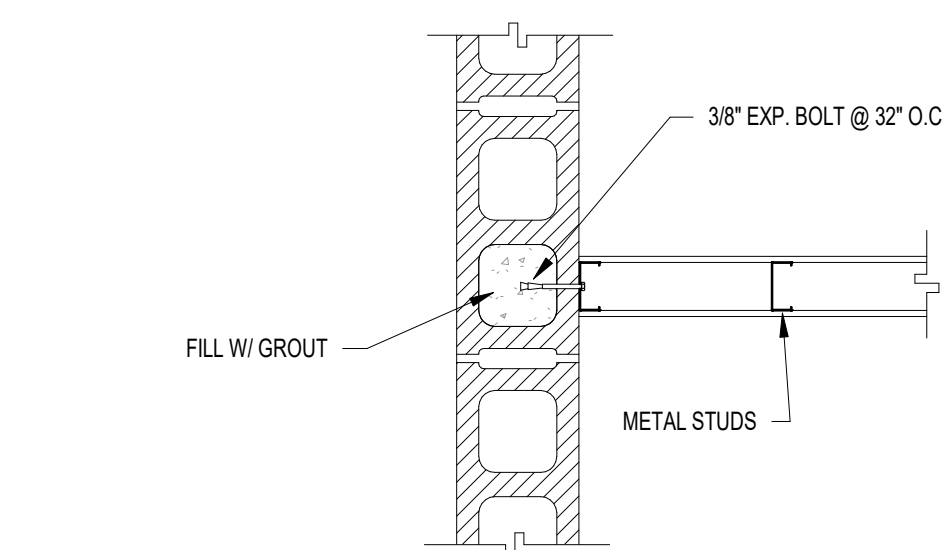
BEAM PARALLEL TO WALL

BEARING PLATE SCHEDULE			
MARK	BEARING PLATE SIZE	GROUT REQUIREMENTS	REINFORCING
A	3/4 x 6 1/2 x 1'-0	MIN. 3 COURSES DEEP x 24" LONG	NONE UNLESS LOCATED AT DISCONTIN. END OF WALL (IF SO, PROVIDE (2) FULL-HEIGHT VERTS. & EXTEND GROUT TO FTG.)
B	1/2 x 6 1/2 x 1'-0	GROUT SOLID TO FTG x 24" LONG	(2) VERTICALS FULL HEIGHT OF WALL. SEE PLANS & DETAILS FOR SIZE.

11  
S402 BEAM BEARING PLATE DETAILS  
SCALE: NONE



12  
S402 LOW-LIFT WALL CONSTRUCTION  
SCALE: NONE



13  
S402 CMU WALL/STUD WALL INTERSECTION  
SCALE: NONE



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550 Virginia Avenue  
Indianapolis, IN 46203  
STRUCTURAL ENGINEERS

PROJECT:  
PLAINFIELD COMMUNITY SCHOOL CORPORATION  
PLAINFIELD OUTBUILDINGS  
PLAINFIELD, IN

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CHECKED BY MDL

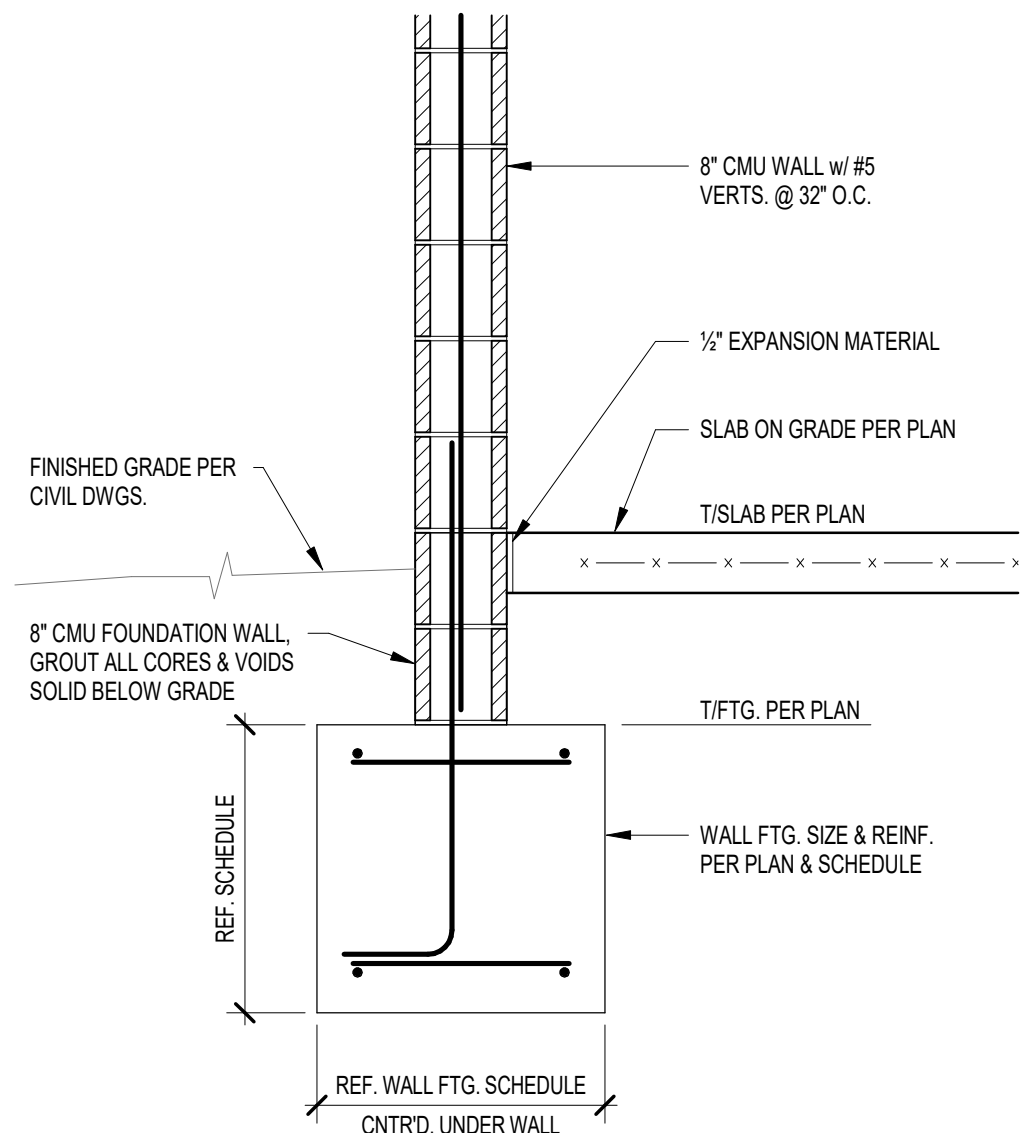
DRAWING TITLE:  
TYPICAL DETAILS

CERTIFIED BY:  
MARK D. LAY  
REGISTERED PROFESSIONAL ENGINEER  
STATE OF INDIANA  
PE19400114

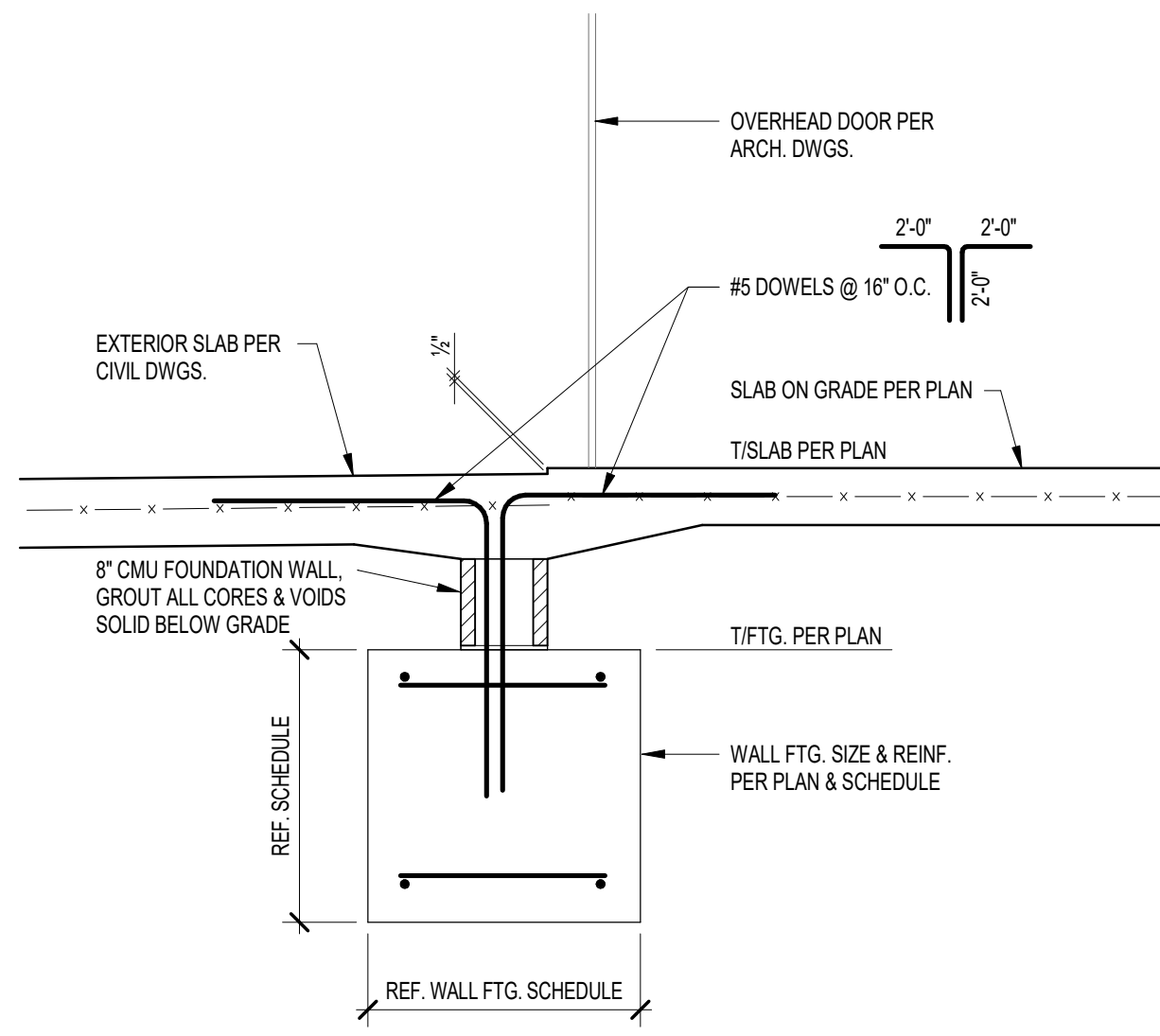
DRAWING NUMBER  
S402

PROJECT NUMBER  
2025016

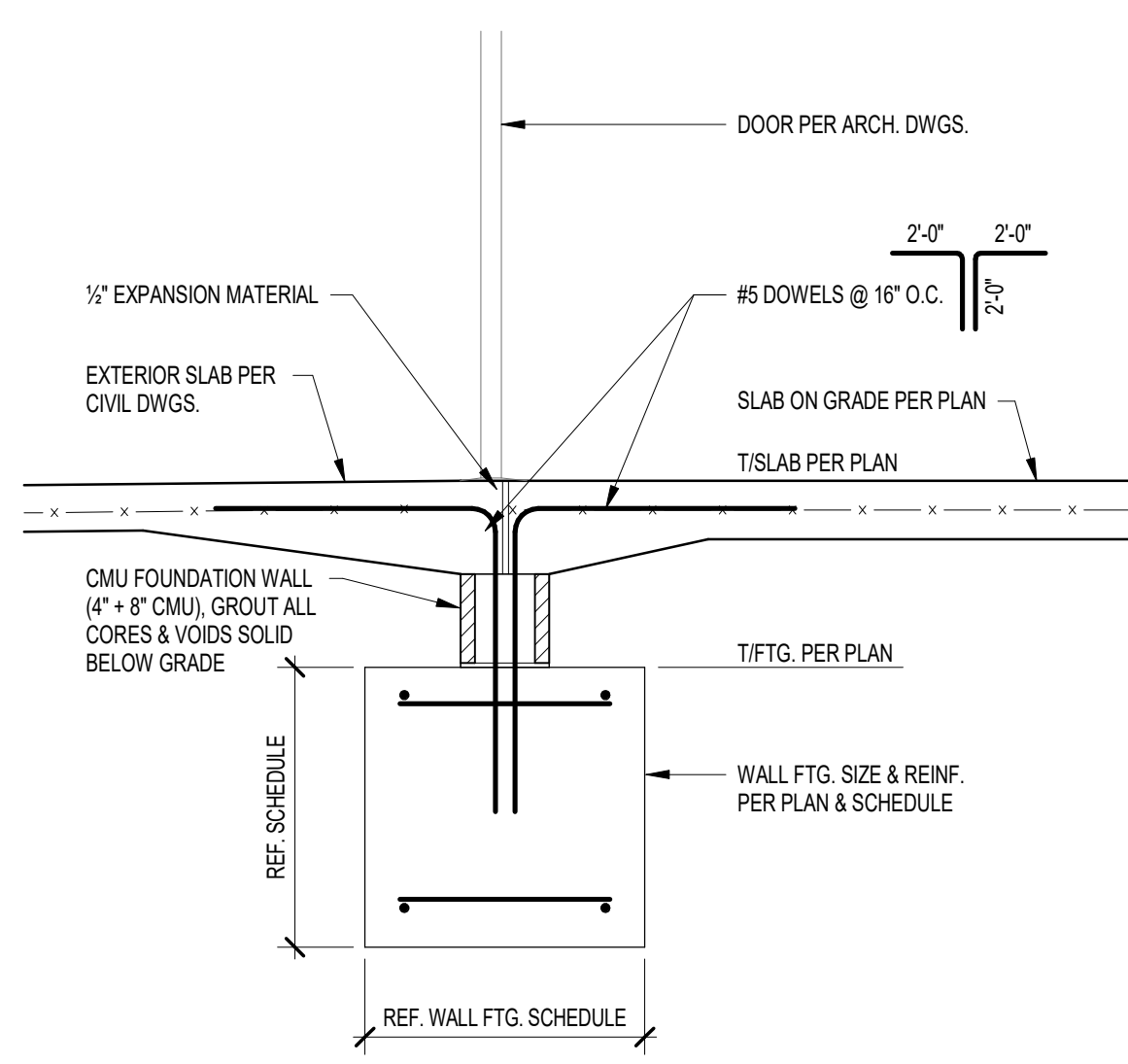




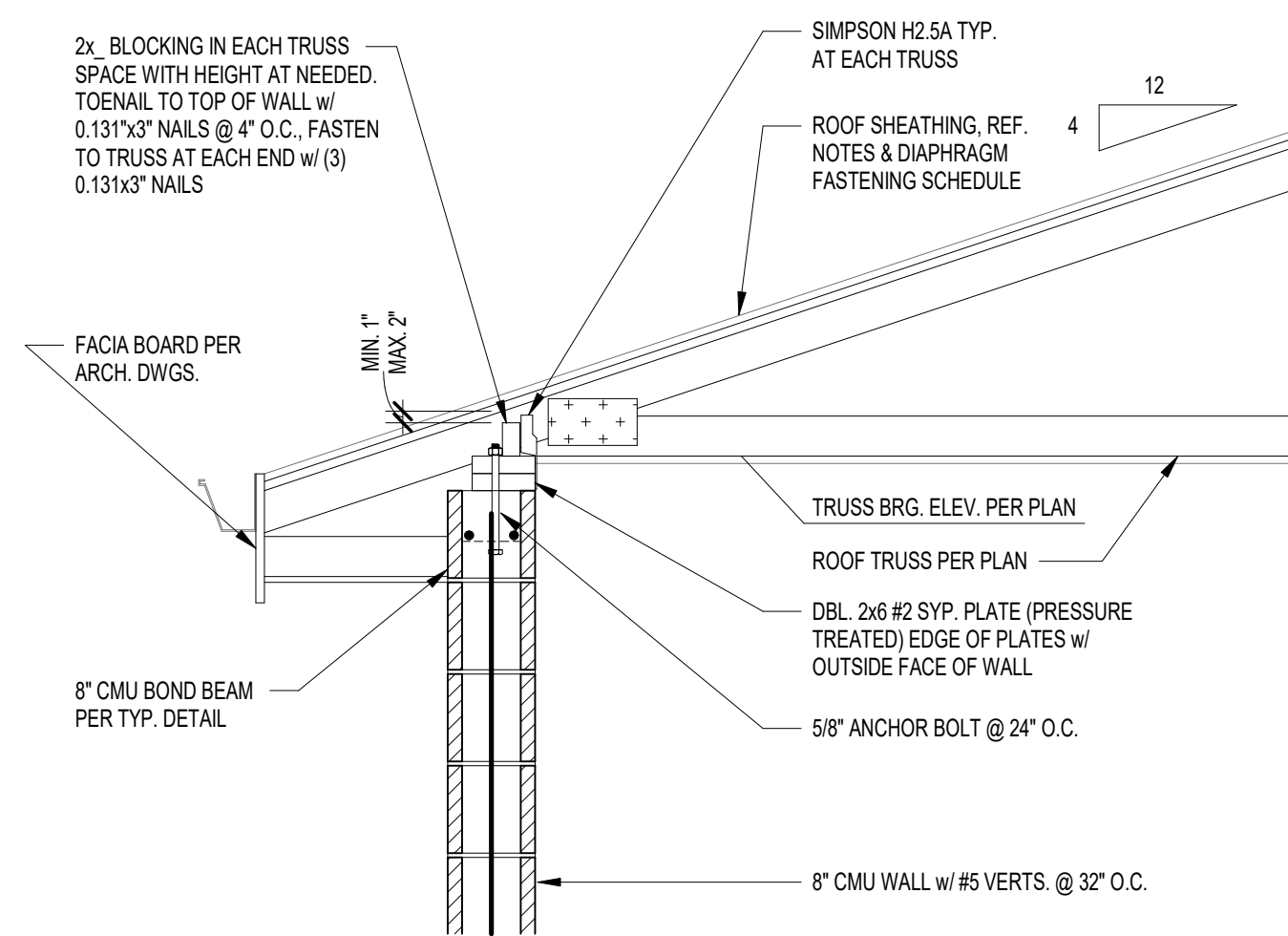
1 SECTION  
S701 SCALE: 3/4" = 1'-0"



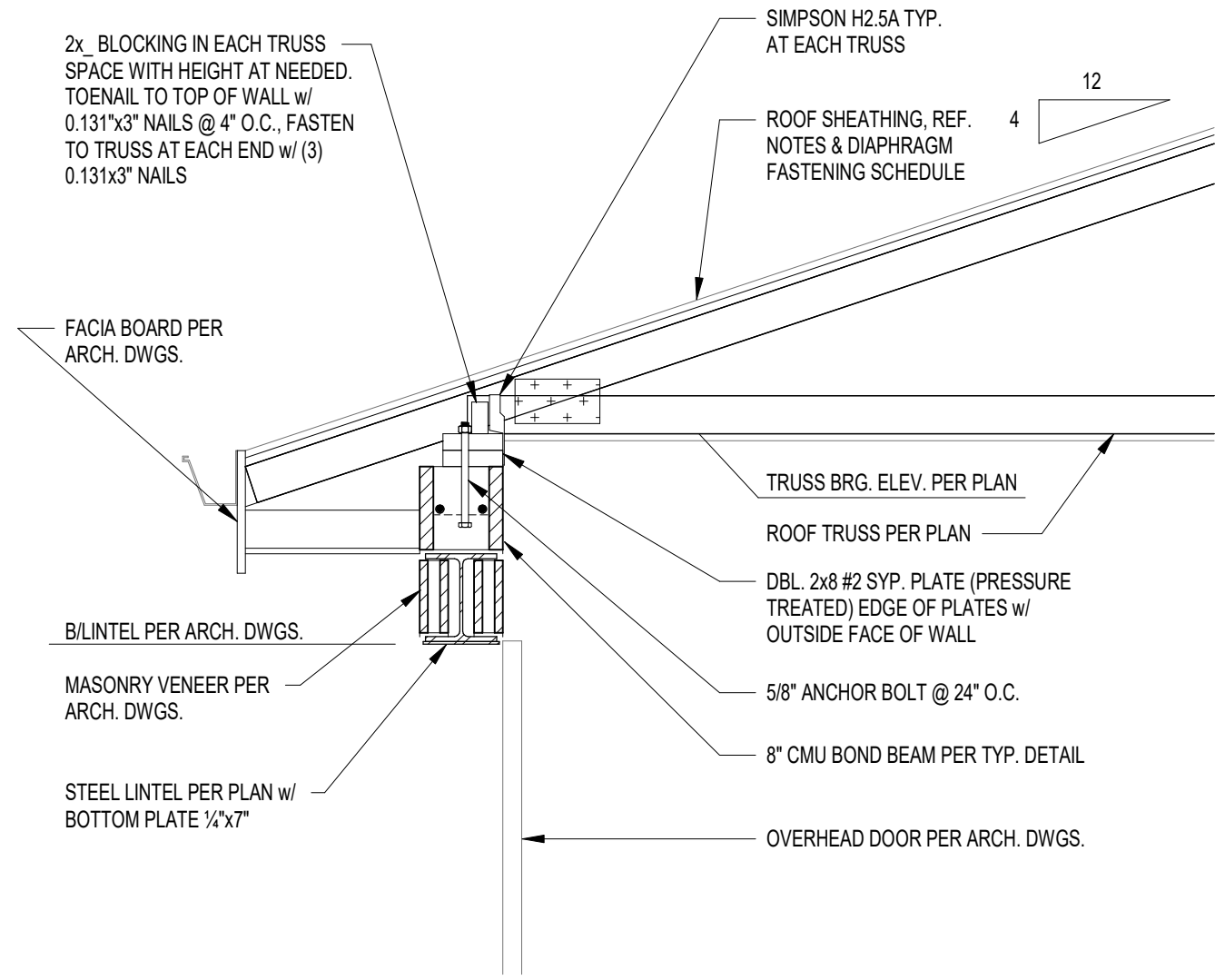
2 SECTION  
S701 SCALE: 3/4" = 1'-0"



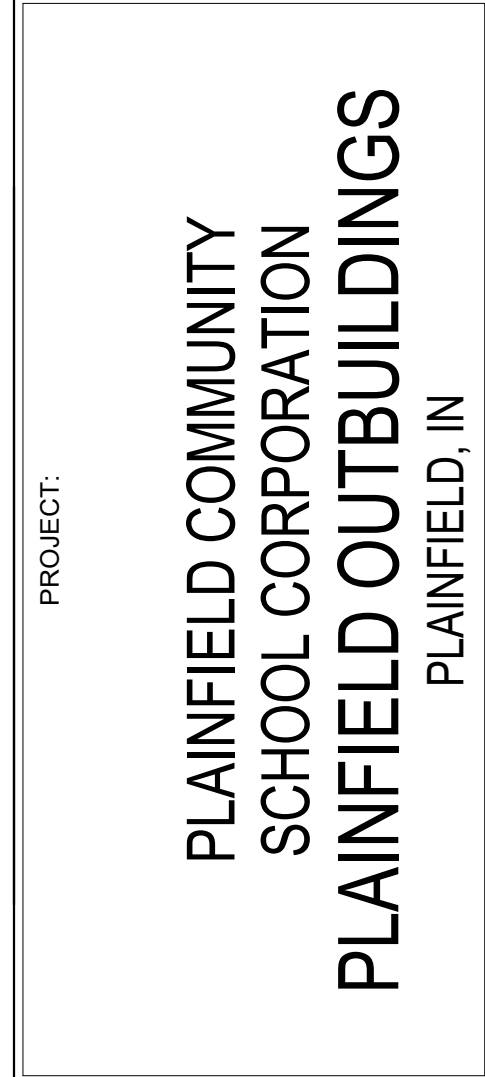
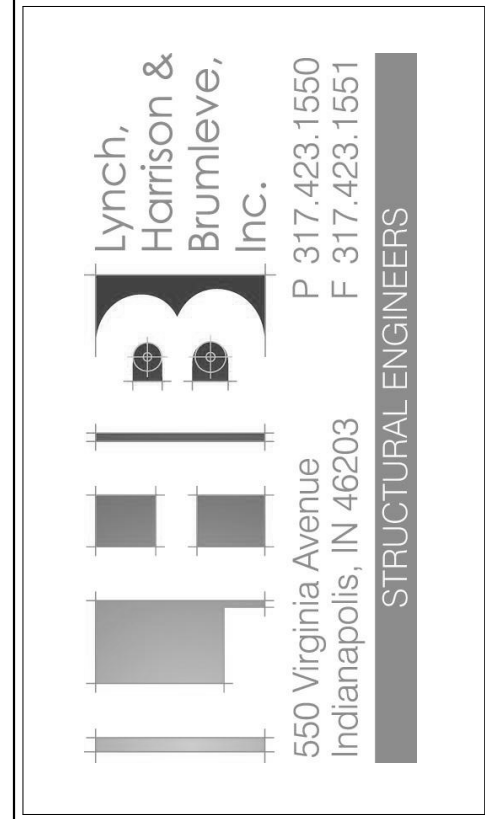
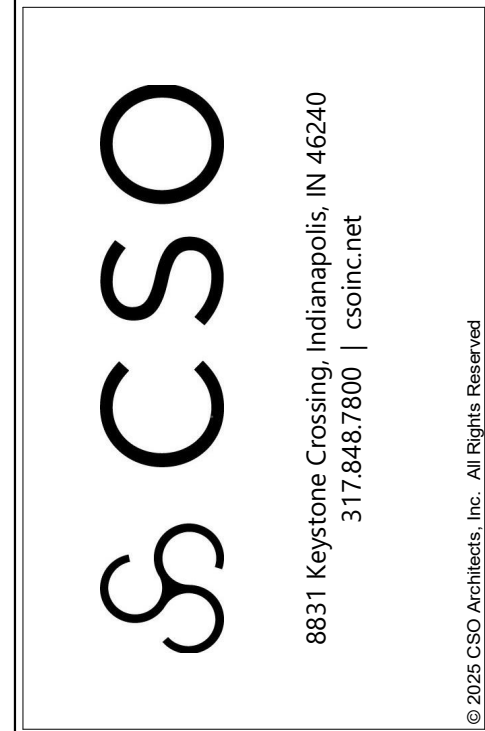
3 SECTION  
S701 SCALE: 3/4" = 1'-0"



4 SECTION  
S701 SCALE: 3/4" = 1'-0"



5 SECTION  
S701 SCALE: 3/4" = 1'-0"

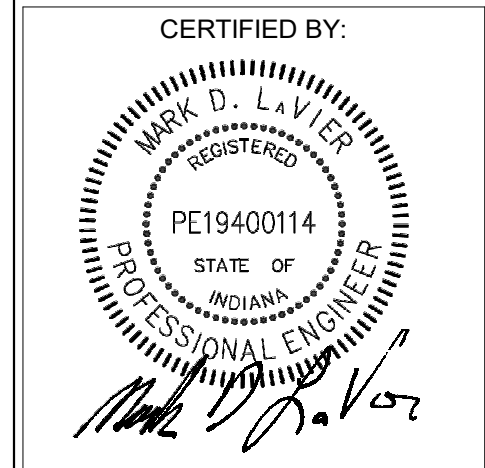


SCOPE DRAWINGS:  
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REVISIONS:

ISSUE DATE	DRAWN BY	CHECKED BY
05/08/2025	MDL	MDL

DRAWING TITLE:  
**SECTIONS AND DETAILS**



DRAWING NUMBER  
**S701**

PROJECT NUMBER  
**2025016**



STORAGE BUILDING ROOF PLAN DETAILS

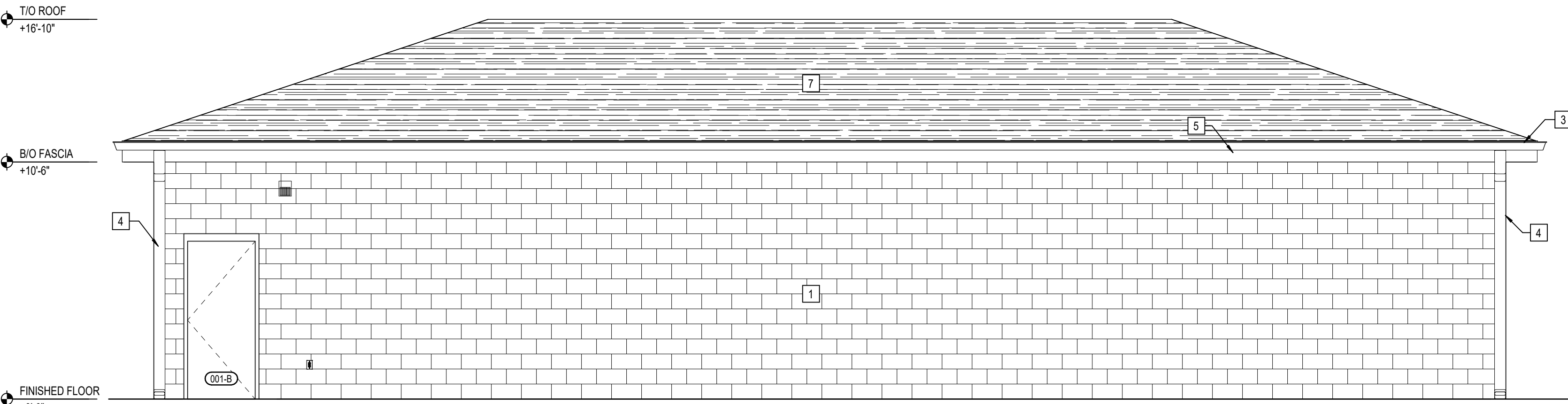
NOTE: IBC 2012 CODE REQUIREMENTS

ATTIC AREA = 2027 S.F.  
(NO DRAFTSTOPPING REQUIRED)

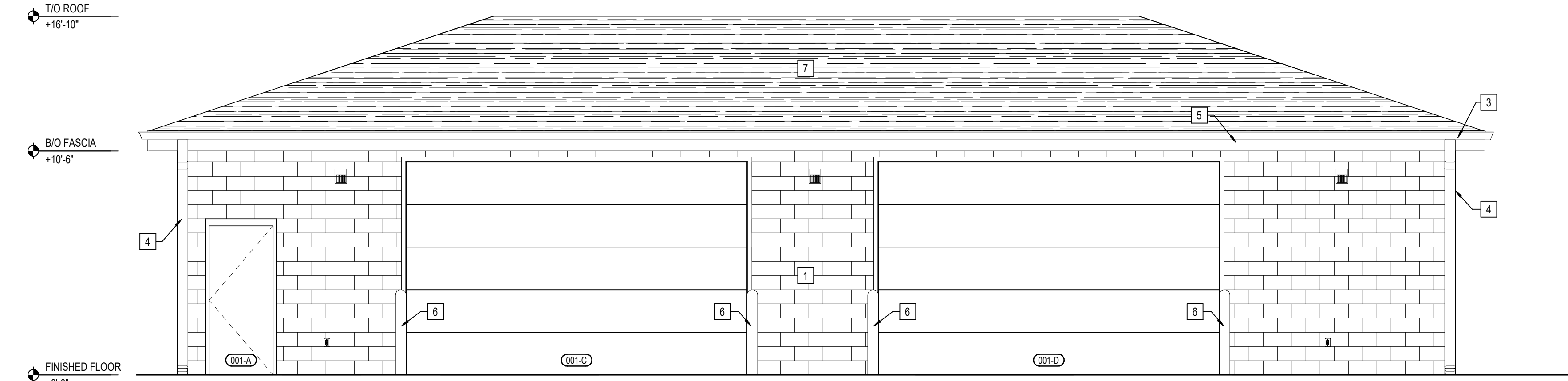
VENTILATION REQUIREMENT: 1/150 = 13.51 S.F.  
(50% LOW 50% HIGH)  
6.76 S.F. LOW - USE HALF VENTED SOFFIT PANEL AT 6% FREE AREA  
6.76 S.F. HIGH - USE RIDGE VENT = 1 FT. LENGTH X 20 SQ. IN. FREE VENT AREA  
24" VENT LENGTH X 20 SQ. IN. = 3.33 S.F.  
EXCEEDS REQUIREMENT

ATTIC ACCESS REQ'D: NOT REQ'D

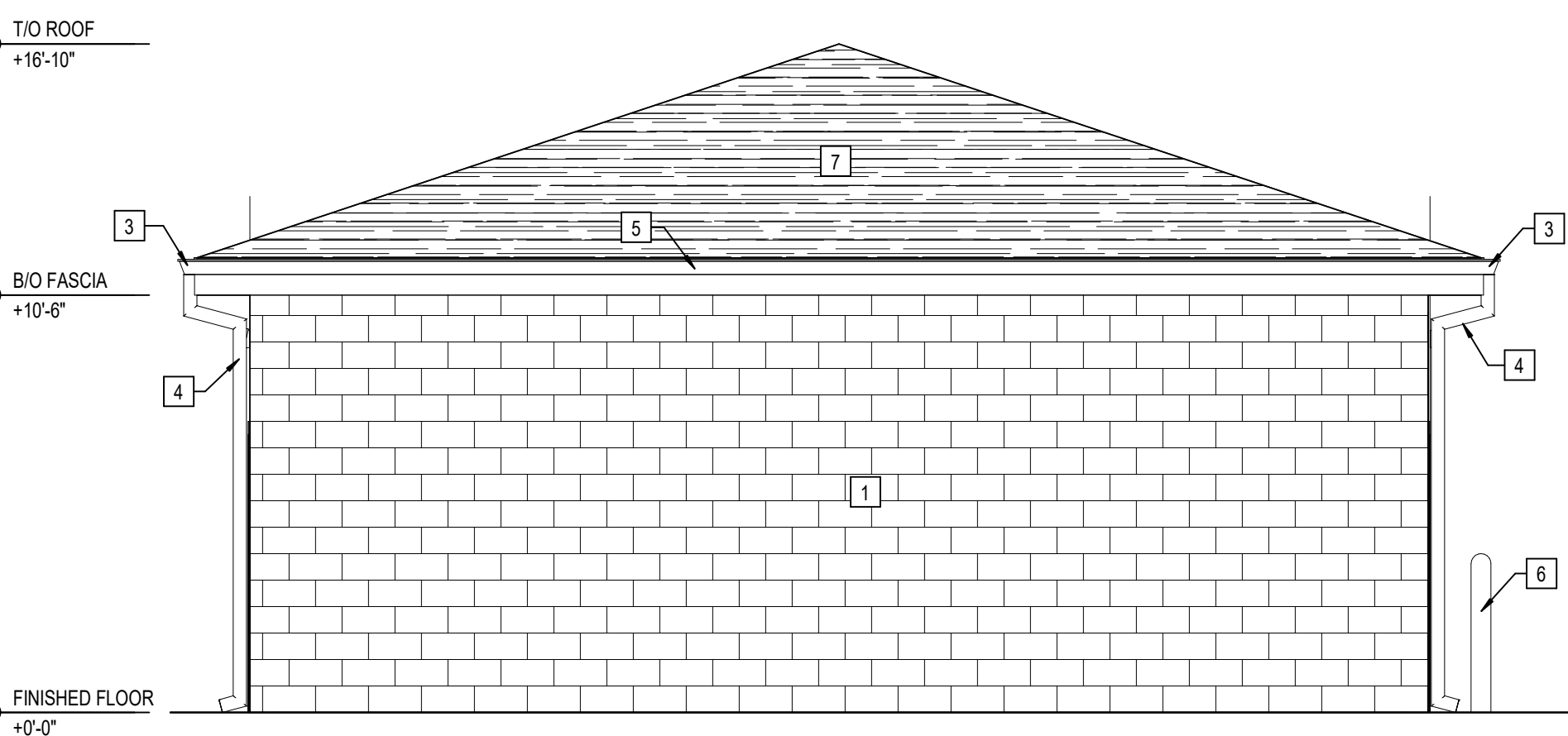
4:12 SHINGLE ROOF:  
1 LAYERS UNDERLAYMENT REQUIRED



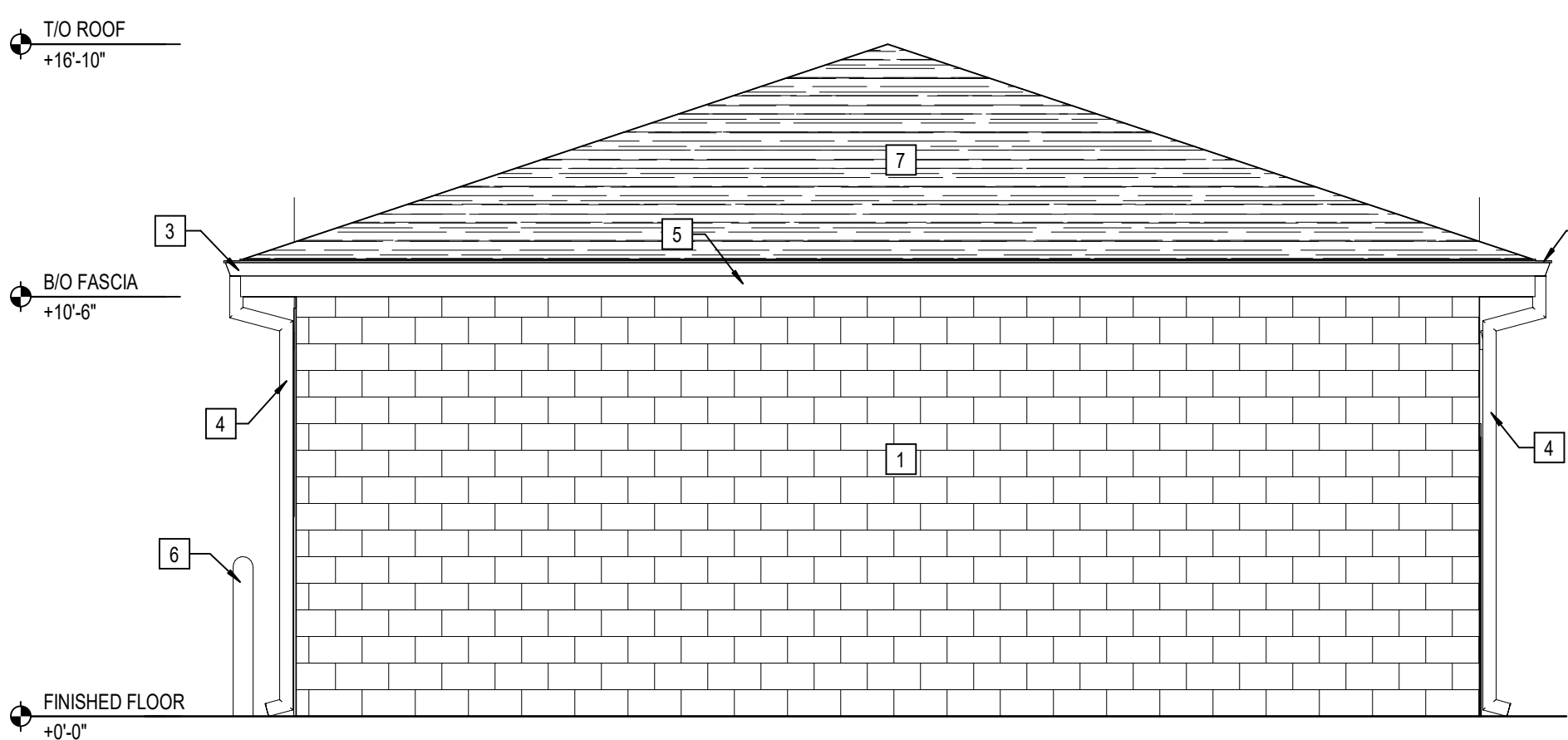
6 CLARKS CREEK STORAGE - SOUTH ELEVATION  
A201 SCALE: 1/4" = 1'-0"



4 CLARKS CREEK STORAGE - NORTH ELEVATION  
A201 SCALE: 1/4" = 1'-0"



7 CLARKS CREEK STORAGE - EAST ELEVATION  
A201 SCALE: 1/4" = 1'-0"



2 CLARKS CREEK STORAGE - WEST ELEVATION  
A201 SCALE: 1/4" = 1'-0"

FINISH LEGEND

FLOOR FINISHES

WALK-OFF CARPET

SC1 TYPE: SEALED CONCRETE  
LOCATION: STORAGE SPACES  
NOTE: SEE SPECIFICATION

FINISH TAG KEY

XXX FLOOR FINISH

X6 WALL FINISH

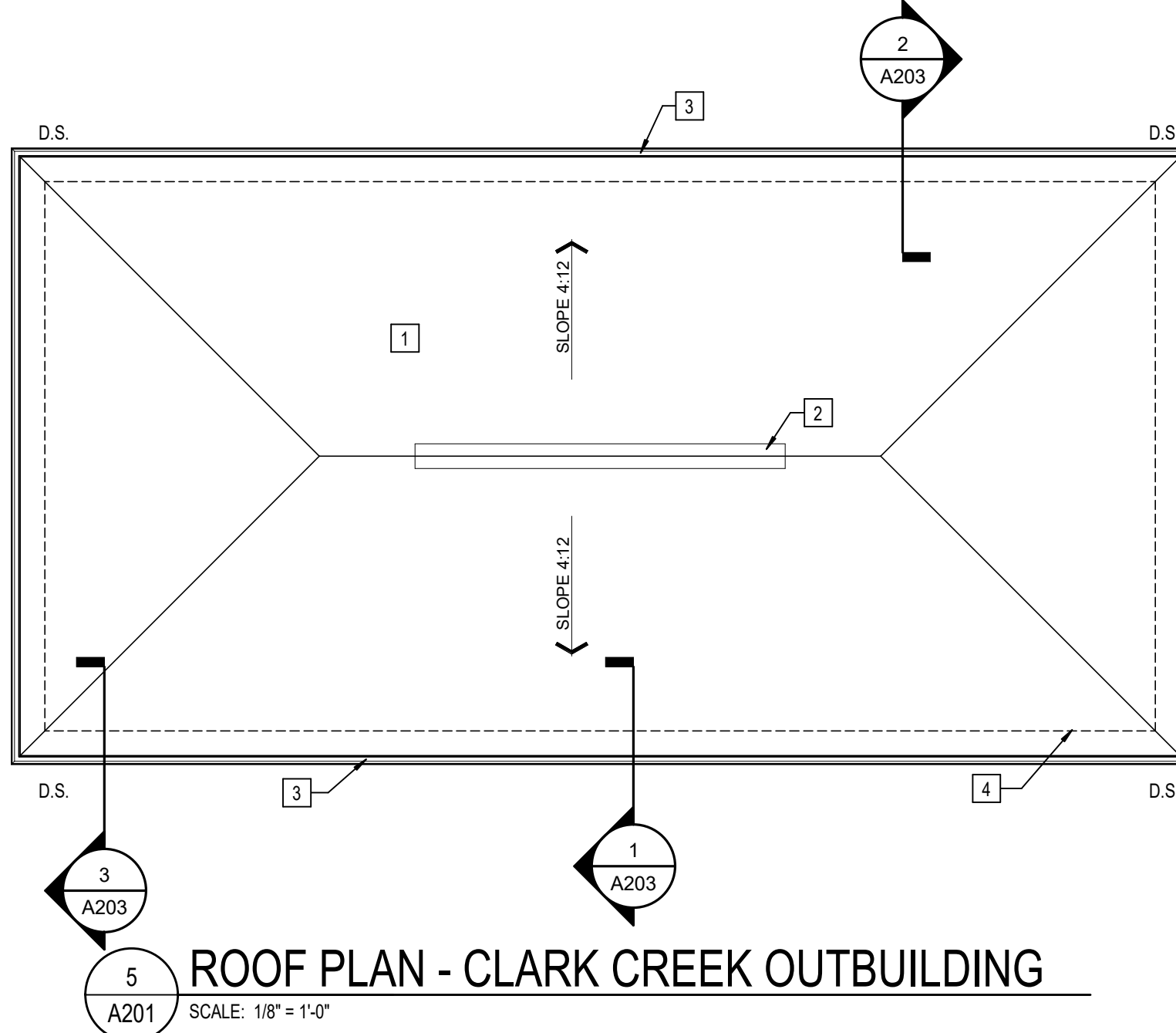
WALL FINISHES

EPOXY PAINT

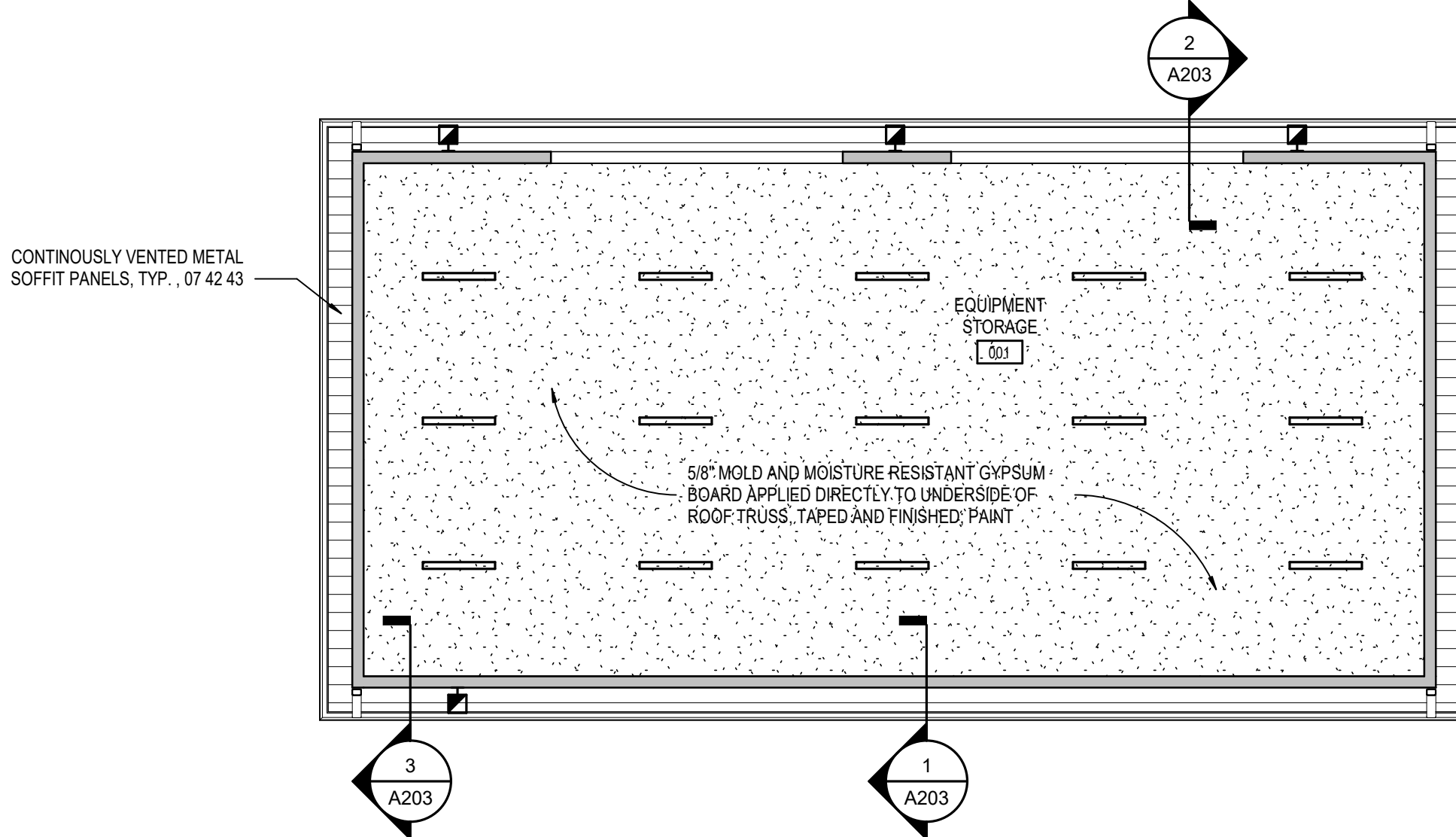
EP1 MFR: SHERWIN WILLIAMS  
COLOR: SW7088 RESERVED WHITE  
NOTE: GENERAL

ELEVATION NOTES

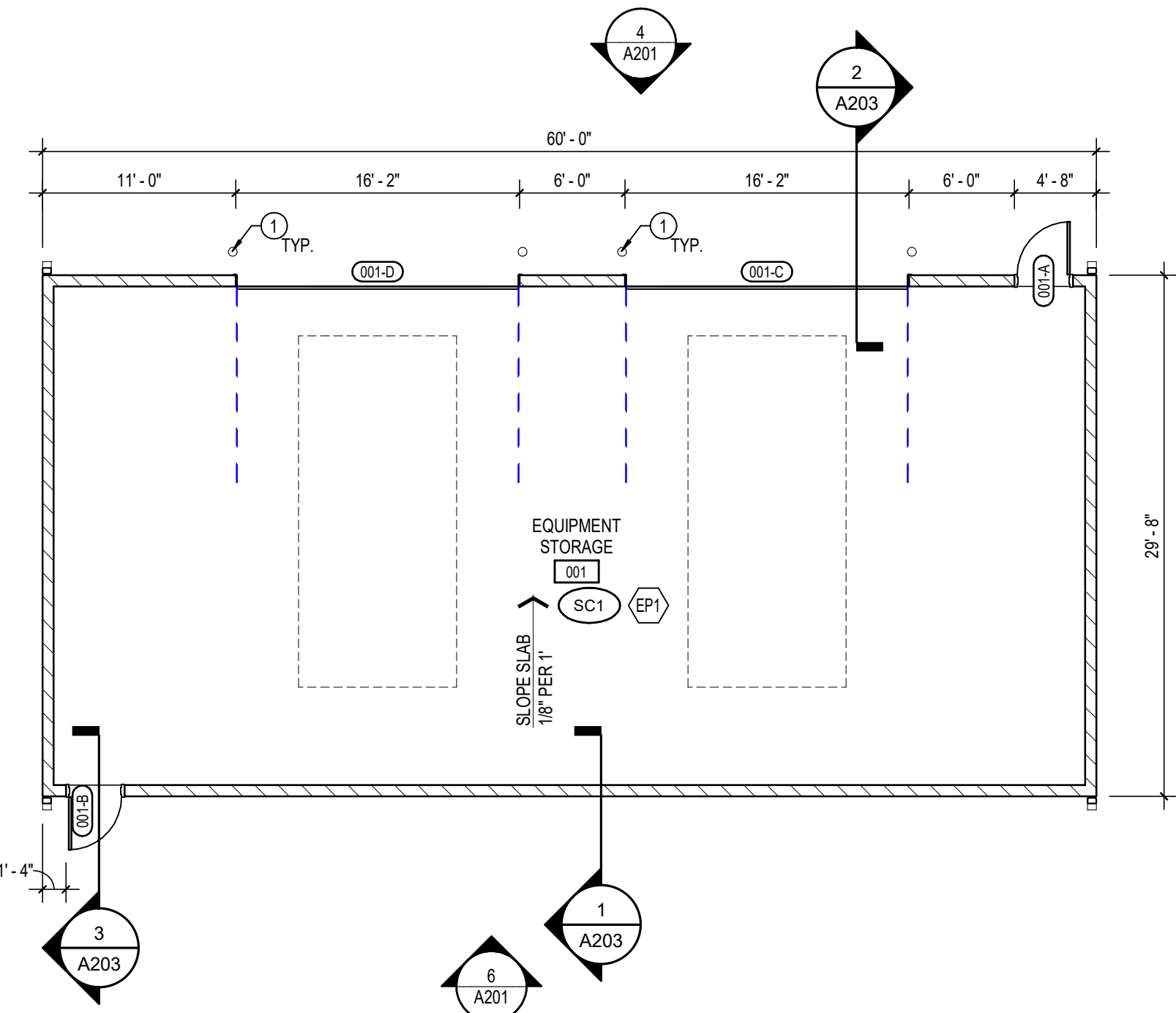
- STRUCTURAL BRICK - TYPE 1, SEE SPECS.
- STRUCTURAL BRICK - TYPE 2, SEE SPECS.
- PRE-MANUFACTURED ALUMINUM GUTTER
- PRE-MANUFACTURED ALUMINUM DOWNSPOUT
- PRE-MANUFACTURED FASCIA BOARD
- CONCRETE BOLLARD, PAINT
- ASPHALT SHINGLE ROOFING SYSTEM.



ROOF PLAN - CLARK CREEK OUTBUILDING  
SCALE: 1/8" = 1'-0"



CEILING PLAN - CLARKS CREEK OUTBUILDING  
A201 SCALE: 1/8" = 1'-0"



FLOOR PLAN - CLARKS CREEK OUTBUILDING  
A201 SCALE: 1/8" = 1'-0"



GENERAL NOTES

- COORDINATE THE WORK OF EACH TRADE WITH THE WORK OF OTHER TRADES.
- ALL WORK IS TO BE COMPLETED IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, ORDINANCES, RULES, REGULATIONS AND STANDARDS INCLUDING, BUT NOT LIMITED TO THOSE LISTED ON THE COVER SHEET. ALL APPLICABLE RULES & REGULATIONS ARE TO BE THE MOST CURRENT ADOPTED EDITIONS.
- FIELD VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO THE COMMENCEMENT OF WORK. DISCREPANCIES BETWEEN THE DOCUMENTS AND THE ACTUAL CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO THE COMMENCEMENT OF WORK.
- ALL DIMENSIONS ARE FROM CENTERLINE OF STRUCTURE. FINISH FACE OF WALL, FACE OF MASONRY, OR FACE OF EXISTING.
- ANY DIMENSIONS NOT SHOWN OR DEEMED QUESTIONABLE ARE TO BE VERIFIED BY ARCHITECT. DO NOT SCALE DRAWINGS.
- REFER TO WALL TYPE SCHEDULE, SHEET A200, TO DETERMINE WHICH WALLS EXTEND TO DECK. SEE STRUCTURAL FOR TOP SUPPORT DETAIL. WHERE METAL STUDS EXTEND TO DECK, PROVIDE SLP CONNECTIONS FOR ROOF FLOOR DEFLECTION.
- ALL STEEL STUDS ARE TO BE BRACED ACCORDING TO MANUFACTURER LIMIT HEIGHT (L240).
- WHERE INSULATED OR SOUND WALLS EXTEND TO DECK, FILL DECK FLUTES WITH INSULATION SOUND ATTENUATION.
- REFER TO PLUMBING PLANS FOR LOCATION OF FLOOR DRAINS.
- WHERE ACCESS PANELS ARE SHOWN IN TOILET ROOM CHASES, FINAL LOCATION SHALL BE COORDINATED WITH OTHER TRADES PRIOR TO INSTALLATION.
- ALL CONCRETE MASONRY UNITS (CMU) SHALL BE LAID RUNNING BOND UNO. CMU WALLS THAT DO NOT LAY OUT IN FULL OR HALF LENGTHS SHOULD BE BALANCED SO AS NOT TO HAVE ANY PIECES LESS THAN 4" IN SIZE EXPOSED TO VIEW.
- ALL INTERIOR MASONRY WALLS THAT RUN TO UNDERSIDE OF DECK ABOVE SHALL HAVE A 2" JOINT (UNO.) AT THE DECK TO BE FILLED WITH FIRE STOPPING AT RATED WALLS PER PROJECT MANUAL, AND MINERAL WOOL TO THE BUILDING STRUCTURE TO ALLOW FOR DEFLECTION.
- THERE SHALL BE PERIMETER INSULATION CONTINUOUS AROUND THE ENTIRE PERIMETER OF THE BUILDING EXTENDING 2'-0" MINIMUM (R-15 MIN.) HORIZONTAL.
- PROVIDE MISCELLANEOUS SUPPORT FOR ALL CEILING SUSPENDED ITEMS. DOOR AND FRAME NUMBERS CORRESPOND TO ROOM NUMBERS, WHERE MORE THAN ONE DOOR OCCURS IN A ROOM, A SUFFIX HAS BEEN ADDED (E.G. A100-1). SEE A500 SERIES DRAWINGS FOR DOOR SCHEDULE AND DETAILS.
- ALL DOOR FRAMES SHALL BE LOCATED 4" OFF FINISH WALLS OR 4" OFF MASONRY WALLS UNLESS NOTED OTHERWISE.
- ALL GLASS AT INTERIOR DOOR FRAMES, DOOR LITES AND WINDOW FRAMES IS TO BE 1/4" CLEAR TEMPERED GLASS UNLESS NOTED OTHERWISE.
- AT BUILDING EXPANSION JOINTS, ALL PARTITIONS, CEILINGS, FLOORS AND ALL WALL, FLOOR OR CEILING MOUNTED ITEMS SHALL BE ANCHORED TO THE BUILDING STRUCTURE ON ONLY ONE SIDE OF THE EXPANSION JOINTS. CONTRACTOR SHALL COORDINATE CONSTRUCTION OR INSTALLATION OF ALL ITEMS NOTED TO ASSURE THAT NO SUCH ITEMS BRIDGE ACROSS THE EXPANSION JOINT.
- ALL SUB-ON GRADE CONTROL JOINTS TO BE CLEANED AND CAULKED PRIOR TO PLACEMENT OF FLOOR FINISH.
- SEE REFLECTED CEILING PLANS FOR BULKHEAD LOCATIONS AND DETAILS.
- REFER TO MECHANICAL DRAWINGS FOR WALL LOWER LOCATIONS, SIZES AND QUANTITIES.
- SEE A500 SERIES DRAWINGS FOR FINISH SCHEDULE AND PLANS.
- SEE A500 SERIES DRAWINGS FOR EQUIPMENT SCHEDULE AND PLANS.
- PROVIDE BLOCKING IN STUD WALLS AND/OR GROUTED MASONRY CORES AS REQUIRED TO SUPPORT EQUIPMENT.
- PROVIDE FIRE RESISTANT TREATED WOOD BLOCKING SUPPORTS AS REQUIRED FOR ALL SURFACE MOUNTED ITEMS.
- WHERE DISJUNCT FLOOR MATERIALS MEET, THEY SHALL DO SO UNDER THE CENTERLINE OF THE DOOR UNLESS NOTED OTHERWISE.
- APPLY SEALANT AT ALL JUNCTURES BETWEEN DIFFERENT MATERIALS (E.G. MASONRY TO GYPSUM WALL BOARD) UTILIZING THE APPROPRIATE TYPE PER SPECIFICATIONS. COLOR TO BE SELECTED BY ARCHITECT.
- APPLY SEALANT AT ALL COUNTERTOPS AND BLACKSPASHES AT JUNCTURE WITH WALL.
- ALL DOORS MUST BE INSTALLED WITH AT LEAST THE MINIMUM MANEUVERING CLEARANCE AT THE DOOR APPROACH PER THE MOST CURRENT AMERICANS WITH DISABILITIES ACT.
- BASE FLOOR ELEVATION INDICATED FOR THIS PROJECT IS 100'-0". REFER TO SITE PLAN FOR CORRELATION TO UGSS DATUM.

PLAN NOTES

- ALIGN CONCRETE BOLLARD FLUSH WITH DOOR JAMB. SEE SECTION 4/A401 FOR ADDITIONAL INFORMATION.

ROOF ABBREVIATIONS

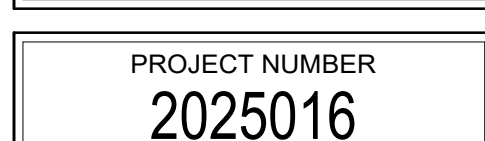
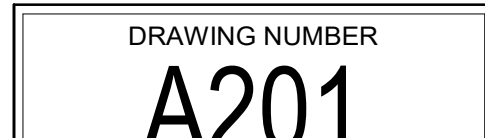
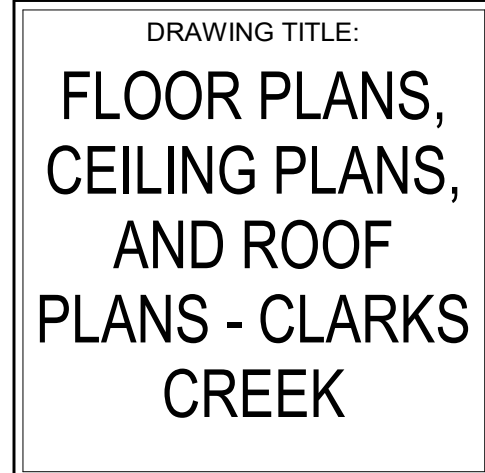
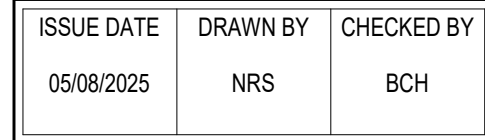
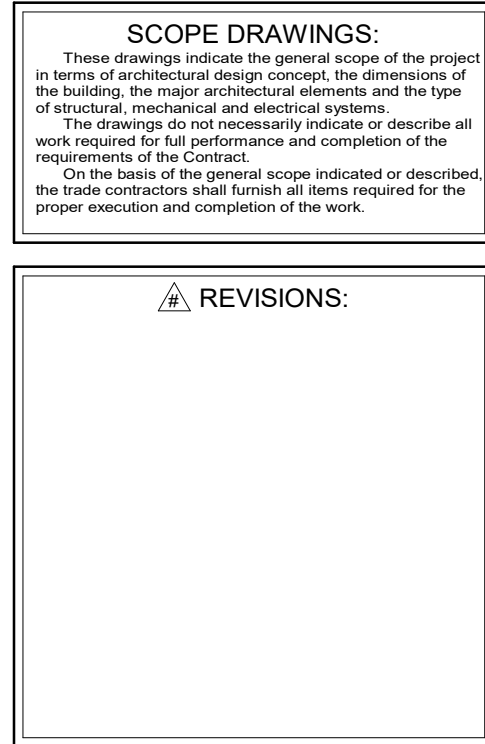
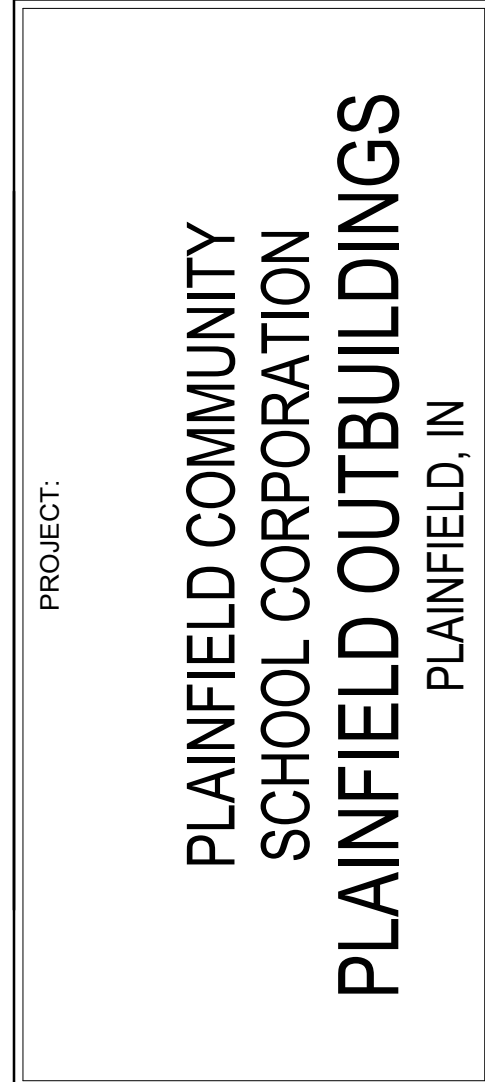
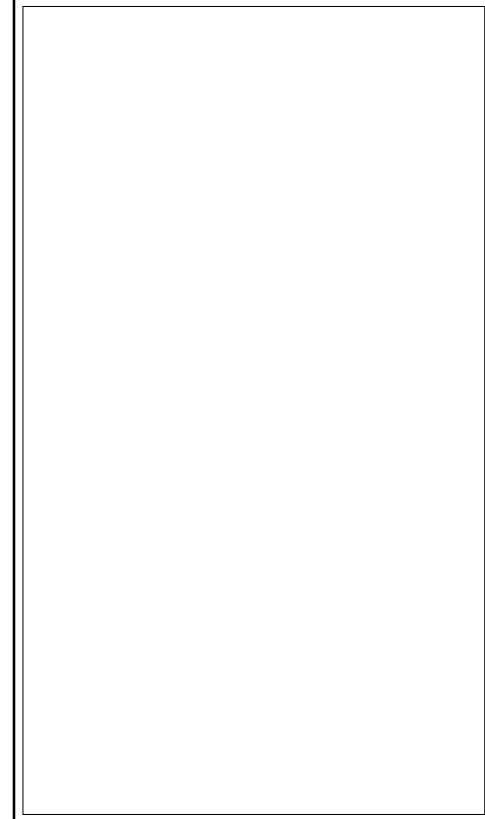
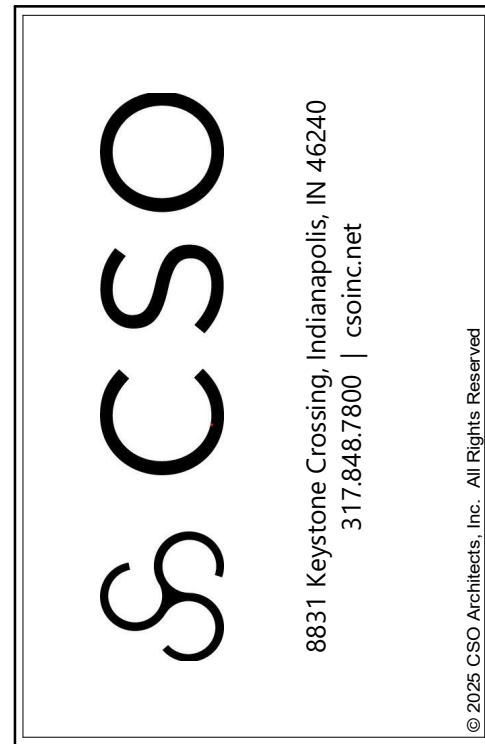
DR DUCT THRU ROOF  
DS METAL DOWNSPOUT  
EF EXHAUST FAN, SEE MECHANICAL  
EJ EXPANSION JOINT  
FS FLUE STACK, SEE MECHANICAL  
GU METAL GUTTER  
PV PLUMBING VENT  
RAV RELIEF AIR VENT, SEE MECHANICAL  
RH ROOF HATCH

GENERAL ROOF NOTES

- ALL WORK SHALL BE IN ACCORDANCE WITH THE BEST QUALITY STANDARDS OF THE TRADE, AND SHALL CONFORM WITH THE LATEST EDITION OF ALL FEDERAL, STATE, AND LOCAL CODES AND STANDARDS. THE SAME ARE MADE A PART OF THESE CONTRACT DOCUMENTS, AS IF REPEATED HEREIN.
- CONTRACT DOCUMENTS CONSIST OF BOTH THE PROJECT MANUAL AND DRAWINGS, AND BOTH ARE INTENDED TO BE COMPLEMENTARY. ANYTHING APPEARING ON EITHER MUST BE EXECUTED THE SAME AS IF SHOWN ON BOTH.
- CONSTRUCTION DOCUMENTS SHALL BE FOLLOWED AS CLOSELY AS POSSIBLE. HOWEVER, SYSTEMS HAVE BEEN SHOWN DIAGRAMMATICALLY AND IN SOME CASES, ENLARGED FOR CLARITY. PROVIDE ADDITIONAL ITEMS AS REQUIRED TO PROVIDE A COMPLETE AND COORDINATED SYSTEM.
- CONTRACTOR SHALL PROVIDE ANY AND ALL TEMPORARY UTILITY SERVICE REQUIRED TO CONSTRUCT THE WORK. CONTRACTOR MAY EXTEND SERVICES FROM EXISTING LOCATIONS TO WHERE THEY ARE REQUIRED. REMOVE TEMPORARY UTILITIES AND RELATED EXTENSIONS AS SOON AS PRACTICABLE. RESTORE ALL AFFECTED AREAS TO ORIGINAL CONDITION.
- CONTRACTOR SHALL REMOVE CONSTRUCTION DEBRIS FROM THE BUILDING AND ROOF DAILY.
- STORE VOLATILE OR FLAMMABLE LIQUIDS IN UL LISTED FIRE CABINETS.
- CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE SECURITY OF ALL STORED MATERIALS AND EQUIPMENT INSIDE OR OUTSIDE THE BUILDING.
- CONTRACTOR SHALL FURNISH NECESSARY TEMPORARY PROTECTION FROM WEATHER TO PROTECT INTERIOR OF BUILDING FROM ELEMENTS OF WEATHER AT ALL TIMES.
- CONTRACTOR RESPONSIBLE FOR TRAFFIC PROTECTION DURING CONSTRUCTION. AREAS OF WORK SUBJECTED TO TRAFFIC BY VARIOUS TRADES SHALL BE PROTECTED BY TEMPORARY WALK PADS.
- PROVIDE TREATED WOOD BLOCKING EQUAL IN THICKNESS TO INSULATION SYSTEM AT ROOF PERIMETER AND AROUND ALL ROOF PENETRATIONS. ANCHOR PER SECTION 4-45 OF THE FM GLOBAL LOSS PREVENTION GUIDE.
- EXTEND ALL PLUMBING VENTS TO PROVIDE A MIN. OF 12" OF HEIGHT FROM TOP OF INSULATION. ALL FITTINGS TO BE AIR AND WATER TIGHT. SEE PLUMBING PLANS FOR LOCATIONS.
- ROOF INSULATION SADDLES AND CRICKETS ARE DIAGRAMMATIC. ROOF INSULATION MANUFACTURER SHALL DESIGN AND SIZE THESE PER THE ROOF MEMBRANE MANUFACTURERS RECOMMENDATIONS. CRICKETS AND SADDLES SHOULD HAVE A MINIMUM OF TWO TIMES THE SLOPE OF THE PRIMARY TAPERED SYSTEM OR STRUCTURAL SLOPE. THE RATIO OF A CRICKETS WIDTH TO LENGTH SHOULD BE NO LESS THAN 1 TO 3.
- PROVIDE SADDLES/CRICKETS AROUND ALL NEW ROOF TOP EQUIPMENT. SEE MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR LOCATIONS OF ALL ROOF PENETRATIONS.
- PROVIDE TAPERED INSULATION WHERE REQUIRED TO TRANSITION FROM ONE INSULATION HEIGHT TO ANOTHER.
- NOTCH ALL INSULATION AS REQUIRED TO ACCOMMODATE SURFACE MOUNTED CONDUIT, FASTENERS, OFFSETS AND OTHER PROJECTIONS EXTENDING ABOVE THE SURFACE OF THE DECK.
- PERIMETER EDGE METAL TO COMPLY WITH ANSIPRIES-1 FM GLOBAL 1-49.
- SEE MECHANICAL, ELECTRICAL AND PLUMBING (MEP) SHEETS FOR ROOF TOP EQUIPMENT.
- INSPECT ALL WOOD BLOCKING SCHEDULED TO REMAIN. NOTIFY ARCHITECT OF ANY DETERIORATED BLOCKING NEEDING REPLACEMENT. CONTRACTOR TO REPLACE AND DAMAGED BLOCKING ON A TIME AND MATERIAL BASIS, SEE SPECIFICATIONS.
- ALL ROOF DETAIL DRAWINGS CONTAINED IN THIS SET ARE DIAGRAMMATIC. ADJUST ROOF DETAILS BASED ON SPECIFIC ROOFING SYSTEM SELECTED ACCORDING TO MANUFACTURERS WRITTEN SPECIFICATIONS AND APPROVED DETAIL DRAWINGS. ALL ASSEMBLY COMPLICATIONS SHOULD BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.
- THE ROOF CONTRACTOR SHALL PROTECT ALL ROOF DRAINS, GUTTERS AND DOWNSPOUTS FROM DEBRIS CREATED DURING CONSTRUCTION. THE ROOF CONTRACTOR SHALL CLEAR ALL DRAINS, GUTTERS AND DOWNSPOUTS PRIOR TO COMPLETION OF WORK AND TO ENSURE THAT THEY ARE FREE OF DEBRIS AND FUNCTIONING PROPERLY.
- MECHANICAL, ELECTRICAL AND PLUMBING INFORMATION SHOWN ON THIS PLAN IS GENERAL IN NATURE. REFER TO P, M AND E DRAWINGS FOR FURTHER INFORMATION AND COORDINATE ALL REQUIRED ROOF OPENINGS OR ROOF MOUNTED EQUIPMENT.

ROOF PLAN NOTES

- ASPHALT SHINGLE ROOFING SYSTEM, SEE SPECS.
- CONTINUOUS RIDGE VENT
- ALUMINUM GUTTER, TYP.
- LINE OF BUILDING BELOW





#### STORAGE BUILDING ROOF PLAN DETAILS

NOTE: IBC 2012 CODE REQUIREMENTS

ATTIC AREA = 1000 S.F.  
(NO DRAFTSTOPPING REQUIRED)

VENTILATION REQUIREMENT: 1/150 = 6.67 S.F.  
(50% LOW 50% HIGH)

3.33 S.F. LOW - USE HALF VENTED SOFFIT PANEL AT 0% FREE AREA  
 $106 \times 103 \text{ S.F.} = 11,019 \text{ S.F.}$

3.33 S.F. HIGH - USE RIDGE VENT = 1 FT. LENGTH = 20 SQ.IN. FREE VENT AREA  
24" VENT LENGTH X 20 SQ.IN. = 3.33 S.F.

EXCEEDS REQUIREMENT

ATTIC ACCESS REQ'D: NOT REQ'D

4:12 SHINGLE ROOF:  
1 LAYERS UNDERLAYMENT REQUIRED

#### FINISH LEGEND

##### FLOOR FINISHES

WALK-OFF CARPET

(SC1) TYPE: SEALED CONCRETE  
LOCATION: STORAGE SPACES  
NOTE: SEE SPECIFICATION

#### FINISH TAG KEY

(XXX) FLOOR FINISH

(X) WALL FINISH

##### WALL FINISHES

EPOXY PAINT

(EP) MFR: SHERWIN WILLIAMS  
COLOR: SW7086 RESERVED WHITE  
NOTE: GENERAL

#### ELEVATION NOTES

- STRUCTURAL BRICK - TYPE 1, SEE SPECS.
- STRUCTURAL BRICK - TYPE 2, SEE SPECS.
- PRE-MANUFACTURED ALUMINUM GUTTER
- PRE-MANUFACTURED ALUMINUM DOWNSPOUT
- PRE-MANUFACTURED FASCIA BOARD
- CONCRETE BOLLARD, PAINT
- ASPHALT SHINGLE ROOFING SYSTEM.

#### GENERAL NOTES

- COORDINATE THE WORK OF EACH TRADE WITH THE WORK OF OTHER TRADES.
- ALL WORK IS TO BE COMPLETED IN STRICT ACCORDANCE WITH ALL APPLICABLE CODES, ORDINANCES, RULES, REGULATIONS AND STANDARDS INCLUDING, BUT NOT LIMITED TO THOSE LISTED ON THE COVER SHEET. ALL APPLICABLE RULES & REGULATIONS ARE TO BE THE MOST CURRENT ADOPTED EDITIONS.
- FIELD VERIFY EXISTING CONDITIONS AND DIMENSIONS PRIOR TO THE COMMENCEMENT OF WORK. DISCREPANCIES BETWEEN THE DOCUMENTS AND THE ACTUAL CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT PRIOR TO THE COMMENCEMENT OF WORK.
- ALL DIMENSIONS ARE FROM CENTERLINE OF STRUCTURE. FINISH FACE OF WALL, FACE OF MASONRY, OR FACE OF EXISTING.
- ANY DIMENSIONS NOT SHOWN OR DEEMED QUESTIONABLE ARE TO BE VERIFIED BY ARCHITECT. DO NOT SCALE DRAWINGS.
- REFER TO WALL TYPE SCHEDULE, SHEET A200, TO DETERMINE WHICH WALLS EXTEND TO DECK. SEE STRUCTURAL FOR TOP SUPPORT DETAIL. WHERE METAL STUDS EXTEND TO DECK, PROVIDE SLIP CONNECTIONS FOR ROOF FLOOR DEFLECTION.
- ALL STEEL STUDS ARE TO BE BRACED ACCORDING TO MANUFACTURER LIMIT HEIGHT (L240).
- WHERE INSULATED OR SOUND WALLS EXTEND TO DECK, FILL DECK FLUTES WITH INSULATION SOUND ATTENUATION.
- REFER TO PLUMBING PLANS FOR LOCATION OF FLOOR DRAINS.
- WHERE ACCESS PANELS ARE SHOWN IN TOILET ROOM CHASES, FINAL LOCATION SHALL BE COORDINATED WITH OTHER TRADES PRIOR TO INSTALLATION.
- ALL CONCRETE MASONRY UNITS (CMU) SHALL BE LAID RUNNING BOND UNLESS CMU WALLS THAT DO NOT LAY OUT IN FULL OR HALF LENGTHS SHOULD BE BALANCED SO AS NOT TO HAVE ANY PIECES LESS THAN 4" IN SIZE EXPOSED TO VIEW.
- ALL INTERIOR MASONRY WALLS THAT RUN TO UNDERSIDE OF DECK ABOVE SHALL HAVE A 2" JOINT (U.N.O.) AT THE DECK TO BE FILLED WITH FIRE STOPPING AT RATED WALLS PER PROJECT MANUAL AND MINERAL WOOL AT THE NON-RATED WALLS TO ALLOW FOR DEFLECTION.
- THERE SHALL BE PERIMETER INSULATION CONTINUOUS AROUND THE ENTIRE PERIMETER OF THE BUILDING EXTENDING 2'-0" MINIMUM (R-15 MIN.) HORIZONTAL.
- PROVIDE MISCELLANEOUS SUPPORT FOR ALL CEILING SUSPENDED ITEMS. DOOR AND FRAME NUMBERS CORRESPOND TO ROOM NUMBERS, WHERE MORE THAN ONE DOOR OCCURS IN A ROOM, A SUFFIX HAS BEEN ADDED (E.G. A100-1). SEE A600 SERIES DRAWINGS FOR DOOR SCHEDULE AND DETAILS.
- ALL DOOR FRAMES SHALL BE LOCATED 4" OFF FINISH WALLS OR 4" OFF MASONRY WALLS UNLESS NOTED OTHERWISE.
- ALL GLASS AT INTERIOR DOOR FRAMES, DOOR LITES AND WINDOW FRAMES IS TO BE 1/4" CLEAR TEMPERED GLASS UNLESS NOTED OTHERWISE.
- AT BUILDING EXPANSION JOINTS, ALL PARTITIONS, CEILINGS, FLOORS AND ALL WALL, FLOOR OR CEILING MOUNTED ITEMS SHALL BE ANCHORED TO THE BUILDING STRUCTURE ON ONLY ONE SIDE OF THE EXPANSION JOINTS. CONTRACTOR SHALL COORDINATE CONSTRUCTION OR INSTALLATION OF ALL ITEMS NOTED TO ASSURE THAT NO SUCH ITEMS BRIDGE ACROSS THE EXPANSION JOINT.
- ALL SLAB ON GRADE CONTROL JOINTS TO BE CLEANED AND CAULKED PRIOR TO PLACEMENT OF FLOOR FINISH.
- SEE REFLECTED CEILING PLANS FOR BULKHEAD LOCATIONS AND DETAILS.
- REFER TO MECHANICAL DRAWINGS FOR WALL LOWER LOCATIONS, SIZES AND QUANTITIES.
- SEE A600 SERIES DRAWINGS FOR FINISH SCHEDULE AND PLANS.
- SEE A600 SERIES DRAWINGS FOR EQUIPMENT SCHEDULE AND PLANS.
- PROVIDE BLOCKING IN STUD WALLS AND/OR GROUTED MASONRY CORES AS REQUIRED TO SUPPORT EQUIPMENT.
- PROVIDE FIRE RESISTANT TREATED WOOD BLOCKING SUPPORTS AS REQUIRED FOR ALL SURFACE MOUNTED ITEMS.
- WHERE DISSIMILAR FLOOR MATERIALS MEET, THEY SHALL DO SO UNDER THE CENTERLINE OF THE DOOR UNLESS NOTED OTHERWISE.
- APPLY SEALANT AT ALL JUNCTURES BETWEEN DIFFERENT MATERIALS (E.G. MASONRY TO GYPSUM WALL BOARD) UTILIZING THE APPROPRIATE TYPE PER SPECIFICATIONS. COLOR TO BE SELECTED BY ARCHITECT.
- APPLY SEALANT AT ALL COUNTERTOPS AND BACKSPASHES AT JUNCTURE WITH WALL.
- ALL DOORS MUST BE INSTALLED WITH AT LEAST THE MINIMUM MANEUVERING CLEARANCE AT THE DOOR APPROACH PER THE MOST CURRENT AMERICANS WITH DISABILITIES ACT.
- BASE FLOOR ELEVATION INDICATED FOR THIS PROJECT IS 100'-0". REFER TO SITE PLAN FOR CORRELATION TO UGSS DATUM.

#### PLAN NOTES

- ALIGN CONCRETE BOLLARD FLUSH WITH DOOR JAMB. SEE SECTION A4401 FOR ADDITIONAL INFORMATION.

#### ROOF ABBREVIATIONS

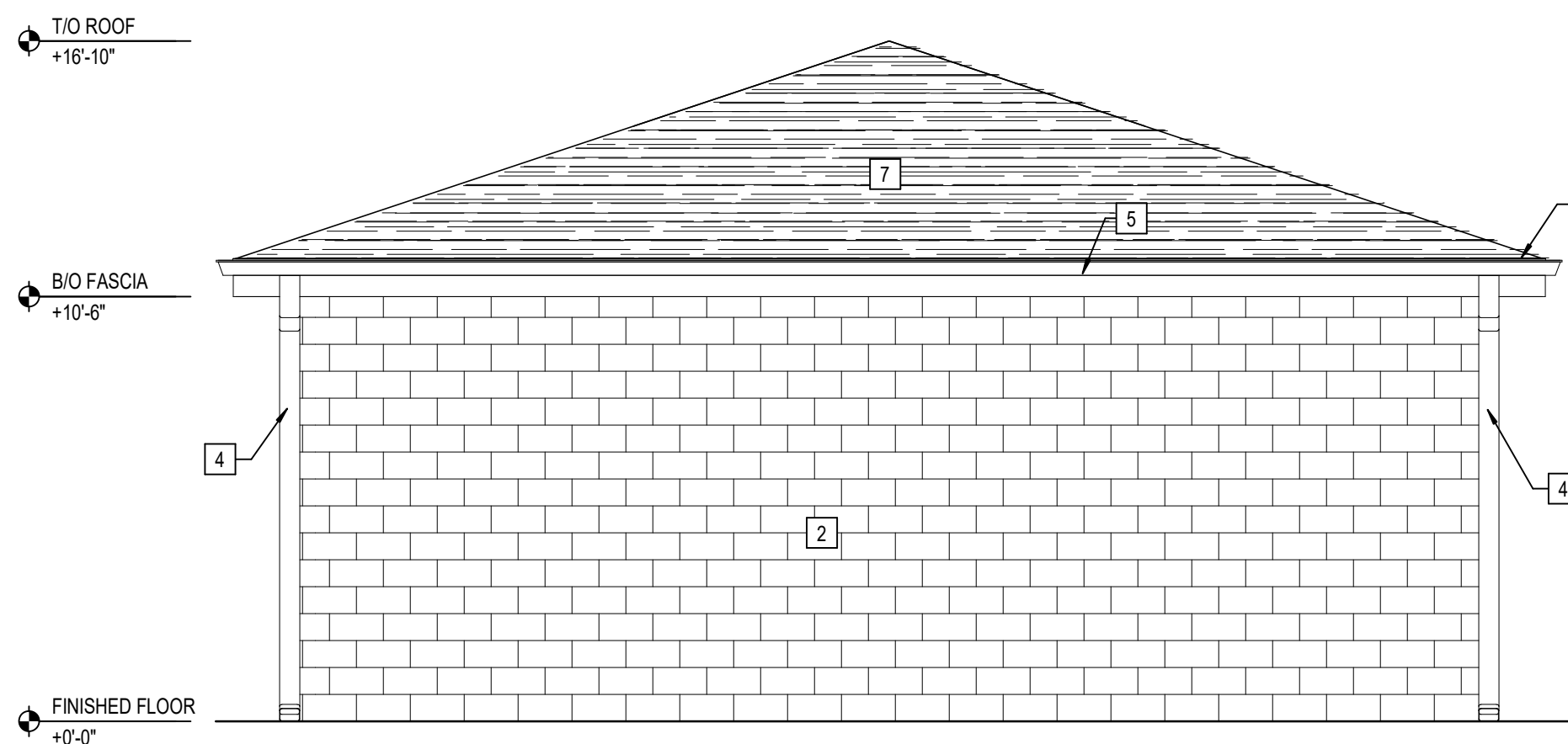
DR DUCT THRU ROOF  
DS METAL DOWNSPOUT  
EF EXHAUST FAN, SEE MECHANICAL  
EJ EXPANSION JOINT  
FS FLUE STACK, SEE MECHANICAL  
GU METAL GUTTER  
PV PLUMBING VENT  
RAV RELIEF AIR VENT, SEE MECHANICAL  
RH ROOF HATCH

#### GENERAL ROOF NOTES

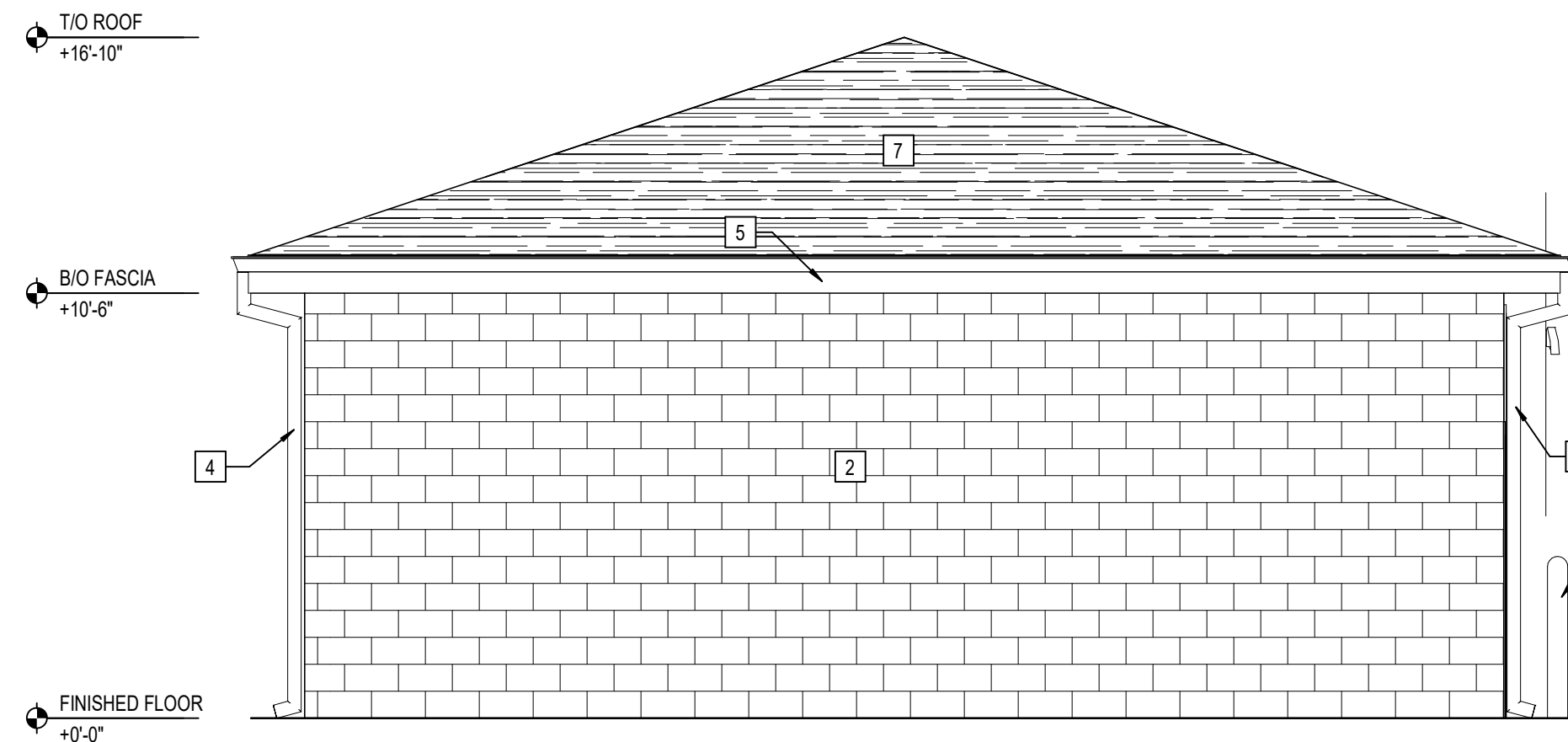
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- CONTRACT DOCUMENTS CONSIST OF BOTH THE PROJECT MANUAL AND DRAWINGS, AND BOTH ARE INTENDED TO BE COMPLEMENTARY. ANYTHING APPEARING ON EITHER MUST BE EXECUTED THE SAME AS IF SHOWN ON BOTH.
- CONSTRUCTION DOCUMENTS SHALL BE FOLLOWED AS CLOSELY AS POSSIBLE. HOWEVER, SYSTEMS HAVE BEEN SHOWN DIAGRAMMATICALLY AND IN SOME CASES, ENLARGED FOR CLARITY. PROVIDE ADDITIONAL ITEMS AS REQUIRED TO PROVIDE A COMPLETE AND COORDINATED SYSTEM.
- CONTRACTOR SHALL PROVIDE ANY AND ALL TEMPORARY UTILITY SERVICE REQUIRED TO CONSTRUCT THE WORK. CONTRACTOR MAY EXTEND SERVICES FROM EXISTING LOCATIONS TO WHERE THEY ARE REQUIRED. REMOVE TEMPORARY UTILITIES AND RELATED EXTENSIONS AS SOON AS PRACTICABLE. RESTORE ALL AFFECTED AREAS TO ORIGINAL CONDITION.
- CONTRACTOR SHALL REMOVE CONSTRUCTION DEBRIS FROM THE BUILDING AND ROOF DAILY.
- STORE VOLATILE OR FLAMMABLE LIQUIDS IN UL LISTED FIRE CABINETS.
- CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE SECURITY OF ALL STORED MATERIALS AND EQUIPMENT INSIDE OR OUTSIDE THE BUILDING.
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- PROVIDE TREATED WOOD BLOCKING EQUAL IN THICKNESS TO INSULATION SYSTEM AT ROOF PERIMETER AND AROUND ALL ROOF PENETRATIONS. ANCHOR PER SECTION 445 OF THE FM GLOBAL LOSS PREVENTION GUIDE.
- EXTEND ALL PLUMBING VENTS TO PROVIDE A MIN. OF 12" OF HEIGHT FROM TOP OF INSULATION. ALL FITTINGS TO BE AIR AND WATER TIGHT. SEE PLUMBING PLANS FOR LOCATIONS.
- ROOF INSULATION SADDLES AND CRICKETS ARE DIAGRAMMATIC. ROOF INSULATION MANUFACTURER SHALL DESIGN AND SIZE THESE PER THE ROOF MEMBRANE MANUFACTURERS RECOMMENDATIONS. CRICKETS AND SADDLES SHOULD HAVE A MINIMUM OF TWO TIMES THE SLOPE OF THE PRIMARY TAPERED SYSTEM OR STRUCTURAL SLOPE. THE RATIO OF A CRICKETS WIDTH TO LENGTH SHOULD BE NO LESS THAN 1 TO 3.
- PROVIDE SADDLES/CRICKETS AROUND ALL NEW ROOF TOP EQUIPMENT. SEE MECHANICAL, ELECTRICAL AND PLUMBING DRAWINGS FOR LOCATIONS OF ALL ROOF PENETRATIONS.
- PROVIDE TAPERED INSULATION WHERE REQUIRED TO TRANSITION FROM ONE INSULATION HEIGHT TO ANOTHER.
- NOTCH ALL INSULATION AS REQUIRED TO ACCOMMODATE SURFACE MOUNTED CONDUIT, FASTENERS, OFFSETS AND OTHER PROJECTIONS EXTENDING ABOVE THE SURFACE OF THE DECK.
- PERIMETER EDGE METAL TO COMPLY WITH ANSI/APRI ES-1 FM GLOBAL 1-49.
- SEE MECHANICAL, ELECTRICAL AND PLUMBING (MEP) SHEETS FOR ROOF TOP EQUIPMENT.
- INSPECT ALL WOOD BLOCKING SCHEDULED TO REMAIN. NOTIFY ARCHITECT OF ANY DETERIORATED BLOCKING NEEDING REPLACEMENT. CONTRACTOR TO REPLACE AND DAMAGED BLOCKING ON A TIME AND MATERIAL BASIS, SEE SPECIFICATIONS.
- ALL ROOF DETAIL DRAWINGS CONTAINED IN THIS SET ARE DIAGRAMMATIC. ADJUST ROOF DETAILS BASED ON SPECIFIC ROOFING SYSTEM SELECTED ACCORDING TO MANUFACTURERS WRITTEN SPECIFICATIONS AND APPROVED DETAIL DRAWINGS. ALL ASSEMBLY COMPLICATIONS SHOULD BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.
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- MECHANICAL, ELECTRICAL AND PLUMBING INFORMATION SHOWN ON THIS PLAN IS GENERAL IN NATURE. REFER TO M, P, N AND E DRAWINGS FOR FURTHER INFORMATION AND COORDINATE ALL REQUIRED ROOF OPENINGS OR ROOF MOUNTED EQUIPMENT.

#### ROOF PLAN NOTES

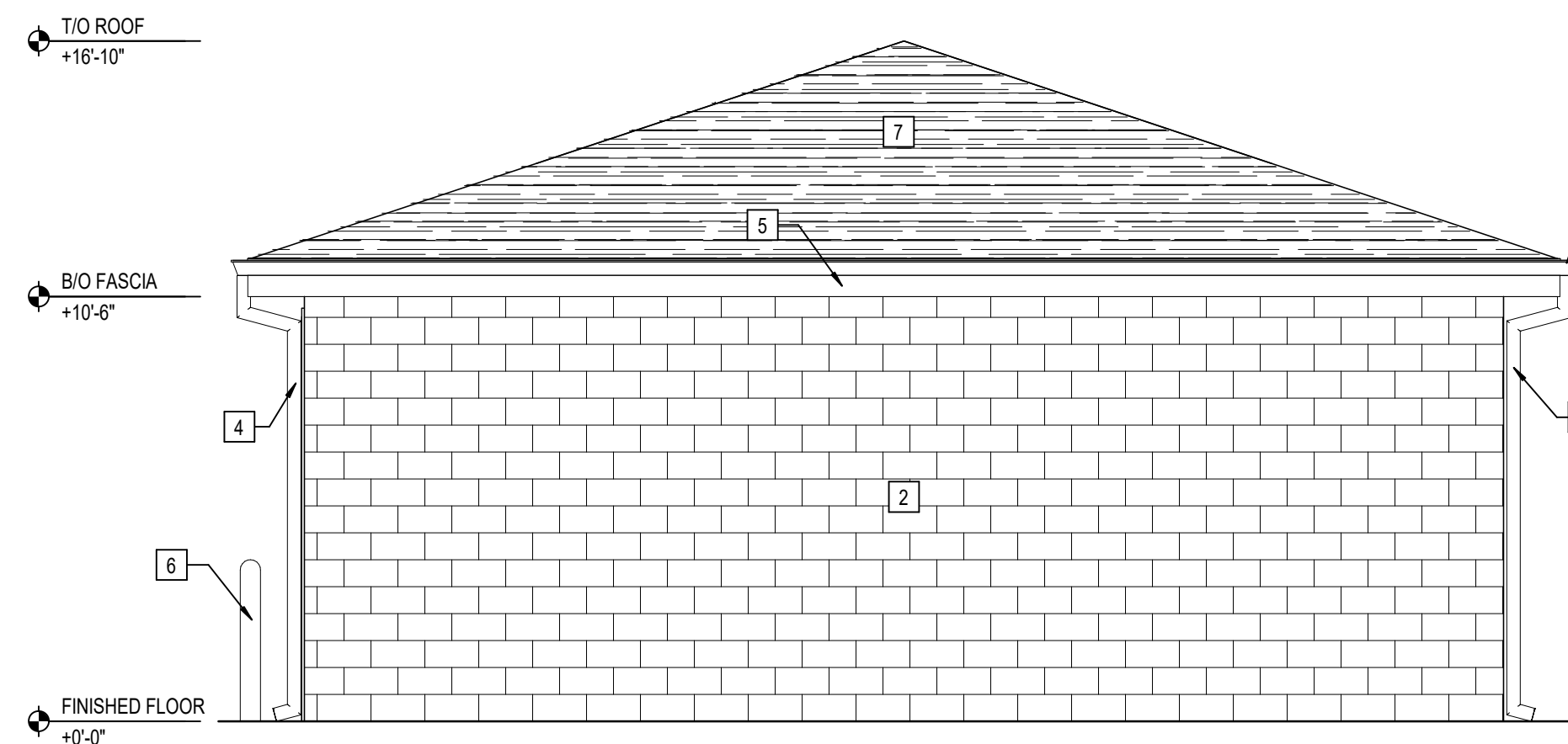
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- CONTINUOUS RIDGE VENT
- ALUMINUM GUTTER, TYP.
- LINE OF BUILDING BELOW



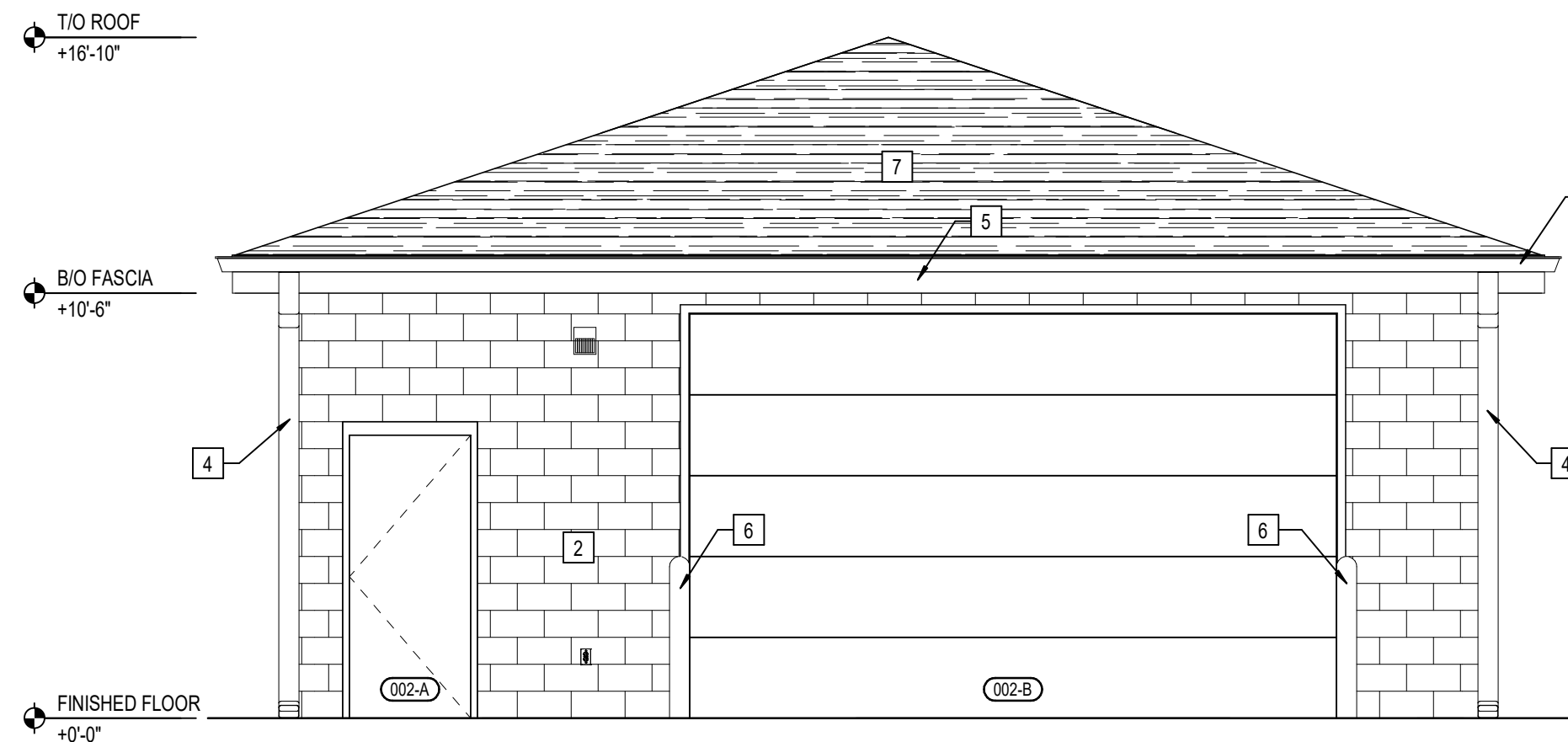
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A202 SCALE: 1/4" = 1'-0"



5 BRENTWOOD ELEVATION - SOUTH  
A202 SCALE: 1/4" = 1'-0"

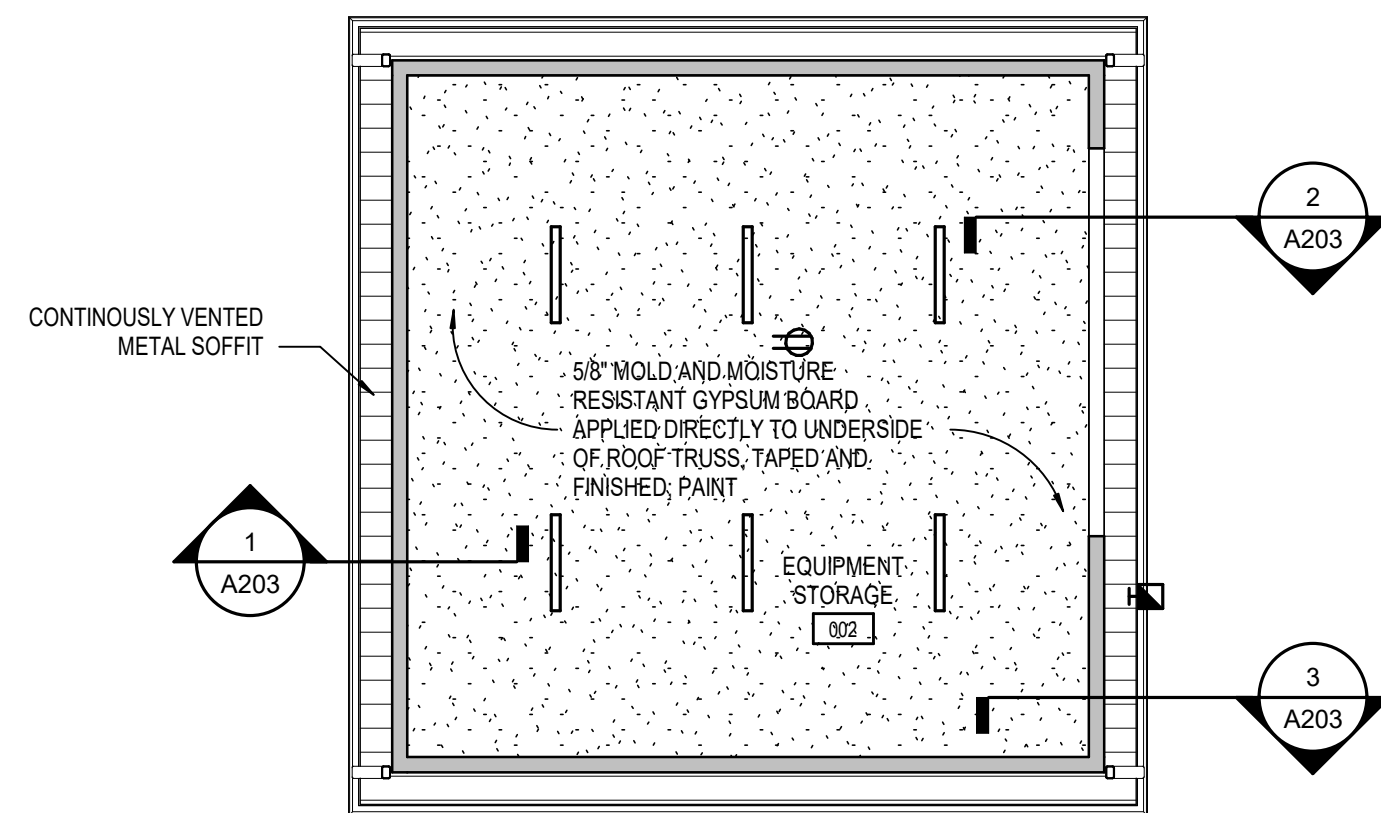


6 BRENTWOOD ELEVATION - NORTH  
A202 SCALE: 1/4" = 1'-0"

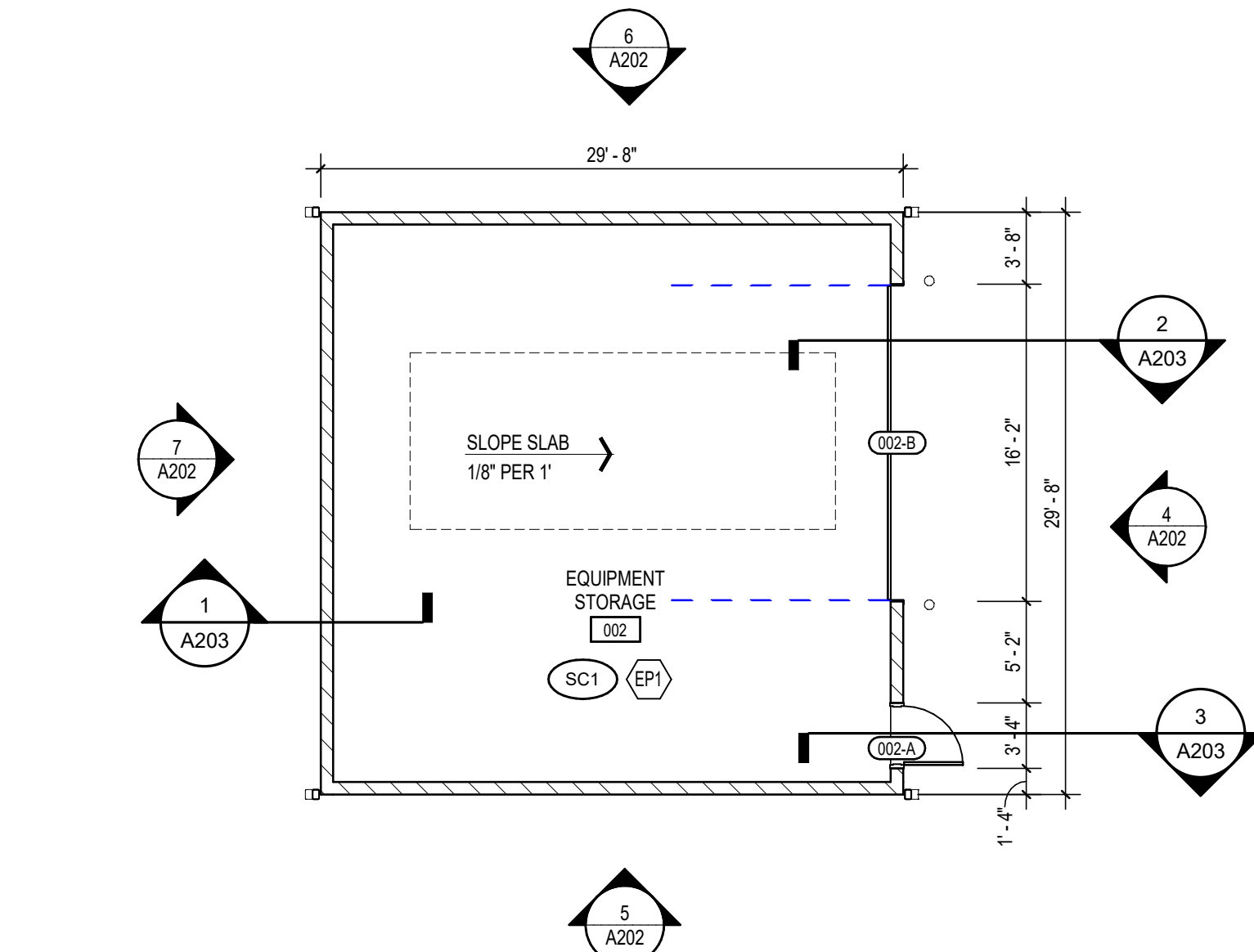


4 BRENTWOOD ELEVATION - EAST  
A202 SCALE: 1/4" = 1'-0"

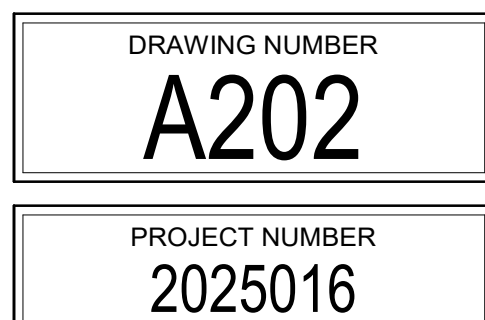
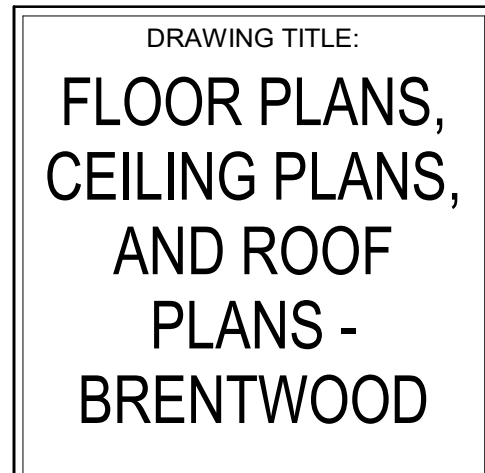
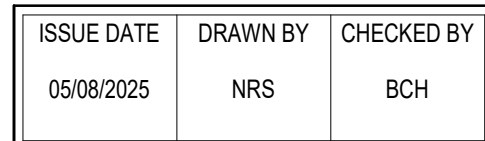
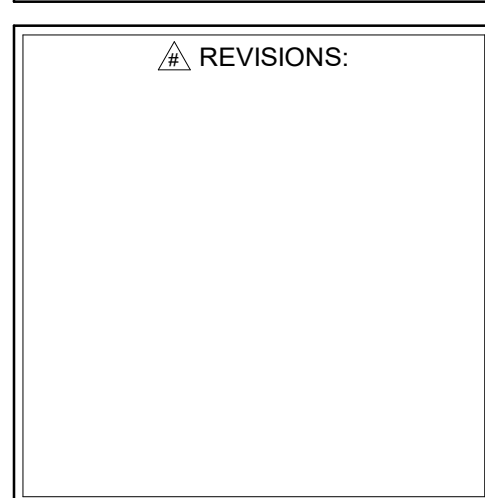
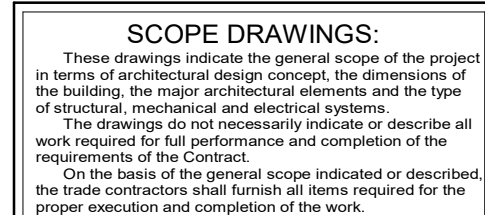
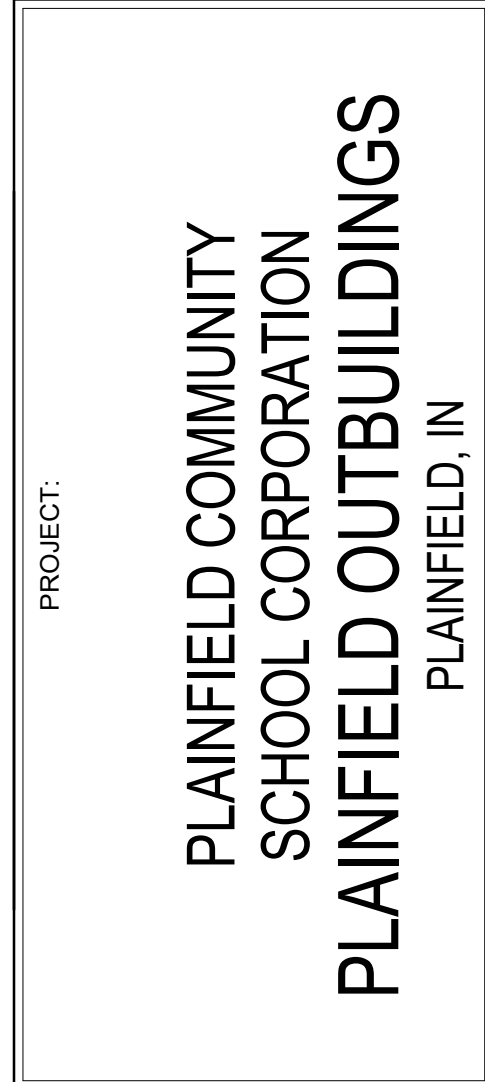
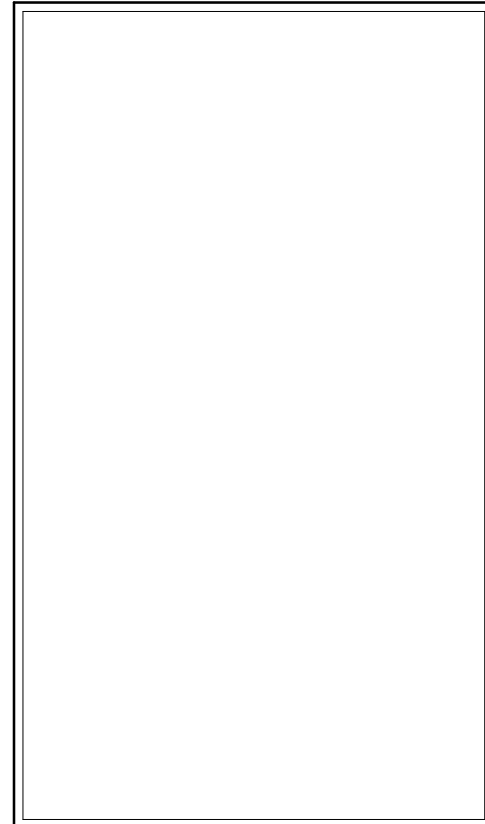
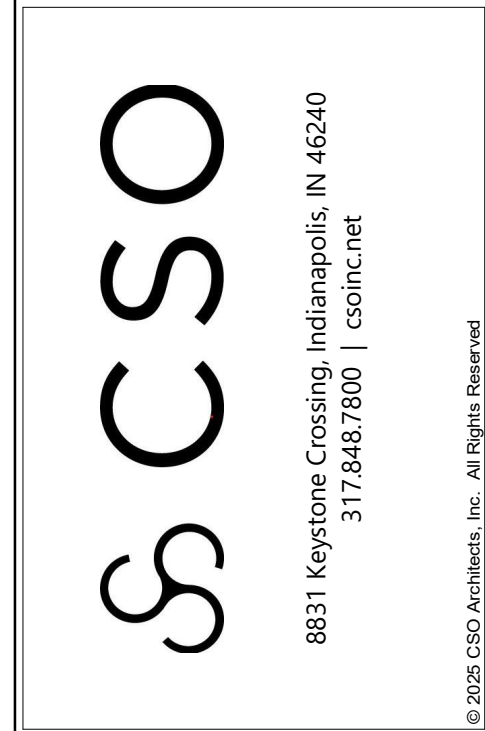
3 ROOF PLAN - BRENTWOOD OUTBUILDING  
A202 SCALE: 1/8" = 1'-0"



2 CEILING PLAN - BRENTWOOD OUTBUILDING  
A202 SCALE: 1/8" = 1'-0"

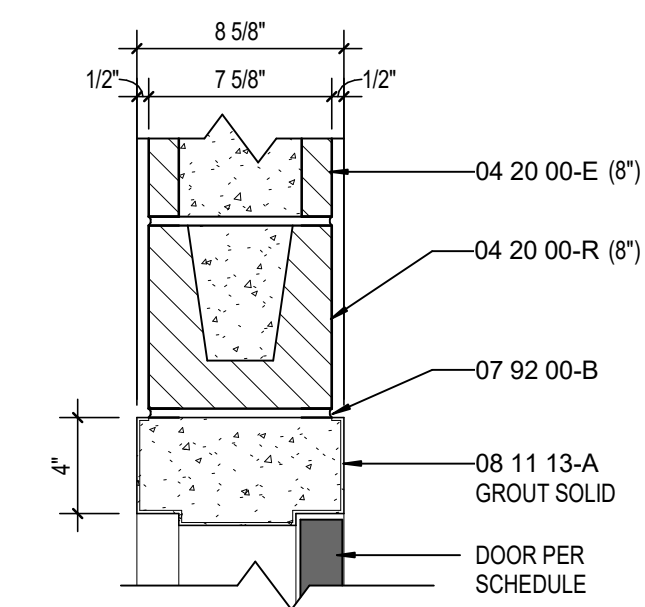


1 FLOOR PLAN - BRENTWOOD OUTBUILDING  
A202 SCALE: 1/8" = 1'-0"

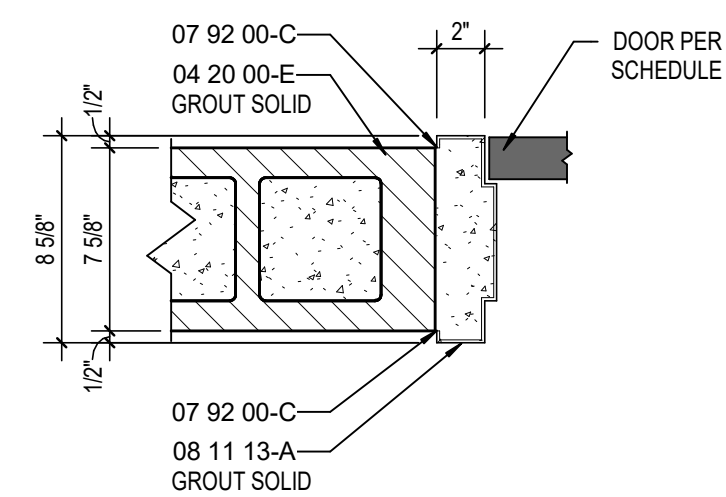




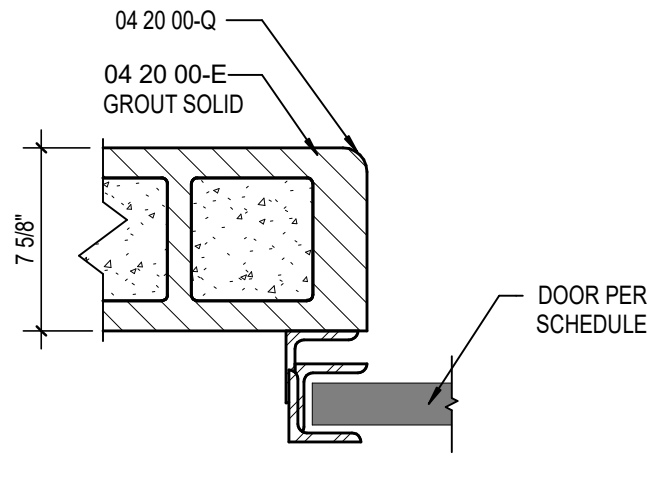
DOOR AND FRAME SCHEDULE																			
DOOR	MARK	SIZE			MATERIAL	FINISH	ELEVATION	FRAME				DETAIL				UL RATING	STC RATING	REMARKS	
		WIDTH	HEIGHT	THICKNESS				GLASS	MATERIAL	FINISH	ELEVATION	GLASS	HEAD	JAMB	SILL				
001-A	SGL	3'-0"	7'-0"	1 3/4"	HM	PT	D1	-	-	HM	PT	-	-	H1	J1	-	-	-	-
001-B	SGL	3'-0"	7'-0"	1 3/4"	HM	PT	D1	-	-	HM	PT	-	-	H1	J1	-	-	-	-
001-C	SOD	16'-0"	10'-0"	1 3/4"	ST	PF	D2	-	-	ST	PF	-	-	2/A401	J2	-	-	-	-
001-D	SOD	16'-0"	10'-0"	1 3/4"	ST	PF	D2	-	-	ST	PF	-	-	2/A401	J2	-	-	-	-
002-A	SGL	3'-0"	7'-0"	1 3/4"	HM	PT	D1	-	-	HM	PT	F1	-	H1	J1	-	-	-	-
002-B	SOD	16'-0"	10'-0"	1 3/4"	ST	PF	D2	-	-	ST	PF	-	-	2/A401	J2	-	-	-	-



HEAD DETAIL - H1  
SCALE: 1 1/2" = 1'-0"

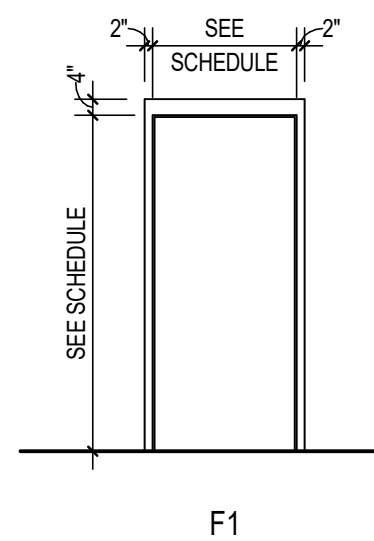


JAMB DETAIL - J1  
SCALE: 1 1/2" = 1'-0"

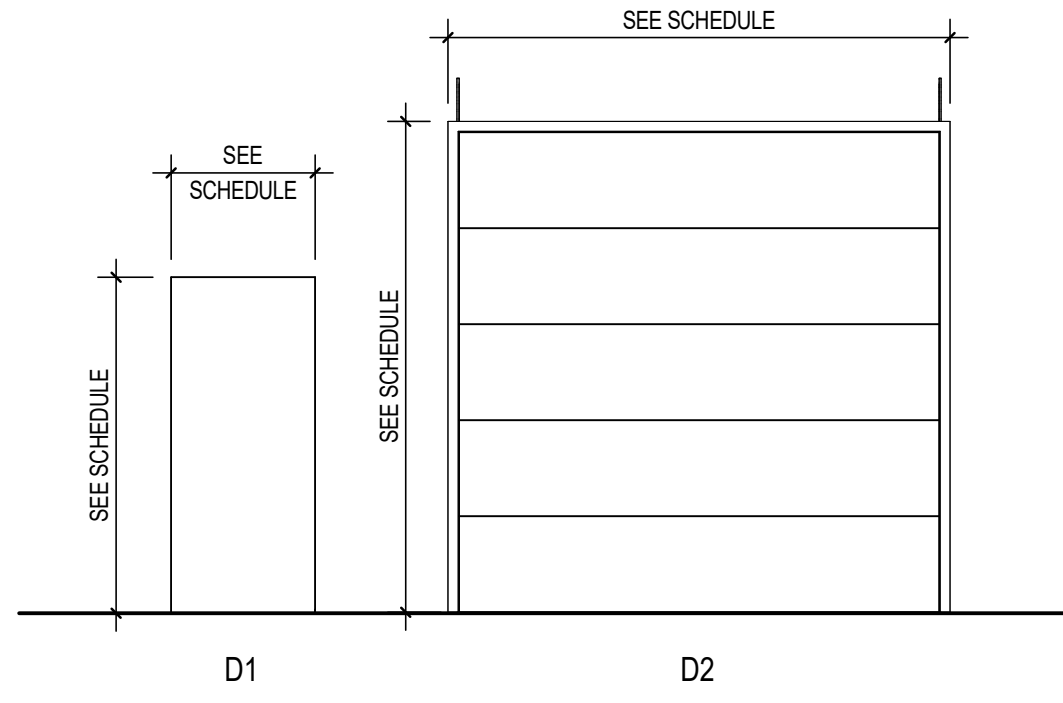


JAMB DETAIL - J2  
SCALE: 1 1/2" = 1'-0"

HARDWARE GROUP NO. 01					
FOR USE ON DOOR #(s):					
001-A	001-B	002-A			
PROVIDE EACH SGL DOOR(s) WITH THE FOLLOWING:					
QTY	QTY	DESCRIPTION	CATALOG NUMBER	FINISH	MFR
1	EA	CONT. HINGE	224XY	628	IVE
1	EA	STOREROOM LOCK	L960BDC 06A	626	SCH
1	EA	SFC CYLINDER	KEYED TO MATCH EXISTING SYSTEM	626	BES
1	EA	SURFACE CLOSER	4040P SCUSH ST-1595	689	LCN
1	EA	KICK PLATE	8400 10" X 1 1/2" LDW B-CS	630	IVE
1	EA	RAIN DRIP	142AA	AA	ZER
1	SET	WEATHERSTRIPPING	CATALOG NUMBER	AA	ZER
1	EA	DOOR SWEEP, BRUSH W/ DRIP	CATALOG NUMBER	AA	ZER
1	EA	THRESHOLD, 1/2"	CATALOG NUMBER	A	ZER



FRAME ELEVATIONS  
SCALE: 1/4" = 1'-0"



DOOR ELEVATIONS  
SCALE: 1/4" = 1'-0"

## KEYNOTE LEGEND

03 30 00-B	CONCRETE SLAB OVER VAPOR BARRIER ON DRAINAGE FILL. SEE STRUCTURAL.
03 30 00-C	CONCRETE FOUNDATION-SEE STRUCTURAL.
03 30 00-D	CONCRETE FOOTING- SEE STRUCTURAL.
04 20 00-E	STRUCTURAL BRICK
04 20 00-Q	BULLNOSE UNIT
04 20 00-R	BOND BEAM MASONRY UNIT
06 10 00-B	2X WOOD BLOCKING
06 16 00-A	3/4" EXTERIOR GRADE PLYWOOD
06 16 00-B	5/8" EXTERIOR GRADE PLYWOOD
07 31 13-A	ASPHALT SHINGLE ROOF
07 42 13-C	METAL WALL PANEL- SOFFIT
07 62 00-C	FASOIA CLADDING
07 62 00-N	PREFINISHED METAL DOWNSPOUT
07 71 00-E	MANUFACTURED GUTTER
07 92 00-B	SEALANT EACH SIDE, TYPICAL
07 92 00-C	BACKER ROD AND SEALANT
08 11 13-A	HOLLOW METAL DOORBORROWED LIGHT FRAME
09 29 00-B	5/8" GYPSUM WALL BOARD (SEE SPECS FOR TYPE)

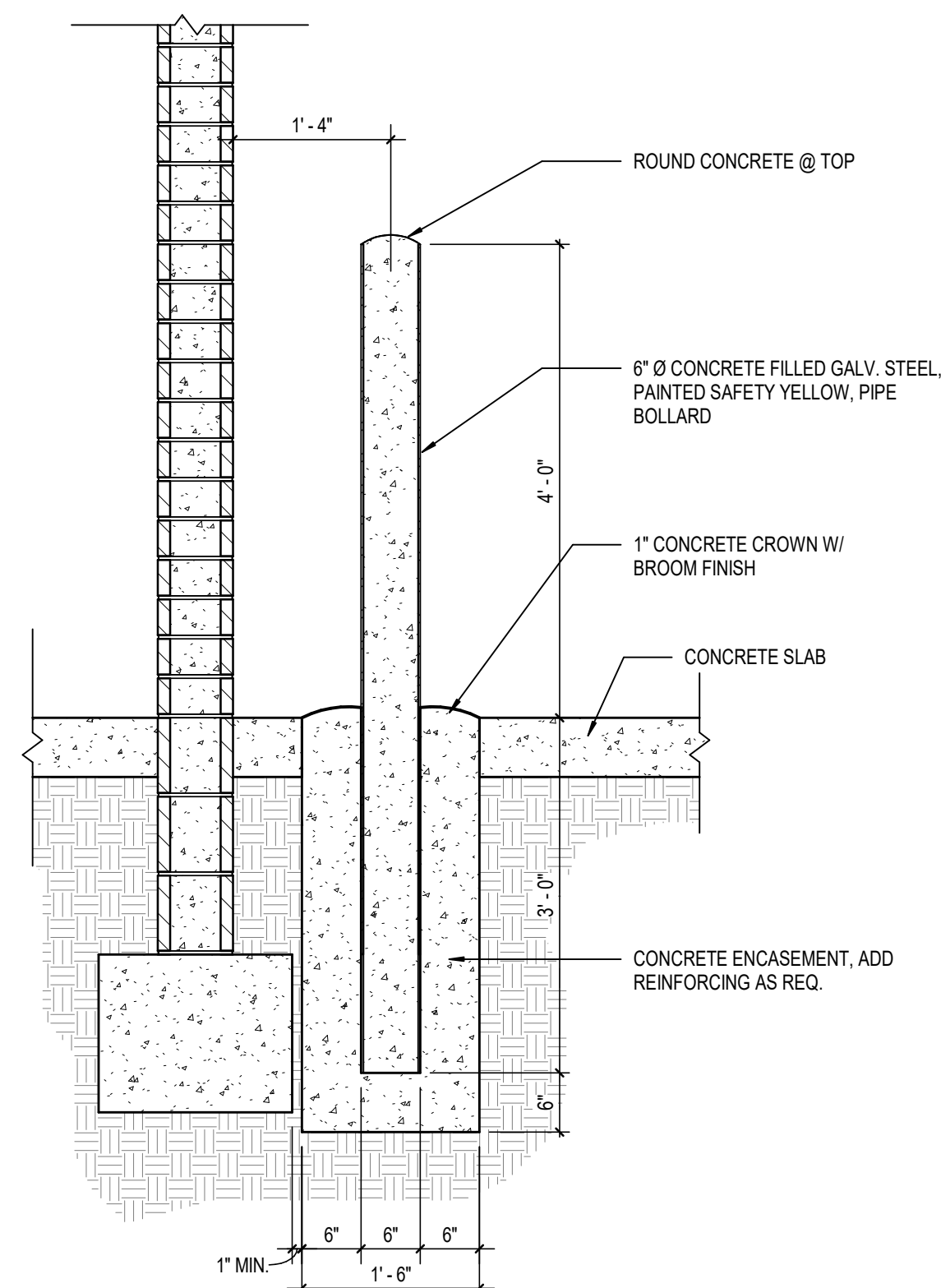
## ABBREVIATIONS LEGEND

AL	=	ALUMINUM
AN	=	ANODIZED
BL	=	BORROWED LITE
GHM	=	GALVANNEALED HOLLOW METAL
GL	=	GLASS
HM	=	HOLLOW METAL
PT	=	PAINT
ST	=	STAIN
SS	=	STAINLESS STEEL
STL	=	STEEL
WD	=	WOOD
90M	=	90 MINUTE ASSEMBLY RATING
*	=	SEE REMARKS COLUMN FOR NOTES

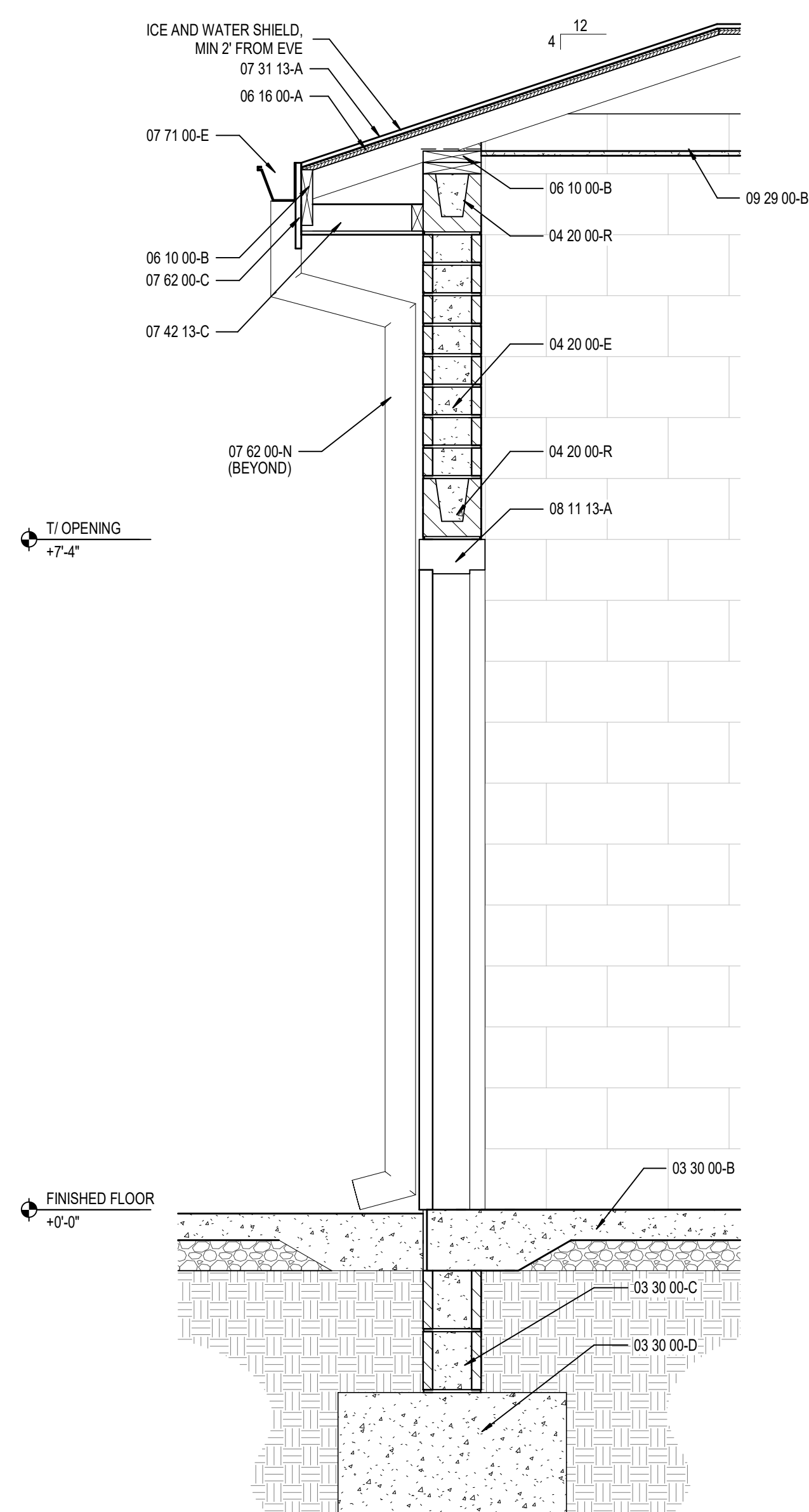
## GENERAL DOOR NOTES

- THESE GENERAL NOTES APPLY TO THE DOOR SCHEDULE.
- DOOR AND FRAME NUMBERS CORRESPOND TO RESPECTIVE ROOM NUMBER. IN ROOMS WITH MULTIPLE OPENINGS, A NUMERICAL SUFFIX HAS BEEN ADDED TO DOOR NUMBERS.
- VERTICAL FRAMING MEMBERS AT ALL DOOR FRAMES SHALL EXTEND TO STRUCTURE ABOVE.
- UNDERCUT ALL DOORS AS REQUIRED BY FINAL FINISH.
- PROVIDE CONTINUOUS SEALANT BETWEEN HOLLOW METAL FRAME PERIMETERS AND SURROUNDING WALL CONSTRUCTION.
- PROVIDE CONTINUOUS SEALANT BETWEEN INTERIOR AND EXTERIOR WINDOW, CURTAINWALL AND STOREFRONT FRAME PERIMETERS AND SURROUNDING CONSTRUCTION UNLESS NOTED OTHERWISE.
- GROUT FILL HOLLOW METAL FRAMES IN MASONRY CONSTRUCTION.
- SPOT GROUT HOLLOW METAL FRAMES IN GYPSUM WALLS.
- WHERE A FIRE RATING IS INDICATED ON THE DOOR SCHEDULE, HARDWARE AND DOOR ASSEMBLY COMPONENTS SHALL MEET THE REQUIREMENTS OF THAT LABEL.
- WHERE AN STC RATING IS INDICATED ON THE DOOR SCHEDULE, HARDWARE AND DOOR ASSEMBLY COMPONENTS SHALL MEET THE REQUIREMENTS OF THAT LABEL.
- INSTALL DOOR GLASS USING WET GLAZING METHOD.
- ALL LITELS ABOVE EXTERIOR OPENINGS SHALL BE GALVANIZED.
- REFER TO SHEETS AXXX & AXXX FOR ADDITIONAL DOOR, FRAME AND BORROWED LITE ELEVATIONS.
- COORDINATE THROAT OPENINGS WITH WALL WIDTH FOR ALL WRAP AROUND FRAMES.
- SCHEDULED HARDWARE FOR ALUMINUM DOORS SHALL BE PROVIDED BY HARDWARE SUPPLIER AND INSTALLED BY ALUMINUM SUPPLIER. ALUMINUM DOORS TO BE PREPARED BY ALUMINUM DOOR SUPPLIER IN ACCORDANCE WITH THE SCHEDULED HARDWARE.
- ALL NEW HOLLOW METAL DOORS, FRAMES AND BORROWED LITE FRAMES TO BE PAINTED AS INDICATED ON THE A800 SERIES FINISH PLANS. SEE FINISH PLANS FOR WOOD DOOR FINISHES.
- PROVIDE SILENCERS ON ALL DOOR FRAMES.
- SEE STRUCTURAL DRAWINGS FOR REQUIREMENTS FOR MASONRY AND STEEL LITELS. PROVIDE STRUCTURAL STEEL LITELS AT OPENINGS OPENINGS WHERE INDICATED ON THE STRUCTURAL STEEL DRAWINGS IN LIEU OF MASONRY LITELS AS SHOWN IN THESE DETAILS.
- VERIFY ALL EXISTING CONDITIONS AND DIMENSIONS IN THE FIELD PRIOR TO FABRICATION OF DOORS AND FRAMES. BRING DISCREPANCIES TO THE ATTENTION OF THE ARCHITECT.

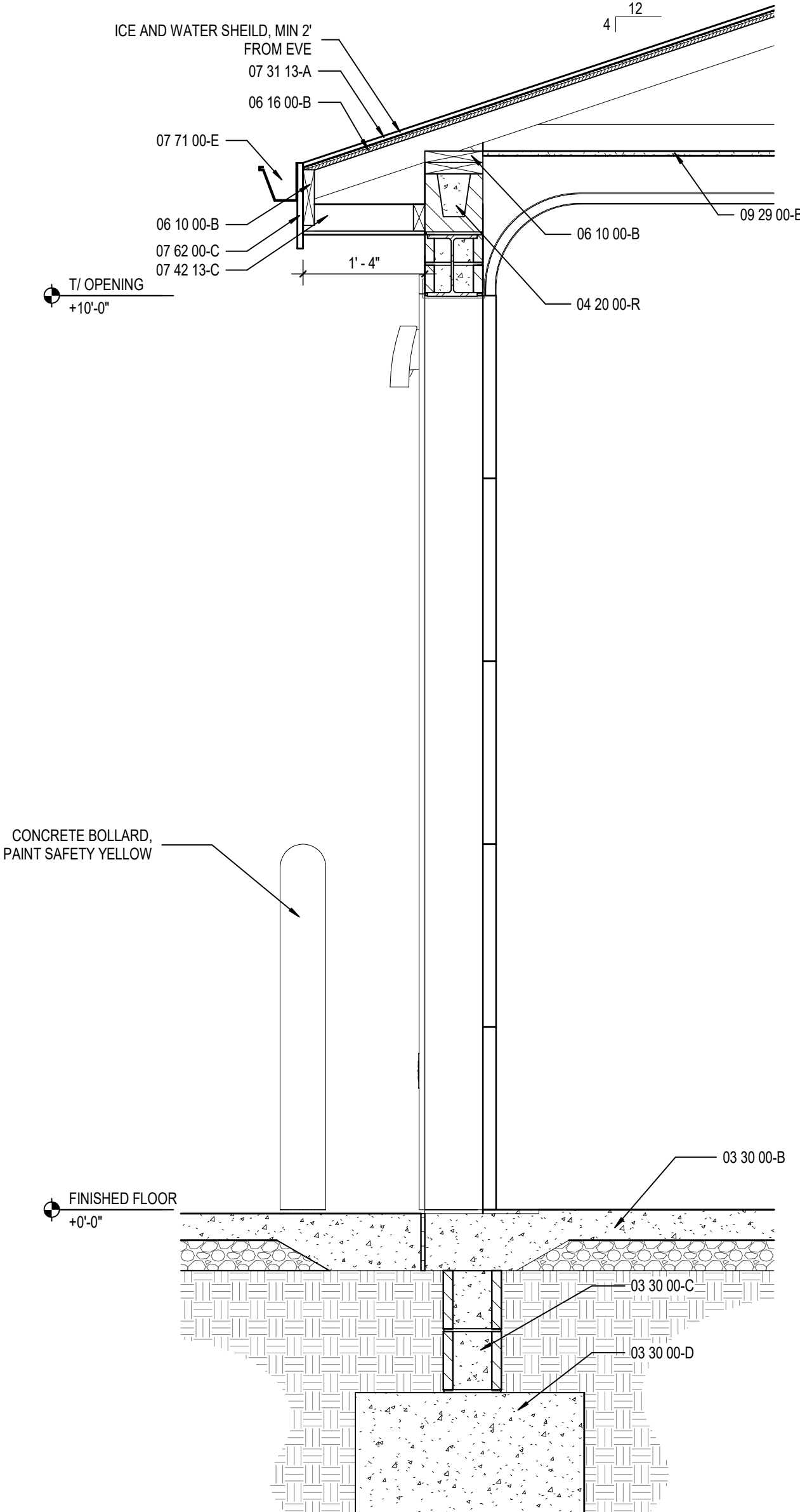
## DOOR NOTES



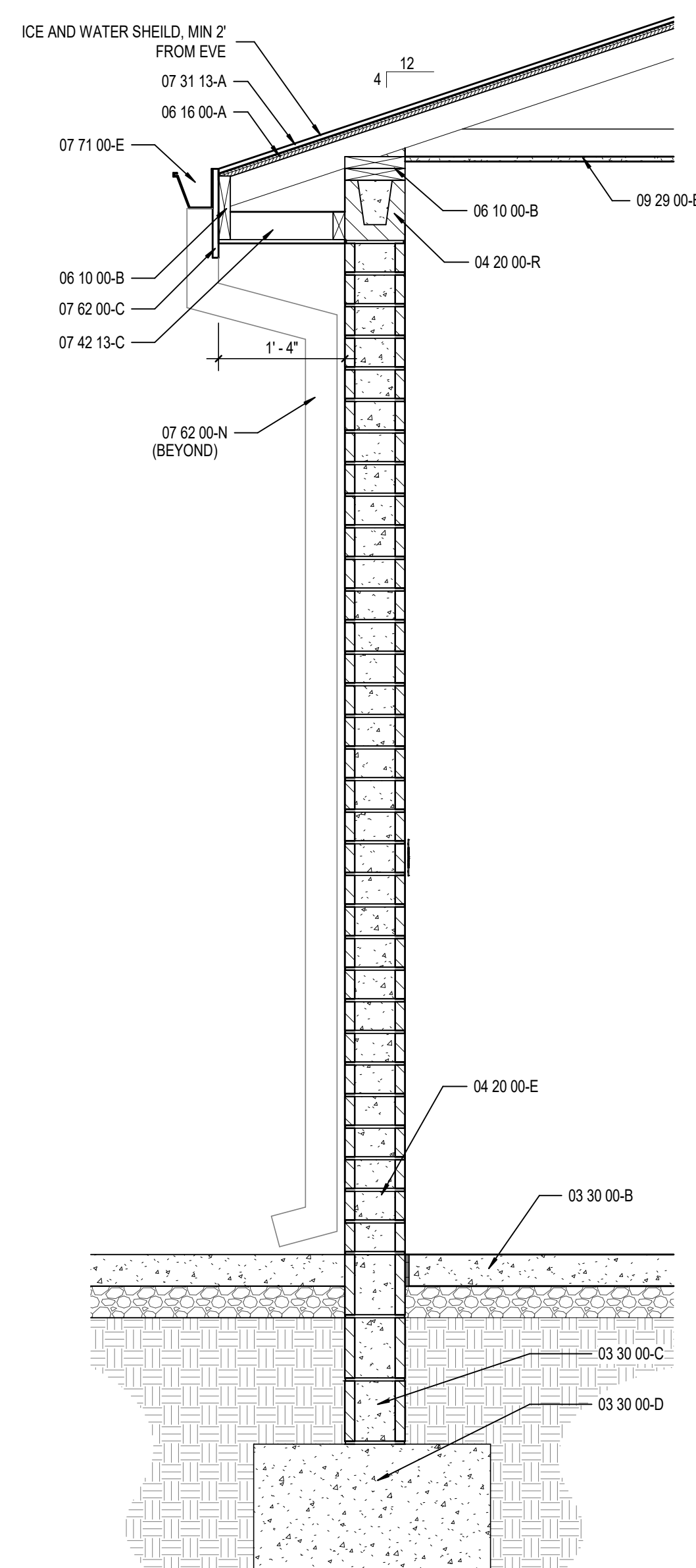
4  
A203  
EXTERIOR PIPE BOLLARD  
SCALE: 3/4" = 1'-0"



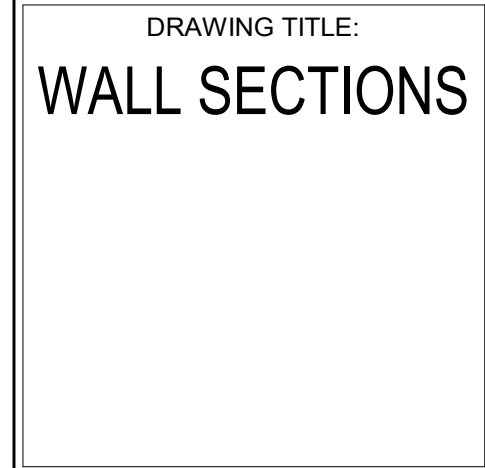
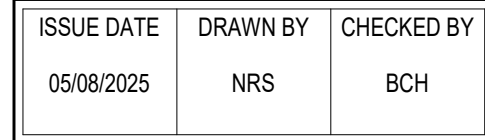
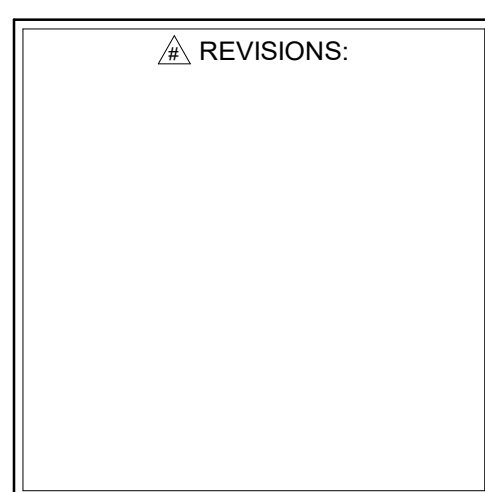
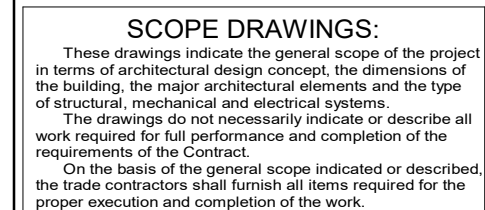
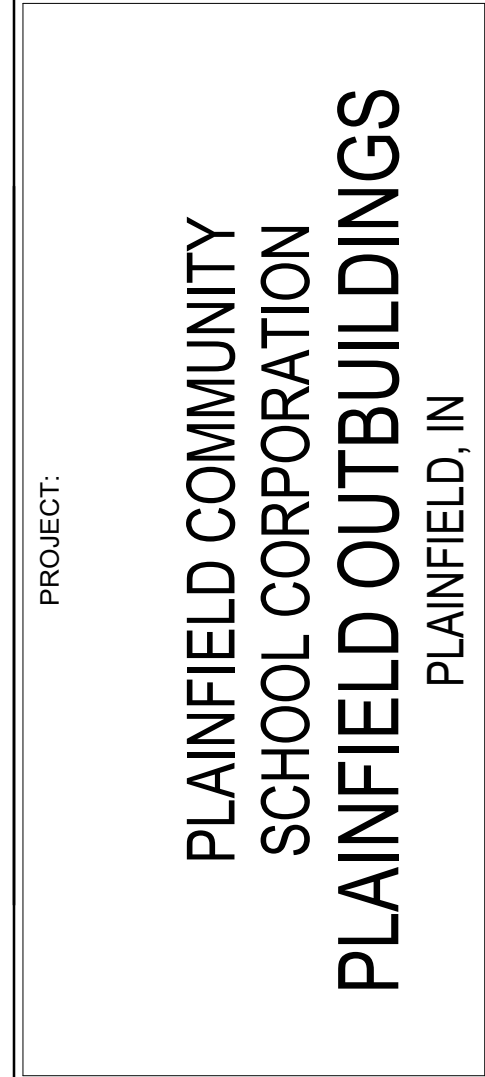
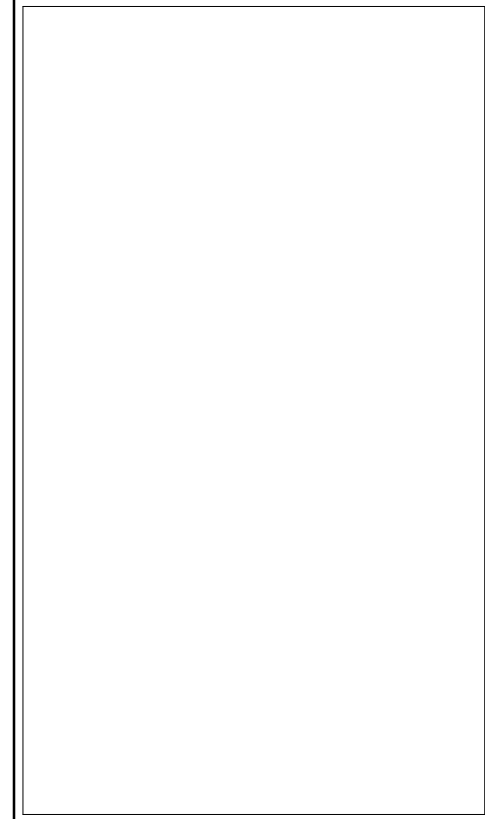
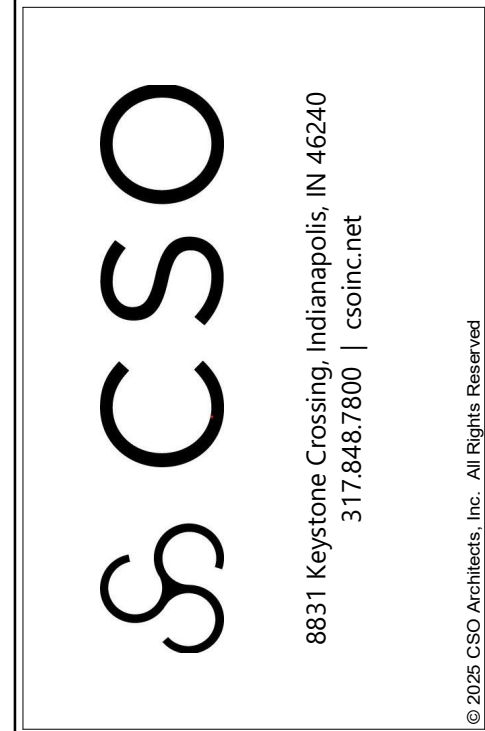
3  
A203  
WALL SECTION  
SCALE: 3/4" = 1'-0"



2  
A203  
WALL SECTION  
SCALE: 3/4" = 1'-0"



1  
A203  
WALL SECTION  
SCALE: 3/4" = 1'-0"



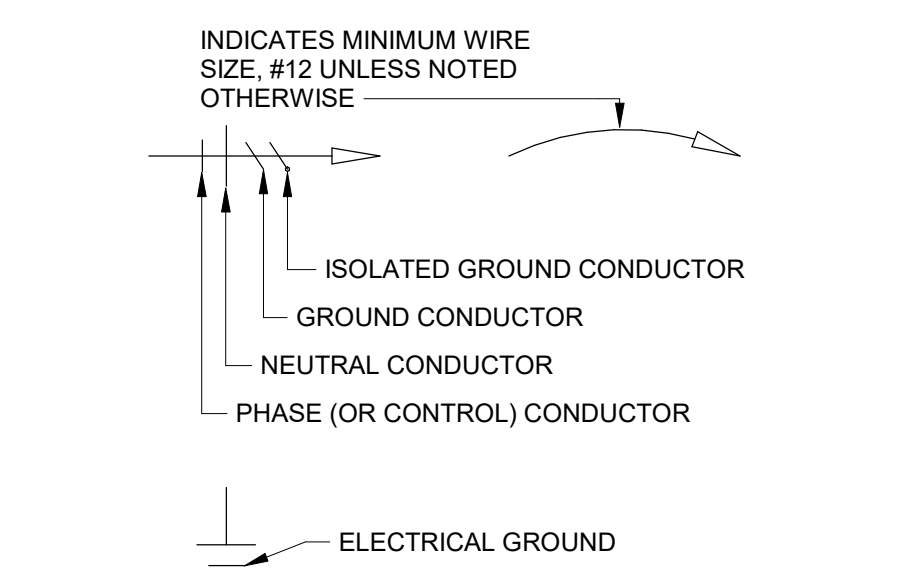


ABBREVIATIONS

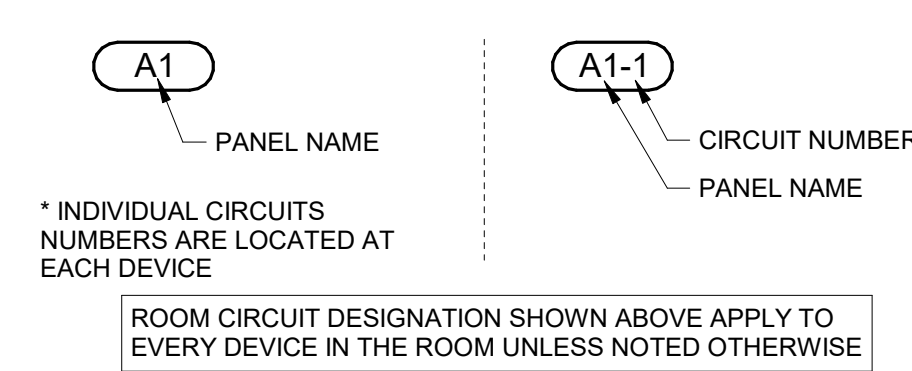
A	AMPERE	MDF	MAIN DISTRIBUTION FRAME
AC	ALTERNATING CURRENT; ARMORED CABLE	MDP	MAIN DISTRIBUTION PANELBOARD
ADJ	ADJUSTABLE	ME	MEDIUM
AF	AMPERE FUSE; AMPERE FRAME	MFG	MANUFACTURING
AFB	ABOVE FINISHED FLOOR	MFR	MANUFACTURER
AFG	ABOVE FINISHED GRADE	MH	MANHOLE; METAL HALIDE; MAN-HOUR
AL	ALUMINUM	MHZ	MEGAHERTZ
ALCR	AUTOMATIC LOAD CONTROL RELAY	MI	MINERAL INSULATED
ANSI	AMERICAN NATIONAL STANDARDS INSTITUTE	MIC	MICROPHONE
ASYM	ASYMMETRICAL	MIN	MINIMUM; MINUTE
AT	AMPERE TRIP	MISC	MISCELLANEOUS
ATS	AUTOMATIC TRANSFER SWITCH	MLO	MAIN LUG ONLY
AUX	AUXILIARY	MOC	MODUL
AWG	AVERAGE	MTD	MOUNTED
BATT	BATTERY	MTS	MANUAL TRANSFER SWITCH
BPS	BOLTED PRESSURE SWITCH	MV	MEGAVOLT; MEDIUM VOLTAGE
C	CONDUIT; CENTRIGRADE	MVA	MEGAVOLT AMPERES
CB	CIRCUIT BREAKER	MVAR	MEGAVOLT AMPERES REACTIVE
CCTV	CLOSED CIRCUIT TELEVISION	MW	MEGAWATT
CD	CANDELA	N	NEUTRAL
CF	CUBIC FEET	N/A	NOT APPLICABLE
CFL	COMPACT FLUORESCENT	N/C	NORMALLY CLOSED
CIRC	CIRCUIT	NEC	NATIONAL ELECTRICAL CODE
CLO	CLOSED	NEHA	NATIONAL ELECTRICAL
CMU	CONCRETE MASONRY UNIT	NEMA	MANUFACTURERS ASSOCIATION
COL	COLUMN	NF	NOT FUSED
COMB	COMBINATION	NFS	NON-FUSED DISCONNECT
CONC	CONCRETE	NIC	NOT IN CONTRACT
COND	CONDUCTOR	NO	NIGHT LIGHT
CONT	CONTINUOUS; CONTINUED	NM	NON-METALLIC SHEATHED CABLE
CP	CONTROL PANEL	NO	NUMBER; NORMALLY OPEN
CT	CURRENT TRANSFORMER	NTS	NOT TO SCALE
CU	COPPER	O&P	OVERHEAD AND PROFIT
CU FT	CUBIC FOOT	OC	ON CENTER; OVERCURRENT
CY	CUBIC YARD	OH	OVERHEAD
CYL	CYLINDER	OL	OVERSIZE
D	DEEP; DEPTH	OS&Y	OUTSIDE SCREW AND YOKE
DB	DECEBEL; DIRECT BURIED	OZ	OUNCE
DC	DIRECT CURRENT	P	POLE; PULL
DDC	DIRECT DIGITAL CONTROL	PA	PUBLIC ADDRESS
DF	DUAL FACE	PB	PUSH BUTTON; PULL BOX
DIA	DIAMETER	PC	PHOTOCELL
DIAG	DIAGONAL	PED	PEDESTAL
DISC	DISCONNECT	PF	POWER FACTOR
DISTR	DISTRIBUTION	PH	PHASE
DN	DOWN	PIV	PISTON INDICATOR VALVE
DPT	DOUBLE POLE; DOUBLE THROW	PL	PILOT LIGHT
DPST	DOUBLE POLE, SINGLE THROW	PNL	PANEL
DWG	DRAWING	PNR	PANEL
DX	DIRECT EXPANSION	PR	PRIMARY
E	EAST; EXISTING	PRF	POUNDS PER SQUARE FOOT
EA	ELECTRIC BASEBOARD RADIATION	PS	POUNDS PER SQUARE INCH
EBR	ELECTRONIC BALLAST	PSG	POUNDS PER SQUARE INCH GUAGE
EC	ELECTRICAL CONTRACTOR	PT	POTENTIAL TRANSFORMER
ESC	EQUIPMENT GROUNDING CONDUCTOR	PV	PER UNIT
ELEC	ELECTRICAL	PVC	POLYVINYL CHLORIDE
ELEV	ELEVATOR; ELEVATION	PWR	POWER
EM	EMERGENCY	QUAN: QTY	QUANTITY
EMS	ENERGY MANAGEMENT SYSTEM	R	RESISTANCE; RELOCATED
EMT	ELECTRICAL METALLIC TUBING	RECEPT	RECEPTACLE
ENCL	ENCLOSURE	REFR	REFRIGERATOR
ENG	ENGINE	REQD	REQUIRED
EQUIP	EQUIPMENT	RGS	RIGID GALVANIZED STEEL
EST	ESTIMATED	RLA	RUNNING LOAD AMPS
EW	ELECTRIC WATER COOLER	RM	ROOM
EW	ELECTRIC WATER HEATER	RMC	RIGID METALLIC CONDUIT
EXP	EXPOSED	RMS	ROOT MEAN SQUARE
EXT	EXTERIOR	RNC	RIGID NON-METALLIC CONDUIT
F	FUSED; FAHRENHEIT	RAINTIGHT	RAINTIGHT
FA	FIRE ALARM	SCCR	SHORT-CIRCUIT CURRENT-RATING
FAA	FIRE ALARM ANNUNCIATOR	SCHED	SCHEDULE
FACP	FIRE ALARM CONTROL PANEL	SCR	SHORT CIRCUIT RATING
FC	FOOT-CANDLE	SEC	SERVICE ENTRANCE; SERVICE EQUIPMENT
FD	FUSED DISCONNECT	SEC	SECONDARY
FDR	FEDER	SF	SINGLE
FIN	FINISHED	SPD	SURGE PROTECTIVE DEVICE
FIXT	FIXTURE	SPOT	SINGLE POLE, DOUBLE THROW
FLA	FULL LOAD AMPS	SPKR	SPEAKER
FLR	FLOOR	SPT	SINGLE POLE, SINGLE THROW
FLUOR	FLUORESCENT	SQ	SQUARE
FM	FREQUENCY MODULATION; FACTORY MUTUAL	SQ FT	SQUARE FEET
FT	FOOT; FEET	SQ IN	SQUARE INCH
FURN	FURNISHED	SS	STAINLESS STEEL; SAFETY SWITCH
FVNR	FULL VOLTAGE NON-REVERSING	SS	STAINLESS STEEL
G	GROUND	ST	SHUNT TRIP
GA	GUAGE	STD	STANDARD
GALV	GALVANIZED	SURF	SURFACE
GC	GENERAL CONTRACTOR	SW	SWITCH
GEN	GENERATOR	SWD	SWITCHING DUTY
GFCI, GFI	GROUND FAULT CIRCUIT INTERRUPTER	SWBD	SWITCHBOARD
GFP	GROUND FAULT PROTECTION	SY	SYMMETRICAL
GND	GROUND	T	TEMPERATURE; TRANSFORMER
GRS, GRC	GALVANIZED RIGID STEEL CONDUIT	TC	TERMINAL BLOCK
H	HIGH	TC	TEMPERATURE CONTROLS CONTRACTOR
HD	HEAVY DUTY; HIGH DEFINITION	TCC	TEMPERATURE CONTROL PANEL
HG	MERCURY	TCP	TEMPERATURE CONTROL PANEL
HOA	HAND-OFF-AUTOMATIC	TD	TIME DELAY
HORIZ	HORIZONTAL	TELE	TELEPHONE
HP	HORSEPOWER	TGB	TELECOMMUNICATIONS GROUNDING BUSBAR
HPS	HIGH PRESSURE SODIUM	THD	TOTAL HARMONIC DISTORTION; THREAD
HR	HOUR	TMBG	TELECOMMUNICATIONS MAIN GROUNDING BUSBAR
HRS/DAY	HOURS PER DAY	TO	TELECOMMUNICATIONS OUTLET
HT	HEIGHT	TAMP	TAMPER RESISTANT
HV	HIGH VOLTAGE	TTB	TELEPHONE TERMINAL BOARD
HZ	HERTZ	TV	TELEVISION
ID	INSIDE DIAMETER	TVSS	TRANSIENT VOLTAGE SURGE SUPPRESSOR
IDF	INTERMEDIATE DISTRIBUTION FRAME	TYP	TYPICAL
IEEE	INSTITUTE OF ELECTRICAL AND ELECTRONICS ENGINEERS	UC	UNDER (CABINET OR COUNTER)
IG	ISOLATED GROUND	UF	UNDERGROUND FEEDER
IMC	INTERMEDIATE METAL CONDUIT	UG	UNDERGROUND
IMP	IMPEDANCE	UHF	ULTRA HIGH FREQUENCY
IN	INCH	UL	UNDERWRITERS LABORATORY
INCN	INCANDESCENT	UNFIN	UNFINISHED
INSUL	INSULATION; INSULATED	UNO	UNLESS NOTED OTHERWISE
INT	INTERIOR; INTERNAL	UTIL	UTILITY
INV EL	INVERTED ELEVATION	UTP	UNSHIELDED TWISTED PAIR
J	JOULE; JUNCTION	V	VOLT
JB	JUNCTION BOX	VA	VOLT AMPERES
K	THOUSAND	VAR	VOLT AMPERES REACTIVE
KCMIL	THOUSAND CIRCULAR MILS	VERT	VERTICAL
KHZ	KILOHERTZ	VFD	VARIABLE FREQUENCY DRIVE
KK	KIRK KEY	VHF	VERY HIGH FREQUENCY
KP	KEYPAD	VOL	VOLUME
KV	KILOVOLT	W	WIRE; WATT; WIDE
KVA	KILOVOLT AMPERE	WI	WITH
KVAR	KILOVOLT AMPERE REACTIVE	WAP	WIRELESS ACCESS POINT
KW	KILOWATT	WG	WIRE GUARD
KWH	KILOWATT-HOUR	WM	"WIREMOLD" (SURFACE RACEWAY)
L	LENGTH; LONG; LUMEN	WP	WEATHERPROOF
LB	POUND; ELL CONDUIT BODY	WT	WEIGHT; WATERTIGHT
LED	LIGHT EMITTING DIODE	XFMR	TRANSFORMER
LF	LINEAR FOOT	XFR	TRANSFER
LDO	LAMP LUMEN DEPRECIATION	Y	WYE
LO	LOCK OUT	-	DEGREE
LRA	LOCKED ROTOR AMPS	Δ	PHASE; DIAMETER
LT	LIGHT, LIGHT-TIGHT	Ø	POUND; NUMBER
LTV	POWER LIMITED LOW VOLTAGE	#	PERCENT
M	METER	%	PERCENT
MA	MILLIAMPERE	@	AT
MAG STR	MAGNETIC STARTER	-	APPROXIMATELY
MAN	MANUAL	-	FEE
MAT	MATERIAL	-	INCHES
MATV	MASTER ANTENNA TELEVISION		
MAX	MAXIMUM		
MC	METAL CLAD CABLE; MOTOR CONTROLLER		
MCA	MINIMUM CIRCUIT AMPS		
MCB	MAIN CIRCUIT BREAKER		
MCC	MOTOR CONTROL CENTER		
MCCB	MOLDED CASE CIRCUIT BREAKER		
MCM	THOUSAND CIRCULAR MILS		
MCP	MOTOR CIRCUIT PROTECTOR		
MCS	MOTOR CIRCUIT SWITCH		

NOT ALL SYMBOLS ON THIS SHEET ARE USED IN THESE DOCUMENTS.

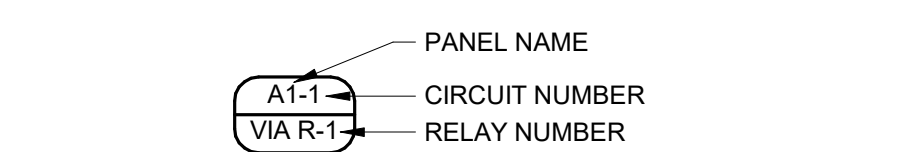
TYPICAL WIRING DESIGNATIONS



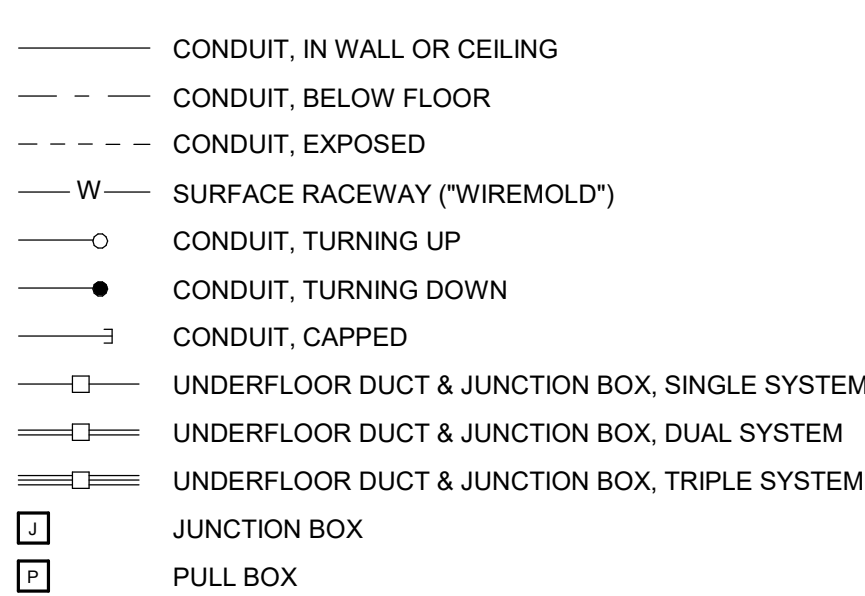
ROOM CIRCUIT DESIGNATIONS



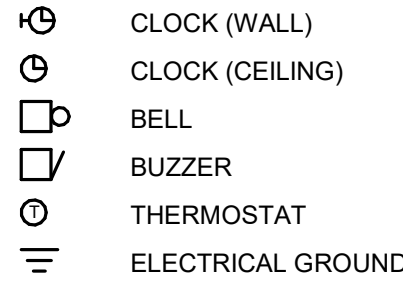
ROOM CIRCUIT DESIGNATIONS WITH RELAY NUMBER



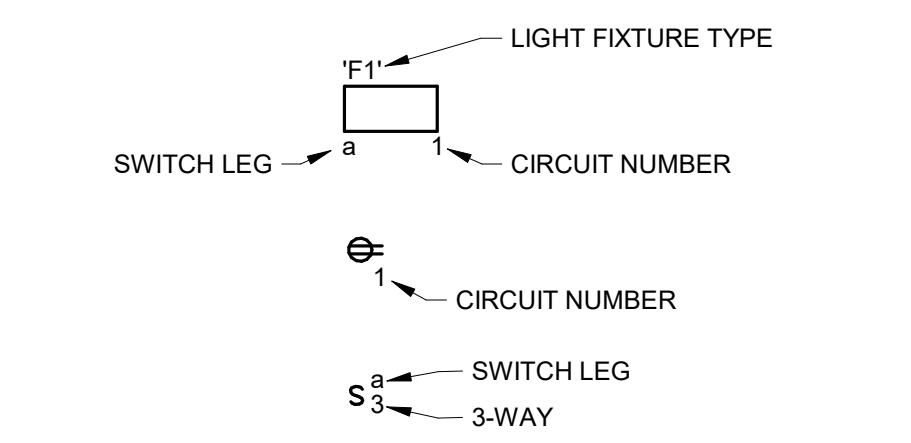
RACEWAYS



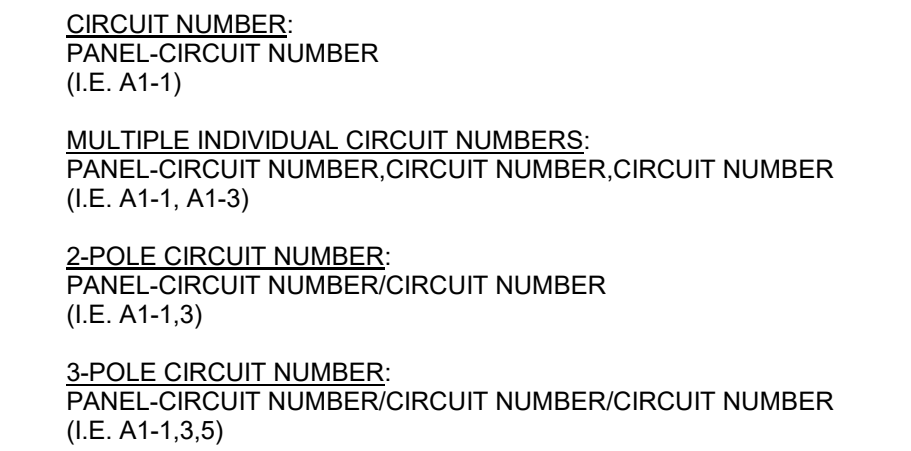
MISCELLANEOUS



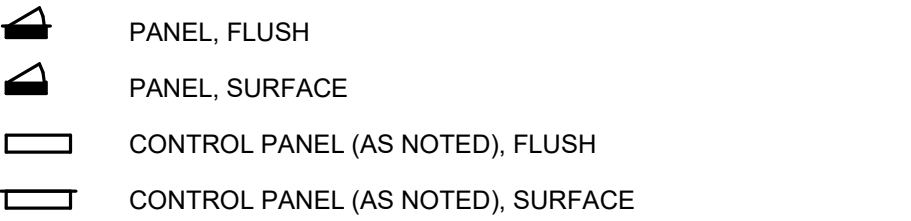
TYPICAL DEVICE DESIGNATIONS



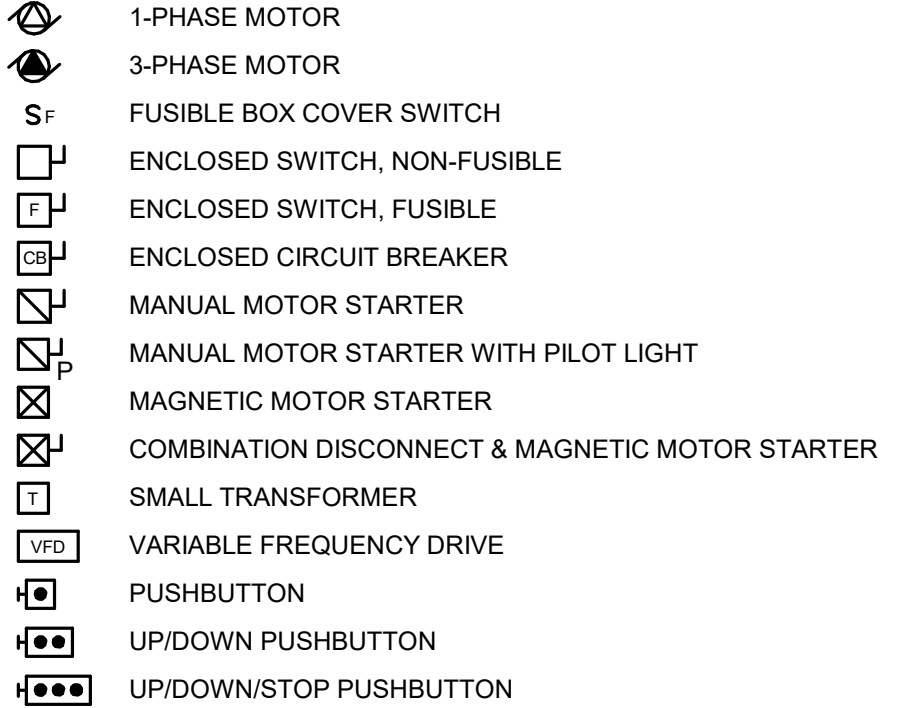
CIRCUIT DESCRIPTIONS



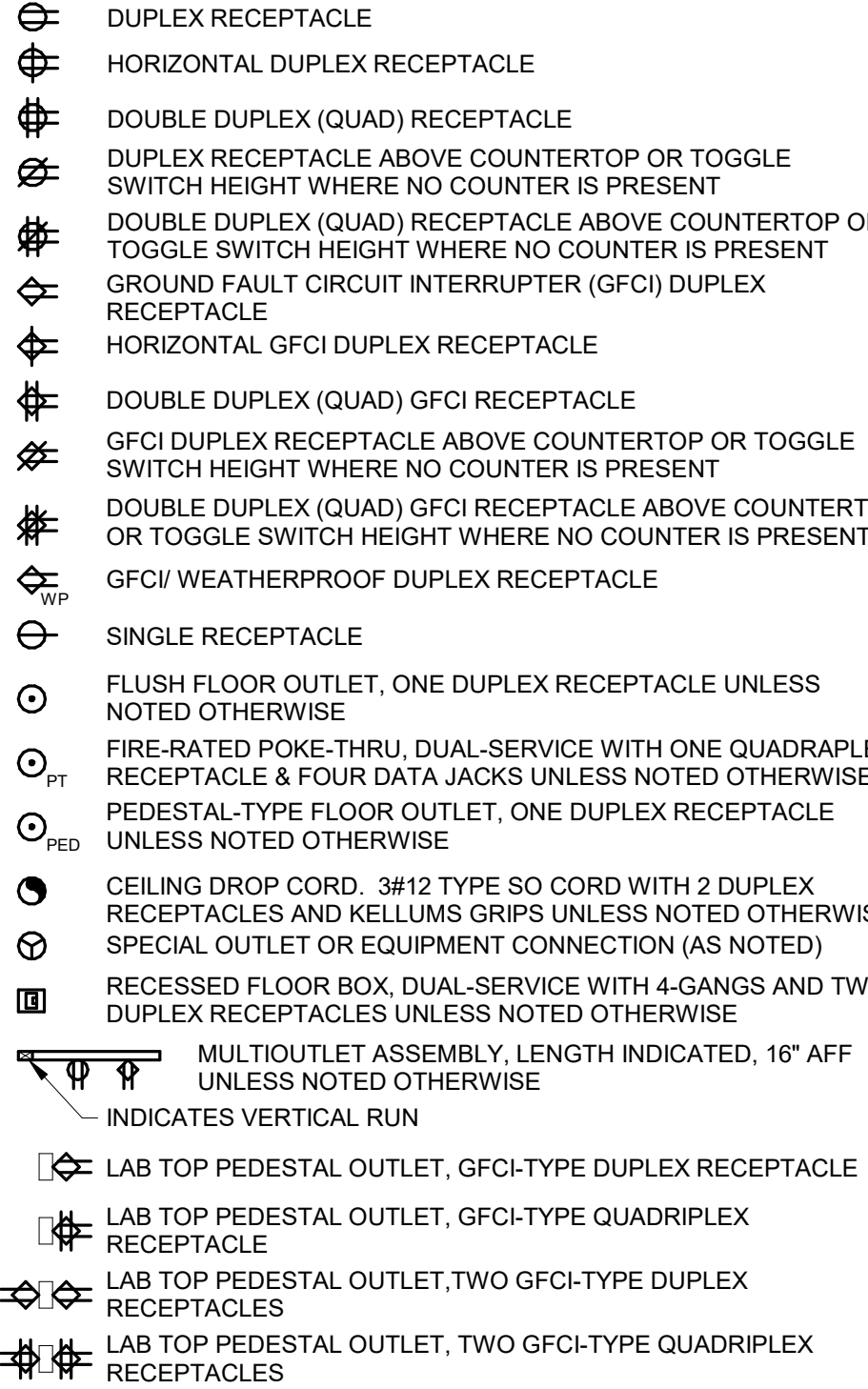
PANELS



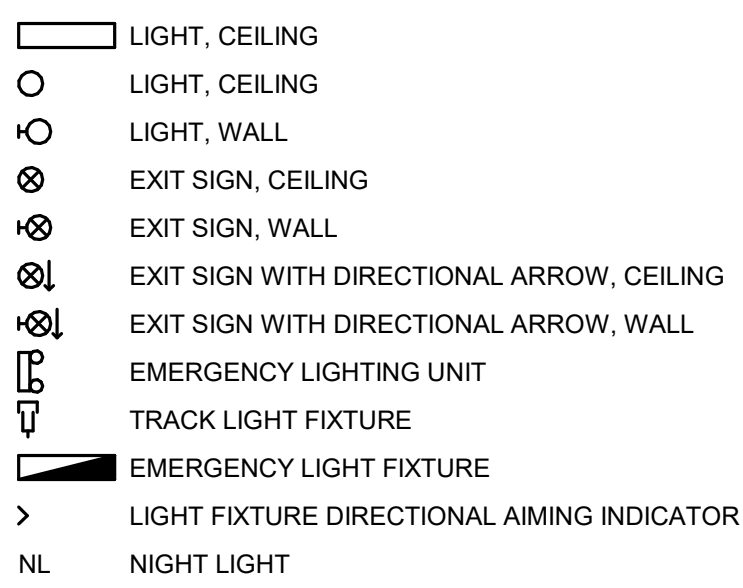
POWER EQUIPMENT



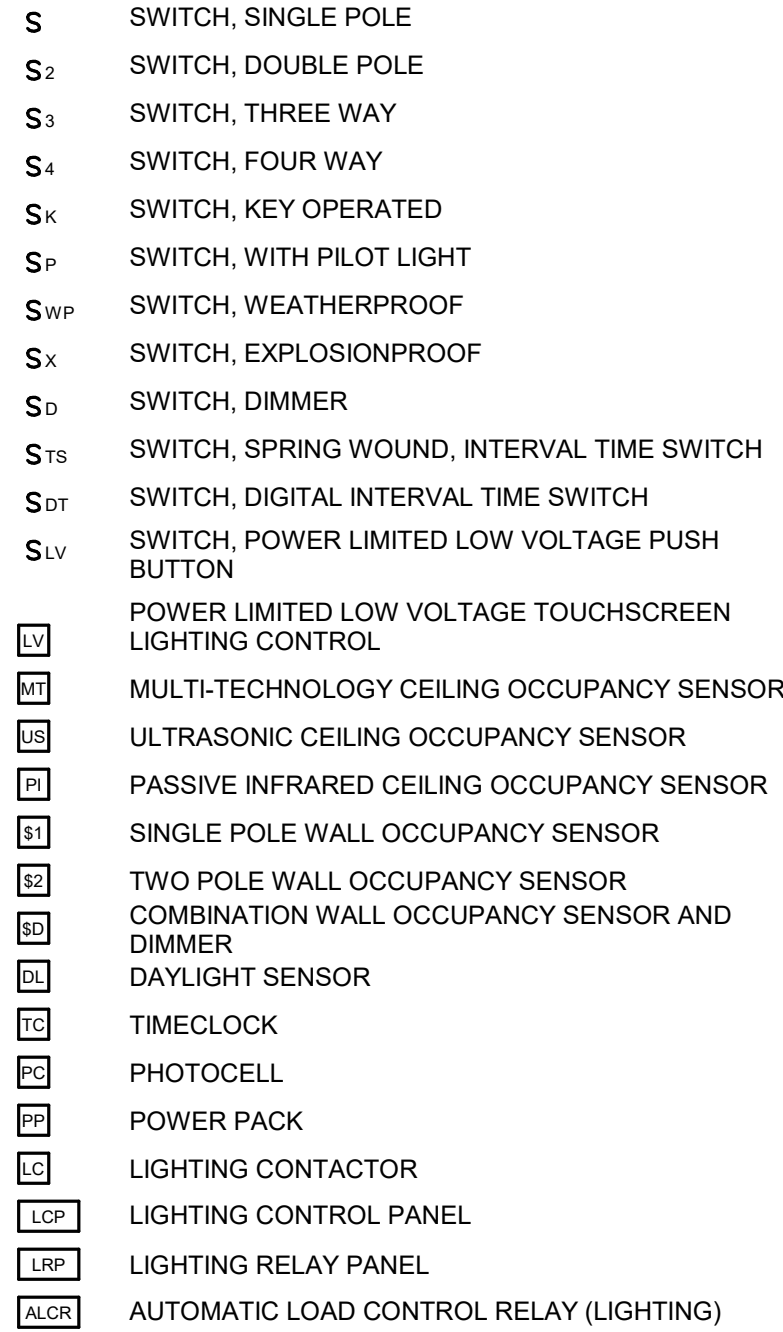
RECEPTACLES AND OUTLETS



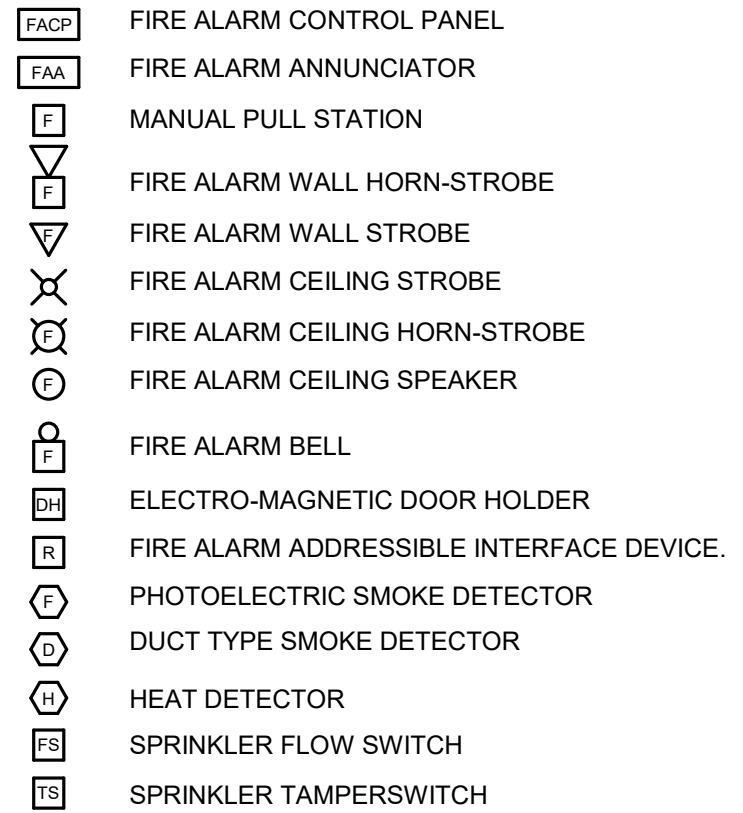
LIGHT FIXTURES



SWITCHES



FIRE ALARM SYSTEMS



GENERAL NOTES:

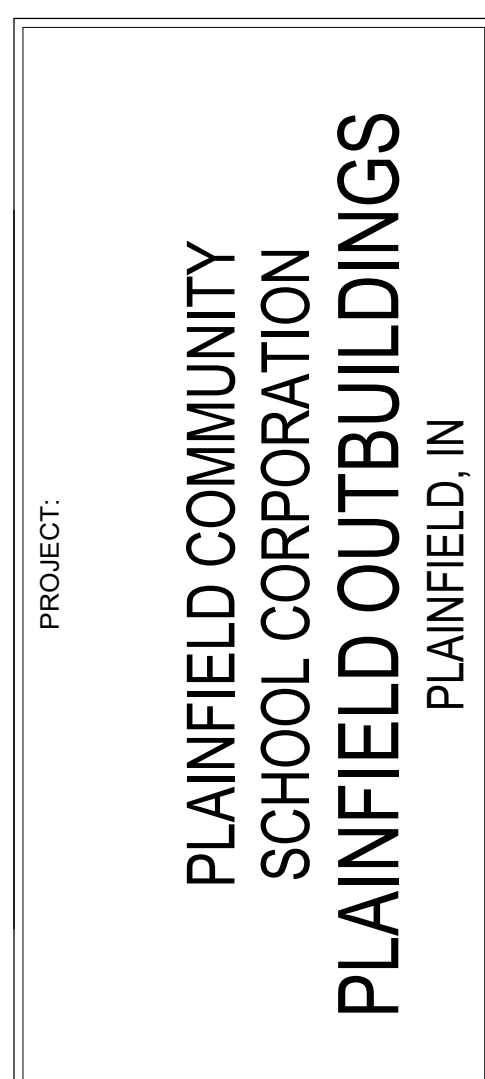
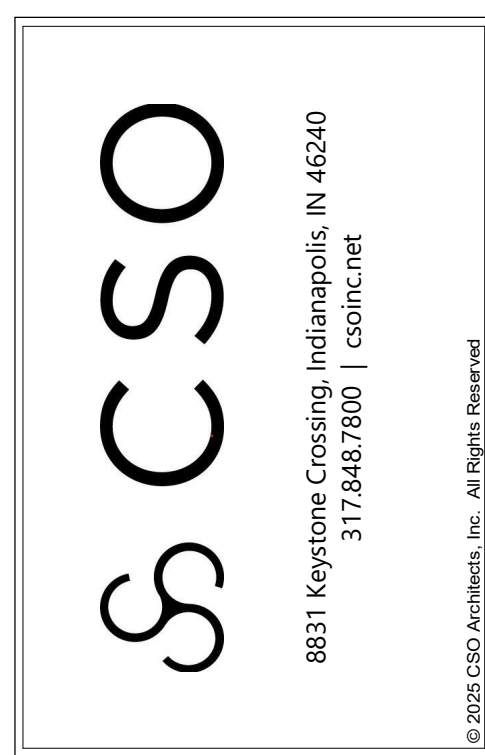
- 120 VOLT CIRCUITS SHALL UTILIZE SEPARATE INDEPENDENT NEUTRAL CONDUCTORS. DO NOT SHARE NEUTRALS.
- CONTRACTOR SHALL COORDINATE WITH ALL TRADES. NO ADDITIONAL COMPENSATION WILL BE ALLOWED FOR INCORRECT WORK, OR FOR INFRINGEMENT UPON OTHERS' WORK, DUE TO A LACK OF COORDINATION.
- DEVICES IN GENERAL SHALL BE CENTERED IN WALL SPACE IN WHICH THEY ARE INSTALLED OR THEY SHALL BE SPACED SYMMETRICALLY (FOR EXAMPLE, CENTER DEVICES WHEN MOUNTED ON FACE OF COLUMNS).
- COORDINATE AND VERIFY LOCATIONS OF DEVICES WITH BLOCK COURSING, FINISH MATERIALS, CASEWORK, ETC. PRIOR TO ROUGH-IN.
- WIRING SHALL BE MINIMUM #12 AWG IN 3/4" EMT CONDUIT UNLESS OTHERWISE NOTED OR REQUIRED.
- WHERE SURFACE DEVICE BOXES ARE PERMITTED, DO NOT USE PLASTER RINGS. USE EXPOSED WORK COVERS INTENDED FOR THE PURPOSE.
- WHERE SURFACE CONDUIT OR EMT IS PERMITTED, DO NOT USE CONDUIT HANGERS LESS THAN 8-FEET AFF. USE ONE- OR TWO-HOLE STRAPS SO THAT NO SHARP EDGES PROTRUDE FROM THE WALL.
- A MAXIMUM OF THREE SINGLE-PHASE CIRCUITS SHALL BE INSTALLED IN A SINGLE CONDUIT.
- LOCATION OF LIGHT FIXTURES SHALL BE COORDINATED IN FIELD AND LOCATED TO PROVIDE THE BEST ILLUMINATION OF THE SPACE AND EQUIPMENT. COORDINATE WITH ENGINEER.
- PROVIDE FIRESTOPPING AT PENETRATIONS THROUGH FIRE-RATED CONSTRUCTION.
- DO NOT INSTALL RACEWAYS IN FLOOR SLABS. INSTALL RACEWAYS BELOW SLAB ON GRADE AT LEAST 6-INCHES BELOW BOTTOM OF SLAB. FEEDER CONDUITS SHALL BE AT LEAST 24-INCHES BELOW BOTTOM OF SLAB.
- UNLESS NOTED OTHERWISE, JUNCTION BOXES AND PULL BOXES SHALL BE LISTED AND LABELLED BY A NATIONALLY RECOGNIZED TESTING LABORATORY.

BRANCH CIRCUIT WIRING CHART

FEEDER CONDUCTOR SIZES SHOWN ON THESE BID DOCUMENTS HAVE BEEN SELECTED TO MAINTAIN LESS THAN 2% VOLTAGE DROP AT POTENTIAL FULL LOAD CONDITION (80% OF CIRCUIT SIZE) PER ANTICIPATED ROUTING AND CONDUCTOR LENGTH. BRANCH CIRCUIT CONDUCTORS SHALL BE SIZED TO MAINTAIN LESS THAN 3% VOLTAGE DROP FROM PANELBOARD TO LOAD BASED UPON 80% OF CIRCUIT SIZE LOAD CONDITIONS. THE FOLLOWING CHART REPRESENTS WIRE SIZES FOR A 20 AMP CIRCUIT BASED UPON CIRCUIT LENGTH IN ORDER TO MAINTAIN LESS THAN 3% VOLTAGE DROP FOR A 12 AMP LOAD. CONTRACTOR SHALL USE THIS CHART FOR BIDDING AND INSTALLATION GUIDELINES. FOR KNOWN CIRCUITS WITH LARGER LOAD CONDITIONS, CONTRACTOR SHALL ADJUST ACCORDINGLY. GROUND CONDUCTOR SIZES SHALL BE INCREASED SAME AS CIRCUIT CONDUCTORS, PER NEC. ADJUST RACEWAY SIZES ACCORDINGLY.

WIRE SIZE	120V-1P	208V-1P	208V-3P	277V-1P	480V-3P
#12	0'-80'	0'-140'	0'-160'	0'-185'	0'-375'
#10	81'-135'	141'-230'	161'-270'	186'-310'	376'-620'
#8	136'-200'	231'-350'	271'-410'	311'-470'	621'-940'
#6	201'-315'	351'-550'	411'-635'	471'-735'	941'-1475'

CONDUCTOR LENGTHS INDICATED ARE TO THE FIRST DEVICE (BUT MAINTAIN MAXIMUM 5% VOLTAGE DROP TO THE LAST DEVICE FOR KNOWN LOADS).

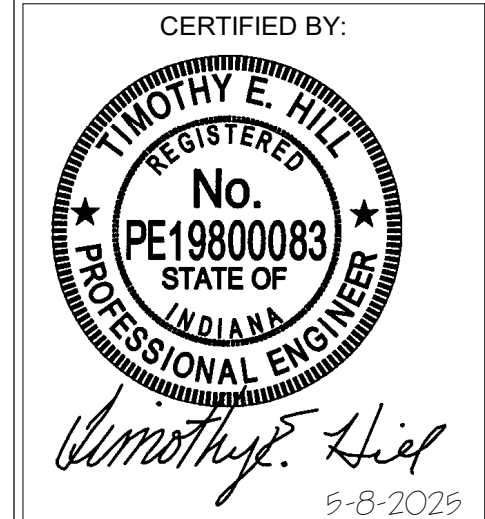


SCOPE DRAWINGS:  
These drawings indicate the general scope of the project. The drawings are not intended to be a substitute for the drawings of the project. The drawings are not intended to be a substitute for the drawings of the project. The drawings are not intended to be a substitute for the drawings of the project.

REVISIONS:  
1. REVISION 1: 05/08/2025

ISSUE DATE: 05/08/2025  
DRAWN BY: DMH  
CHECKED BY: TEH

DRAWING TITLE:  
SYMBOLS, ABBREVIATIONS, & GENERAL NOTES - ELECTRICAL



DRAWING NUMBER:  
E-001

PROJECT NUMBER:  
2025016

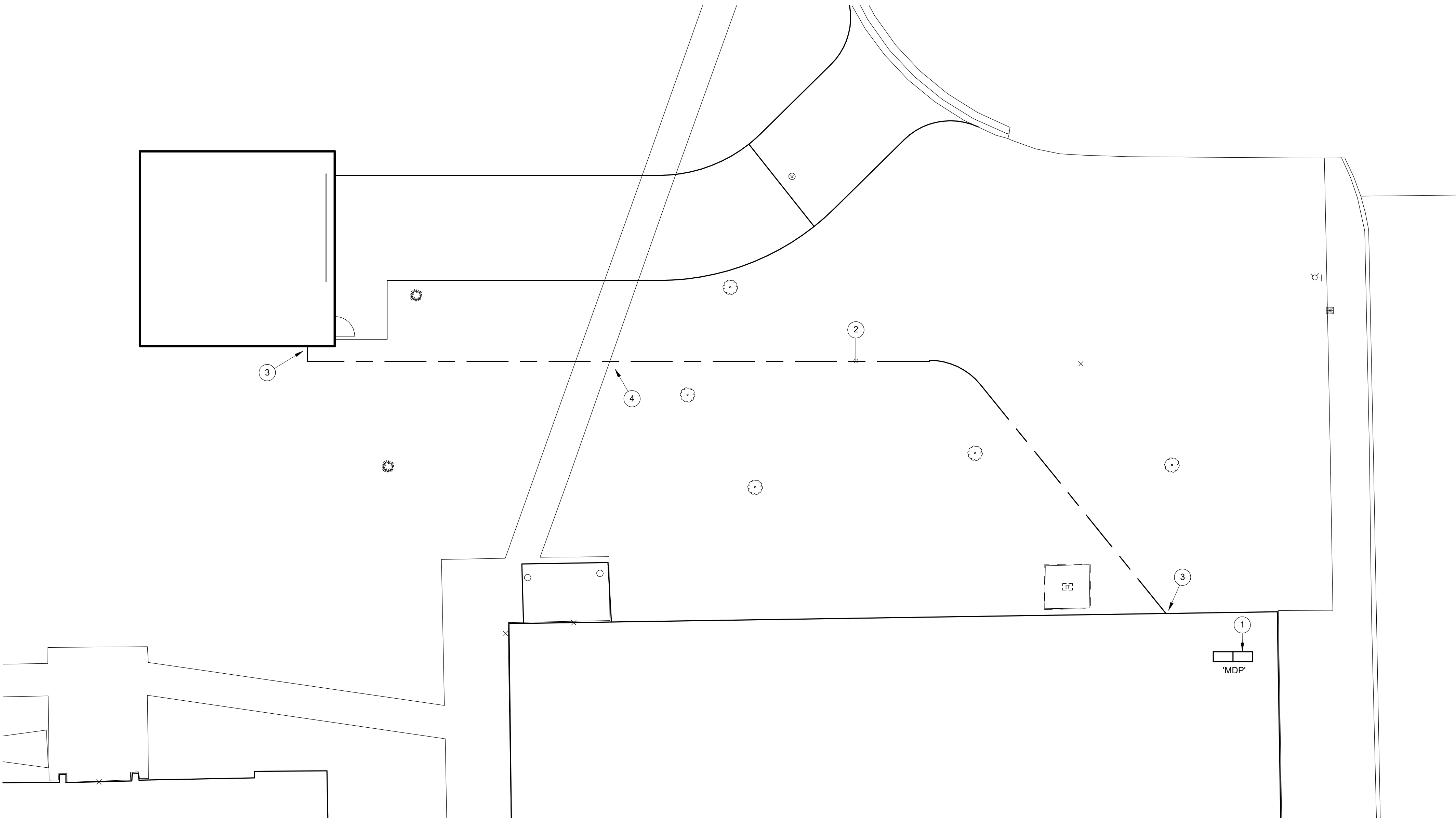






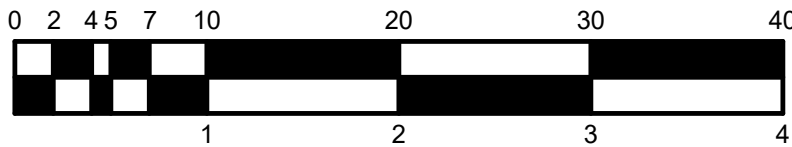


MDP LOCATED IN ELECTRICAL EQUIPMENT ROOM



SITE PLAN - BRENTWOOD - ELECTRICAL

SCALE: 1" = 10'-0"



RENOVATION LEGEND:

- WORK TO BE INSTALLED
- WORK TO REMAIN

GENERAL NOTES:

- SEE E-001 FOR GENERAL NOTES.

PLAN NOTES:

- FEED OUTDOOR STORAGE BUILDING FROM SPARE 60A-3P BREAKER (USING 2 POLES FOR SINGLE PHASE SERVICE).
- 3/4" 1#6G - 2" PVC SCHEDULE 80 BURIED AT 18" BELOW GRADE. PROVIDE TRACEABLE WARNING TAPE AT 12" BELOW GRADE.
- WHERE EXPOSED, THE PVC RACEWAY TO BE CONVERTED TO GRS.
- DIRECTIONAL BORE UNDER EXISTING CONCRETE WALKWAY.



SCSO

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D&A #2500

**RE Dimond**  
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Consulting Engineers

732 North Capitol Avenue  
Indianapolis, IN 46204  
Phone: (317) 834-4972  
Fax: (317) 638-5725

PROJECT:

PLAINFIELD COMMUNITY  
SCHOOL CORPORATION  
PLAINFIELD OUTBUILDINGS  
PLAINFIELD, IN

SCOPE DRAWINGS:  
These drawings indicate the general scope of the project in terms of mechanical design concept, the arrangement of the building, the type of equipment, and the type of structural, mechanical and electrical systems. The drawings do not necessarily indicate or describe all work required for the performance and completion of the requirements of the Contract.  
On the basis of the general scope indicated or described, the trade contractors shall furnish all items required for the proper execution and completion of the work.

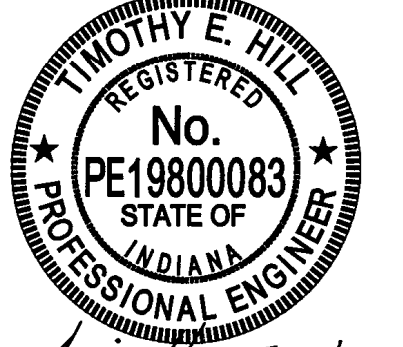
REVISIONS:

ISSUE DATE	DRAWN BY	CHECKED BY
05/08/2025	DMH	TEH

DRAWING TITLE:

SITE PLAN -  
BRENTWOOD -  
ELECTRICAL

CERTIFIED BY:

  
*Timothy E. Hill*  
5-8-2025

DRAWING NUMBER

E-101

PROJECT NUMBER

2025016





PLAINFIELD COMMUNITY  
SCHOOL CORPORATION  
PLAINFIELD OUTBUILDINGS  
PLAINFIELD, IN


**SCOPE DRAWINGS:**  
These drawings indicate the general scope of the project, the scope of architectural design concept, the dimensions of building, the major architectural elements and the type structural, mechanical and electrical systems.  
The drawings do not necessarily indicate or describe all required for full performance and completion of the elements of the Contract.  
On the basis of the general scope indicated or described, trade contractors shall furnish all items required for the proper execution and completion of the work.

 REVISIONS:

SUE DATE	DRAWN BY	CHECKED BY
5/08/2025	DMH	TEH

DRAWING TITLE:  
OUTBUILDING  
PLANS -  
ELECTRICAL

CERTIFIED BY:



*Timothy E. Hill*  
5-8-2025

DRAWING NUMBER  
**E201**

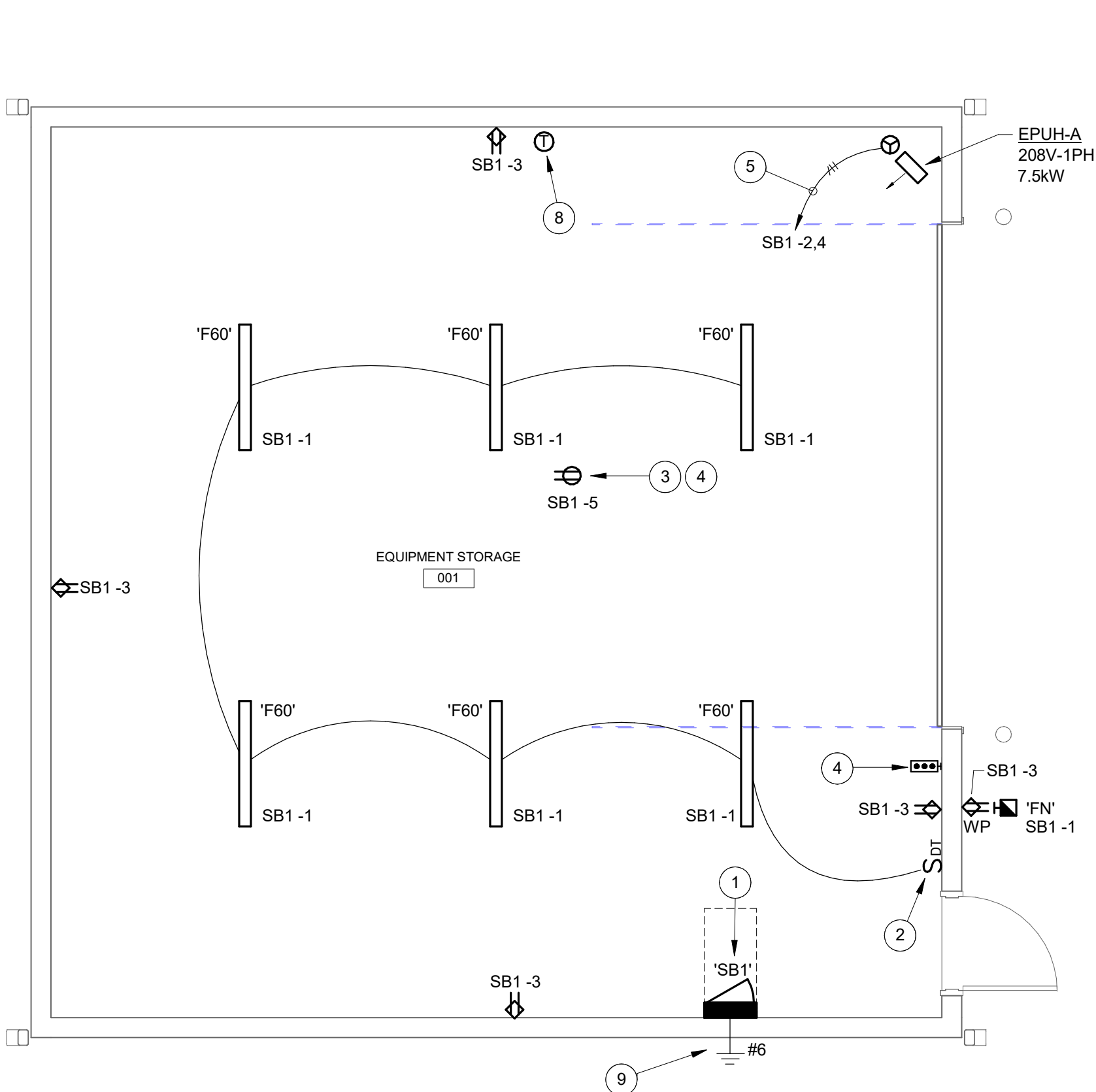
PROJECT NUMBER  
2025016

**GENERAL NOTES:**

1. SEE E-001 FOR GENERAL NOTES.
2. INTERIOR LIGHT FIXTURES TO BE SURFACE MOUNTED TO CEILING.
3. EXTERIOR LIGHT FIXTURES TO BE MOUNED AT 9'-0" AFF.
4. MOUNT INTERIOR WALL RECEPTACLES IN STORAGE BUILDINGS AT 48" AFF.
5. PROVIDE WEATHERPROOF, IN-USE COVERS ON EXTERIOR OUTLETS
6. RECEPTACLE DEVICE COLOR: IVORY

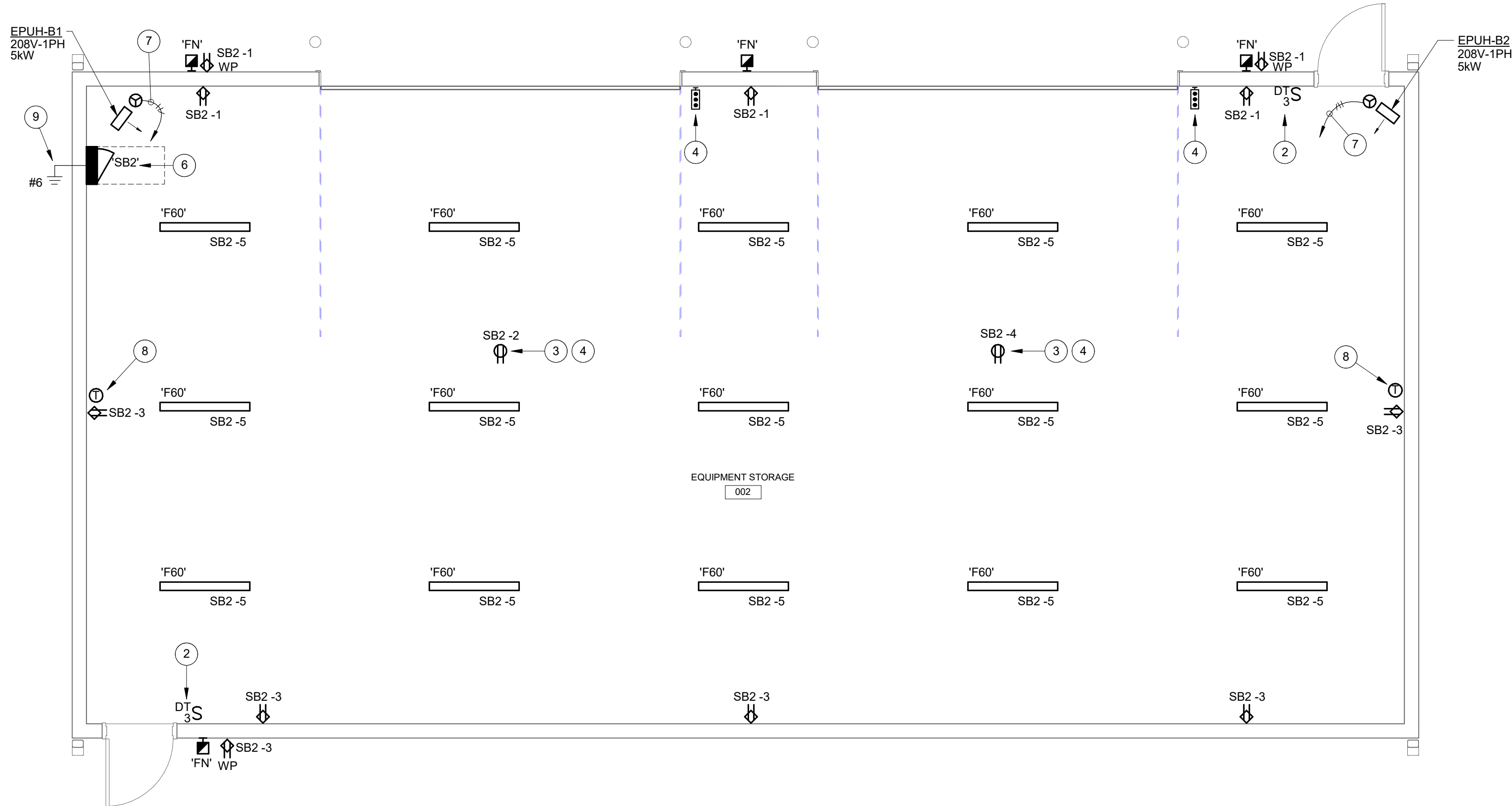
**# PLAN NOTES:**

1. PROVIDE 120/240VAC, 1PH, 16-CKT, NEMA 1 LOAD CENTER WITH 60A MAIN BREAKER.
2. PROVIDE LEVITON #D7202 5-BUTTON DIGITAL, COUNTDOWN TIMER FOR LIGHTS, BULBS TO BE SET AT 15/30/60/120 MINUTE INTERVALS. WHEN TIMERS ARE SET IN A 3-WAY CONTROL, EITHER TIMER CAN TURN ON OR OFF THE LOAD WITH BOTH TIMERS SHOWING THE SAME COUNTDOWN LED SEQUENCE. COLOR: IVORY.
3. MOUNT RECEPTACLE ON CEILING FOR GARAGE DOOR OPERATOR. RECEPTACLE TO BE FED FROM 50A BREAKER, COORDINATE EXACT LOCATION WITH OVERHEAD DOOR PROVIDER.
4. OVERHEAD DOOR OPERATOR AND 3-BUTTON CONTROL TO BE PROVIDED AND INSTALLED BY OVERHEAD DOOR PROVIDER.
5. 2#8, 1#10G, -3/4"
6. PROVIDE 120/240VAC, 1PH, 16-CKT, NEMA 1 POAD CENTER WITH 100A MAIN BREAKER.
7. 2#10, 1#10G, -3/4"
8. PROVIDE RACEWAY AND BOX TO UNIT HEATER THERMOSTAT. MOUNT AT 48" AFF.
9. PROVIDE 10 COPPER-CLAD GROUND ROD.



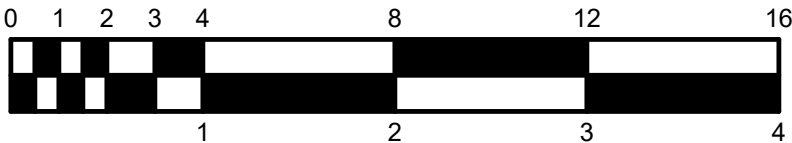
## 1 GROUND FLOOR - BRENTWOOD - ELECTRICAL

SCALE: 1/4" = 1'-0"

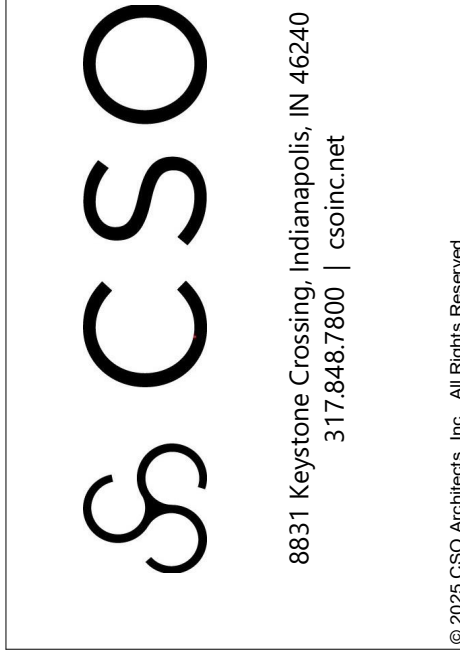


## 2 GROUND FLOOR - CLARKS CREEK - ELECTRICAL

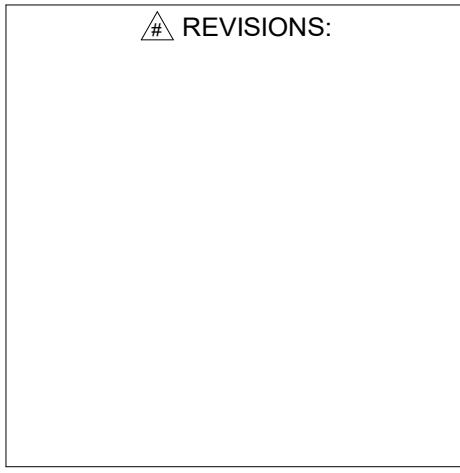
SCALE: 1/4" = 1'-0"



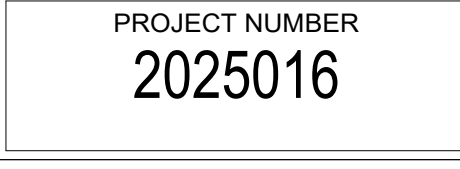
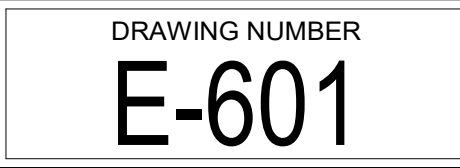
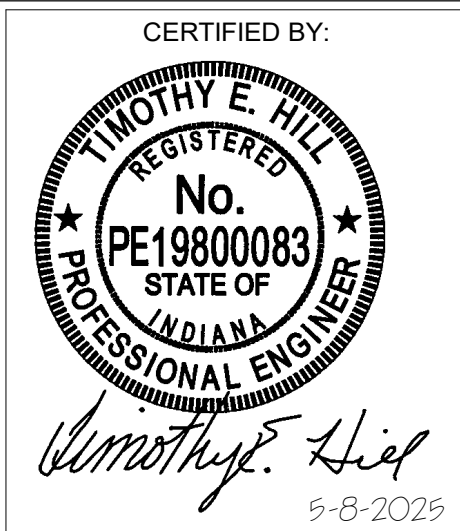
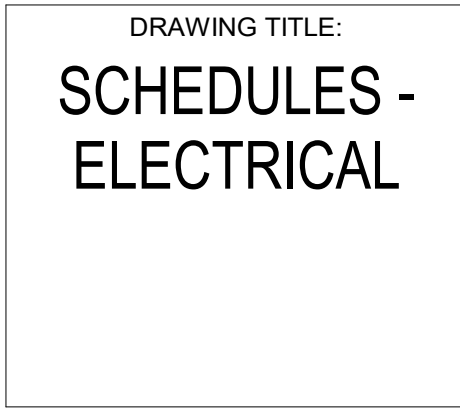




SCOPE DRAWINGS:  
These drawings indicate the general scope of the project. The drawings are not intended to be a contract. The drawings are not intended to be a contract. The drawings are not intended to be a contract. The drawings are not intended to be a contract.



ISSUE DATE	DRAWN BY	CHECKED BY
05/08/2025	DMH	TEH



SB1				PANELBOARD SCHEDULE													
LOCATION: EQUIPMENT STORAGE 001				SCCR (AMPS RMS)...				10,000				SERVICE : 120/208V 1Ø 3-Wire+Ground				MAIN: MCB	
MOUNTIN... SURFACE												NEMA: 1				AMP: 60 A	
CKT	DESCRIPTION			NOTE	AMP	POLE	A	B		POLE	AMP	NOTE	DESCRIPTION			CKT	
1	LIGHTING				20 A	1	328 / 3750			2	40 A		ELECTRIC UNIT HEATER			2	
3	RECEPTACLES				20 A	1		900 / 3750		1	--					4	
5	GARAGE DOOR OPERATOR			1	20 A	1	1920 / 0			1	--		SPACE			6	
7	SPACE			--	1		0 / 0			1	--		SPACE			8	
9	SPACE			--	1		0 / 0			1	--		SPACE			10	
11	SPARE				20 A	1		0 / 0		1	20 A		SPACE			12	
13	SPARE				20 A	1	0 / 0			1	20 A		SPACE			14	
15	SPARE				20 A	1		0 / 0		1	20 A		SPACE			16	
					TOTALS :		5998 VA				4650 VA					2	
TOTAL CONNECTED LOAD (VA) :					10648 VA							TOTAL CONNECTED LOAD (AMPS) :					51 A
REMARKS:					NOTES: 1. GFCI BREAKER												

SB2				PANELBOARD SCHEDULE													
LOCATION: EQUIPMENT STORAGE 002				SCCR (AMPS RMS)...				10,000				SERVICE : 120/208V 1Ø 3-Wire+Ground				MAIN: MCB	
MOUNTIN... SURFACE												NEMA: 1				AMP: 100 A	
CKT	DESCRIPTION			NOTE	AMP	POLE	A	B	POLE	AMP	NOTE	DESCRIPTION			CKT		
1	RECEPTACLES - NORTH				20 A	1	900 / 1920		1	20 A	1	GARAGE DOOR OPERATOR - WEST			2		
3	RECEPTS - EAST/WEST/SOUTH				20 A	1		1080 / 1920	1	20 A	1	GARAGE DOOR OPERATOR - EAST			4		
5	LIGHTING				20 A	1	720 / 0		1	--		SPACE			6		
7	ELECTRIC UNIT HEATER - WEST				30 A	2	2500 / 2500	2500 / 2500	2	30 A		ELECTRIC UNIT HEATER - EAST			8		
9	SPARE				20 A	1		0 / 0	1	20 A		SPARE			10		
11	SPARE				20 A	1	0 / 0		1	20 A		SPARE			12		
13	SPARE				20 A	1		0 / 0	1	20 A		SPARE			14		
15	SPARE				20 A	1		0 / 0	1	20 A		SPARE			16		
TOTALS :					8540 VA			8000 VA									
TOTAL CONNECTED LOAD (VA) : 16540 VA										TOTAL CONNECTED LOAD (AMPS) : 80 A							
REMARKS:					NOTES: GFCI BREAKER												

LIGHT FIXTURE SCHEDULE											
MARK	DESCRIPTION	MOUNTING	WATTS	CRI	COLOR	LUMENS	VOLTS	MANUFACTURER(S)	MARK		
F60	4-FOOT LENSED INDUSTRIAL, FORMED STEEL HOUSING, WHITE FINISH, SEMI-FROST ACRYLIC DIFFUSER.	SURFACE/ CHAIN HUNG	48 W	80	4000K	5129	120-277V	COLUMBIA MPS SERIES GREE LSA SERIES LITHONIA ZL1D SERIES METALUX SNLED SERIES	F60		
FN	ARCHITECTURAL WALL PACK, WET LOCATION LISTED, FINISH: MATTE DARK BRONZE, INTEGRAL PHOTOCELL.	SURFACE WALL	40 W	70	4000K	4000	120-277V	HUBBELL SG SERIES LITHONIA WFX SERIES LUMARK XTOR SERIES	FN		

ELECTRIC UNIT HEATER SCHEDULE															
MARK NO	DRAWING NAME &/OR PURPOSE	SPECIFICATION			MANUFACTURER & MODEL NO	ELECTRIC DATA			LENGTH	WIDTH	HEIGHT	INSTALLED HEIGHT	STYLE	WEIGHT (LBS)	REMARKS
		SECTION	NAME	EQUIPMENT TYPE		WATTS	VOLTS	PHASE							
EPUH-A		23 82 39	UNIT HEATERS - ELECTRIC	PROPELLER UNIT HEATERS	TPI F1FUH07CA1 OR EQUAL	7.5	208	1	20	11	13	9		40	FAN GUARD, HORIZONTAL AND VERTICAL ADJUSTABLE LOUVERS. PROVIDE LINE VOLTAGE THERMOSTAT
EPUH-B		23 82 39	UNIT HEATERS - ELECTRIC	PROPELLER UNIT HEATERS	TPI F1FUH05003 OR EQUAL	5	208	1	20	11	13	9		36	FAN GUARD, HORIZONTAL AND VERTICAL ADJUSTABLE LOUVERS. PROVIDE LINE VOLTAGE THERMOSTAT